# APPENDIX A

- 1. Review comments
- 2. Selection process

#### Seth Asante

From:	Lipton, Amitai I. (DOT) <amitai.lipton@state.ma.us> on behalf of Lipton, Amitai I. (DOT)</amitai.lipton@state.ma.us>
Sent:	Wednesday, November 13, 2019 1:03 PM
То:	Seth Asante
Cc:	Chen-Yuan Wang; Mark Abbott; Lavallee, Carrie E. (DOT); Worhunsky, Courtney (DOT); Kulen, Raj (DOT)
Subject:	RE: FFY 2019 Low-Cost Improvements to Express-Highway Bottleneck Locations

Good afternoon Seth,

D6 Traffic section has reviewed the Draft report and submits the following comments for Chapter 5 (I-93 SB Braintree location):

- Alternative 1 (lengthen HOV/zipper lane merge): We note that a project has been initiated to replace the HOV/zipper systems; it may be possible to incorporate the proposed modifications into that project to avoid duplicated efforts/expenses.
- Alternative 2 (restripe shoulder to lengthen the acceleration lane): While lengthening the Furnace Brook Pkwy acceleration lane past Exit 7 (the split) might have some operational improvements during the PM peak period, we would want to evaluate any safety trade-offs involved with removing a shoulder, as there would not be any space left on the roadway for disabled vehicles. Some drivers might also choose to enter the long acceleration lane to "bypass" slow traffic in the general travel lanes. We would want to evaluate more thoroughly the safety and operational effects of having an unusually long acceleration lane that is less than 12 feet wide.
- Table 8 should include a baseline/no-build condition to compare with the 3 identified alternatives, similar to Table 9.
- Costs -- We feel Alternative 1 would be more expensive than estimated (to account for changes to the zipper barrier operations and any castings in the median area) while Alternative 2 would be less expensive than estimated (since extensive repaying should not be necessary).

Thank you, Amitai

From: Seth Asante <sasante@ctps.org>
Sent: Wednesday, November 13, 2019 09:20
To: Lipton, Amitai I. (DOT) <Amitai.Lipton@dot.state.ma.us>; Vatan, Geraldine T. (DOT)
<Geraldine.Vatan@dot.state.ma.us>; Raphael, Connie J. (DOT) <Connie.Raphael@dot.state.ma.us>; Timoner, Sara (DOT)
<Sara.Timoner@dot.state.ma.us>
Cc: Chen-Yuan Wang <cwang@ctps.org>; Mark Abbott <mabbott@ctps.org>
Subject: FFY 2019 Low-Cost Improvements to Express-Highway Bottleneck Locations

#### Good morning,

This is a friendly reminder to send in your comments on the attached low-cost express-highway bottlenecks study. They were due on November 8.

MPO staff analyzed two bottleneck locations in the study:

- Location 1—I-93 Northbound between Exit 40 (Route 62) and Exit 41 (Route 125) in Wilmington
- Location 2—I-93 Southbound at the end of the HOV Zipper Lane in Quincy and Braintree

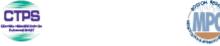
The study results for Locations 1, which is in MassDOT Highway District 4, is presented in Chapter 4 of the report. The study results for Locations 2, which is in MassDOT Highway District 6, is presented in Chapter 5.

Your comments are welcomed; please send them to me by November 20.

Thank you, Seth

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# **BOSTON REGION METROPOLITAN PLANNING ORGANIZATION**



Stephanie Pollack, MassDOT Secretary and CEO and MPO Chair Karl H. Quackenbush, Executive Director, MPO Staff

# TECHNICAL MEMORANDUM

- DATE: December 20, 2018
- TO: Boston Region Metropolitan Planning Organization (MPO)
- FROM: Seth Asante, MPO Staff
- RE: Federal Fiscal Year 2019 Express-Highway Bottleneck Study Locations

This memorandum presents the process used to select the bottleneck study locations. MPO staff will submit this proposal to the MPO for discussion and approval

## 1 BACKGROUND

In Task 2 of the work program for the "Low-Cost Improvements to Express-Highway Bottleneck Locations: FFY 2019," MPO staff indicated in Task 2 screen bottleneck locations and select locations for analysis—that staff will present the results to the MPO for discussion.<sup>1</sup>

According to the Federal Highway Administration (FHWA), "Much of recurring congestion is due to physical bottlenecks—potentially correctible points on the highway system where traffic flow is restricted. While many of the nation's bottlenecks can only be addressed through costly major construction projects, there is a significant opportunity for the application of operational and low-cost infrastructure solutions to bring about relief at these chokepoints."<sup>2</sup>

The cause and duration of highway bottlenecks vary. In general, recurring bottlenecks, the subject of this study, are influenced by the design or operation present at the point where the bottleneck begins, for example, merges, diverges, lane drops, traffic weaving, abrupt changes in highway alignment, low-clearance structures, lane narrowing, intended disruption of traffic for management purposes, and less-than-optimal express-highway design.

<sup>&</sup>lt;sup>1</sup> Work Program to the Boston Region Metropolitan Planning Organization, "Low-Cost Improvements to Express-Highway Bottleneck Locations: FFY 2019," September 20, 2018.

<sup>&</sup>lt;sup>2</sup> Federal Highway Administration, *Recurring Traffic Bottlenecks: A Primer: Focus on Low-Cost Operations Improvements*, US Department of Transportation, Federal Highway Administration, June 2009, p. 1.

MPO staff analyzed several express-highway bottleneck locations in four previous studies; they were very well received by the Massachusetts Department of Transportation (MassDOT) and the FHWA.<sup>3,4,5,6</sup> Previous study locations included sections of Interstate 95 (I-95) in Burlington, Lexington, Waltham, and Weston; I-93 in Reading and Woburn; and sections of Route 24 in Randolph and Canton. Some of the recommendations from those studies have been implemented, and FHWA consultants have interviewed MPO staff about these successful implementations. Cost estimates for low-cost bottleneck improvements that have been implemented by the MassDOT Highway Division, or currently are in design status, range between \$10,000 and \$1 million.

### 2 SELECTION OF STUDY LOCATIONS

Selection of study locations was a two-stage process that comprised inventorying and screening candidate locations.

#### 2.1 Inventorying Candidate Locations

MPO staff developed an initial list of candidate locations in the MPO region based on the following parameters:

- Consultations with MassDOT Highway Division
- Staff knowledge of bottleneck locations in the Boston MPO region
- Review of congestion management process (CMP) monitoring data, and recent MPO and other planning studies

The inventory process yielded nine bottleneck locations for screening, which are presented in the following table. All nine of the locations are in the Boston Region MPO area.

## 2.2 Screening Candidate Locations

MPO staff selected two bottleneck locations for analysis in federal fiscal year (FFY) 2019. After consulting with the MassDOT Highway Division, staff determined that these two locations likely could be corrected with low-cost mitigation strategies. The other bottlenecks in the Boston Region MPO area also

<sup>&</sup>lt;sup>3</sup> Seth Asante, MPO staff, memorandum to the Transportation Planning and Programming Committee of the Boston Region Metropolitan Planning Organization, "Low-Cost Improvements to Bottleneck Locations," June 2, 2011.

<sup>&</sup>lt;sup>4</sup> Chen-Yuan Wang, MPO staff, memorandum to the Transportation Planning and Programming Committee of the Boston Region Metropolitan Planning Organization, "Low-Cost Improvements to Bottleneck Locations, Phase II," March 12, 2012.

<sup>&</sup>lt;sup>5</sup> Seth Asante, MPO staff, memorandum to the Boston Region Metropolitan Planning Organization, "Low-Cost Improvements to Bottleneck Locations," December 3, 2015.

<sup>&</sup>lt;sup>6</sup> Seth Asante, MPO staff, "Low-Cost Improvements to Bottleneck Locations," Boston Region Metropolitan Planning Organization, January 2018.

could be corrected in a low-cost manner, but were not selected because of funding resources—these locations would be considered in future bottleneck studies.

Location		MassDOT		
Number	City/Town	District	Express-Highway Section	Problem
			I-93 northbound between Exit 40 (Route 62)	
1	Wilmington	4	and Exit 41 (Route 125)	Merge and diverge
	Quincy and		I-93 southbound at the end of the HOV Zipper	Merge and weave during
2	Braintree	6	lane	the PM commute
			I-93 southbound between Route 16 on-ramp	
3	Medford	4	and Exit 31 (Route 16 off-ramp)	Weave
			I-95 northbound between Exit 37 (I-93) and	
4	Reading	4	Exit 38 (Route 28)	Weave
			I-93 northbound at the end of the HOV Zipper	Merge during the AM
5	Boston	6	lane in Savin Hill	commute
			I-90 westbound and eastbound (just west of	Westbound—diverge;
6	Boston	6	Ted Williams Tunnel Portal)	Eastbound-merge
	Canton and		I-93 northbound between Exit 1 (I-95) and	Merge, diverge, and
7	Randolph	6	Exit 4 (Route 24)	weave
	Canton and		I-93 southbound between Exit 1 (I-95) and	Merge, diverge, and
8	Randolph	6	Exit 4 (Route 24)	weave
			I-90 eastbound in Newton between Exit 16 and	Merge, diverge, and
9	Newton	6	Exit 17	weave

# Table 1Inventory of Express-Highway Locations for Screening

HOV = High occupancy vehicle. MassDOT = Massachusetts Department of Transportation. Note: Shading indicates locations selected for study Source: Central Transportation Planning Staff

MPO staff used the following criteria to screen the bottleneck locations:

- Does the location qualify as a bottleneck? A long traffic queue upstream trailing free-flowing traffic downstream usually characterizes the location as a bottleneck. In addition, the upstream congestion must be recurring in other words, the location experiences routine and predictable congestion because traffic volume exceeds the available capacity at that location.
- Is a physical design constraint or operational conflict that is inherent in the location the cause of the bottleneck? Examples of these may include the following situations:
  - Lane drop—one or more travel lanes are lost, requiring traffic to merge

- Weaving area—drivers must merge across one or more lanes in order to access an entry or exit ramp
- Merge area—on-ramp traffic merges with mainline traffic in order to enter the freeway
- Major interchanges—high-volume traffic is directed from one freeway to another
- Horizontal curves—abrupt changes in highway alignment force drivers to slow down because of safety concerns
- Can the bottleneck be fixed with low-cost operational and geometric improvements? These would exclude costly long-term solutions such as expansion and major transit investments that alter drivers' mode choice. Examples of low-cost operational and geometric improvements may include the following:
  - Using a short section of shoulder as an additional travel lane, an auxiliary lane, or for lengthening an acceleration or deceleration lane
  - Restriping merge and diverge areas to better serve traffic demand
  - Providing better traveler information to allow drivers to respond to temporary changes in lane assignment, such as using a shoulder as an additional travel lane during peak periods
  - Providing all-purpose reversible lanes
  - Changing or adding signs and striping

Based on the screening criteria and consultations with MassDOT Highway Division officials, MPO staff selected locations one and two for study. Below is staff's rationale for not selecting locations three through nine.

#### Locations 3, 4, 5, 6, 7, and 8

These bottleneck locations may be correctible with low-cost improvements but were not selected because of funding. While the work program for this study assumed that "as many as three" locations would be selected, the MPO staff does not propose studying a third location because the two locations are complex and would require considerable resources for evaluating low-cost improvement plans. MPO staff may consider these locations in the next round of bottleneck studies.

#### Location 9

This bottleneck location was screened but was not considered in the selection process because a proposed project would address the bottleneck. MassDOT is

initiating a project to make improvements to traffic signals, signage, and pavement markings on the rotary around Newton Corner in order to improve traffic flow and safety, and to reduce the likelihood of the ramps backing up onto I-90. The project would also look at the feasibility of either restriping, or restriping with minor widening, the eastbound off-ramp in order to facilitate a second lane on the exit. These improvements would have positive impacts on the bottleneck.

#### 3 SELECTED BOTTLENECK LOCATIONS FOR STUDY

# 3.1 Location 1: I-93 Northbound Between Exit 40 (Route 62) and Exit 41 (Route 125) in Wilmington

This segment of highway, about two miles long, with four travel lanes, frequently is congested because of merging and diverging activities, especially during the AM and PM peak periods. In the segment, there are two exit ramps and three entry ramps connecting Routes 62 and 125 to I-93. The ramps are heavily used because of office and industrial parks located off of Route 125. As a result, weekday rush hour congestion at the ramp-arterial junctions and queuing on the exit ramps are not uncommon.

At both exits, the northbound ramps have approximately 1,000 vehicles per hour (vph) exiting I-93 northbound to Routes 62 and 125 during the AM peak period and 1,500 vph during the PM peak period. During the same time periods, the entry ramps from Routes 62 and 125 to I-93 northbound receive about 700 vph during the AM period and 1,300 vph during the PM peak period.

This entering and exiting traffic interacts with about 5,700 vph on the mainline during the AM peak period and 7,600 vph during the PM peak period. The merging and diverging maneuvers in the vicinity creates a bottleneck that backs up traffic on the mainline.

## 3.2 Location 2: I-93 Southbound at the End of the High Occupancy Vehicle (HOV) Zipper Lane in Quincy and Braintree

This bottleneck is located on I-93 southbound at the end of the Zipper lane, where traffic diverges, merges, and weaves in order to continue onto I-93 southbound or Route 3 southbound. The bottleneck occurs only during PM peak periods when the southbound HOV lane is in operation.

At the bottleneck, traffic from six lanes (four on the mainline, one on the temporary HOV lane, and one from high-volume entry ramp from Furnace Brook Parkway) is forced onto four travel lanes in a short segment, about 0.5 miles long. The reduction in number of lanes dramatically reduces capacity in the segment creating a bottleneck. In addition, a significant amount of lane-changing maneuvers (weaving and diverging) and merging take place within the segment

to disperse traffic to continue on I-93 southbound or head to Route 3 southbound.

During the PM peak period, the entry ramp from Furnace Brook Parkway carries about 800 vph, and upstream of the bottleneck, the mainline and HOV lane carry 5,500 vph and 700 vph, respectively. Consequently, the traffic demand at the bottleneck greatly exceeds the capacity at the bottleneck. As a result, there are long traffic queues on the mainline and in the HOV lane, which extend five miles to Columbia Avenue in Dorchester.

#### 4 SUMMARY

By identifying and evaluating a comprehensive list of potential improvements at the two locations, MPO staff will rely on their technical expertise and judgment regarding the nature of bottlenecks. In addition, MPO staff will seek input from MassDOT Highway Division staff that are familiar with the operations of the region's express-highway system.

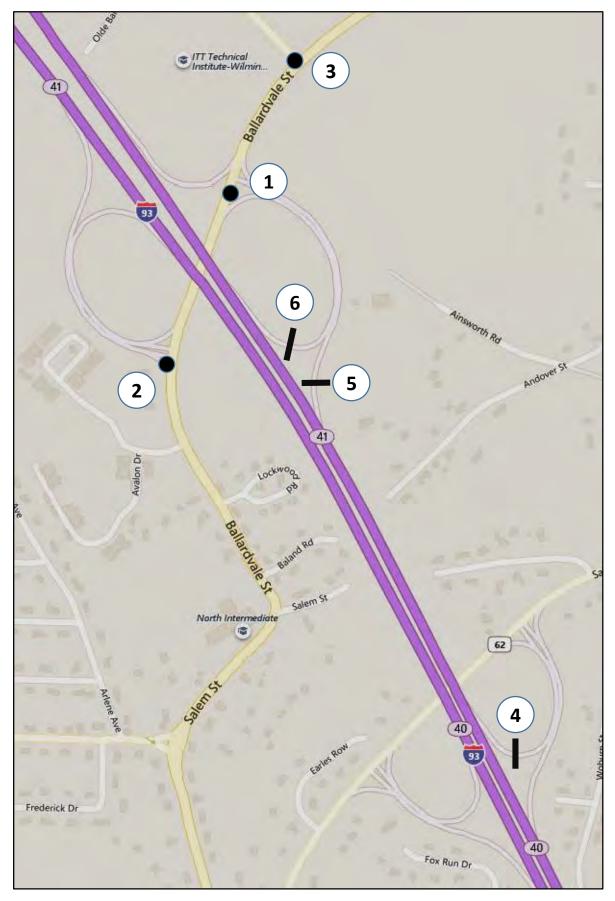
This study addresses the MPO's goal of increasing safety on the region's highway system, capacity management and mobility, and system preservation. MPO staff will submit this proposal to the MPO for discussion. If the MPO approves this selection, staff will meet with officials from MassDOT and discuss the study specifics, conduct field visits, collect data, and perform various analyses.

SA/sa

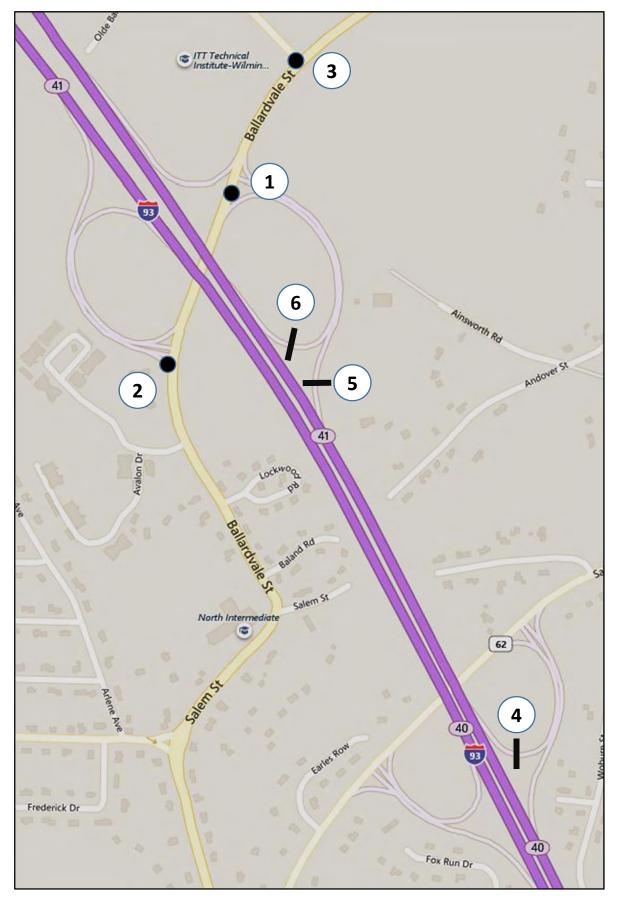
# APPENDIX B

ATR data
 Classification data

# I-93 Northbound between Exit 40 (Route 62) and Exit 41 (Route 125)



# I-93 Northbound between Exit 40 (Route 62) and Exit 41 (Route 125)



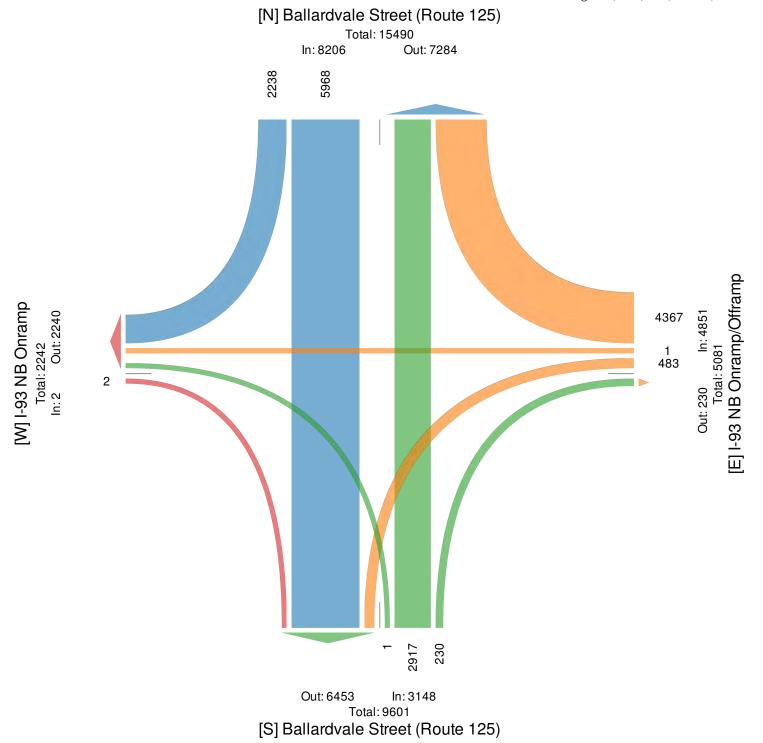
Wed Jan 16, 2019 Full Length (6 AM-9 AM, 3 PM-6 PM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609037, Location: 42.585518, -71.156976

Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

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6:45AM		204	0	0		0 152	0		0	155	0	11	162	0 0	173	0	0		0 0		0	561
Hourly Total	107	689	0	0		0 434	1		0	443	0	37	543	0 0	580	0	0		0 0		0	1819
7:00AM	39	208	0	0		0 170	0		0	170	0	11	177	0 0	188	0	0		0 0	-	0	605
7:15AM	-	246	0	0		0 167	0		0	168	0	11	196	0 0	210	0	0		0 0		0	665
7:30AM	60	245	0	0		0 203	0		0	210	0	17	220	0 0	237	0	0		0 0		0	752
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4:15PM	122	281	0	0	403	0 179	0	41	0	220	0	6	80	0 0	86	0	0	0	0 0	) 0	0	709
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Wed Jan 16, 2019 Full Length (6 AM-9 AM, 3 PM-6 PM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609037, Location: 42.585518, -71.156976

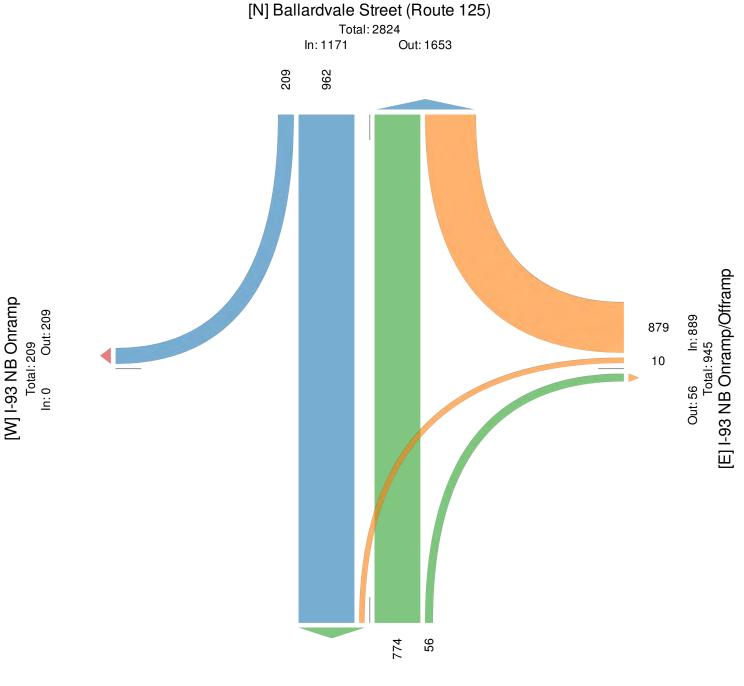


#### TM-1 Ballardvale Street (Route 125) @ I-93 N... - TMC Wed Jan 16, 2019 AM Peak (7:30 AM - 8:30 AM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609037, Location: 42.585518, -71.156976

#### Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

Leg	Ballard	vale Str	eet	Rou	te 125)		I-93 NB	On	ramp/Of	frar	np		Ballard	vale Str	eet	(Rot	te 125)		I-93	NB	Onr	amp			
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7:45AM	42	246	0	0	288	0	242	0	3	0	245	0	15	195	0	0	210	0	0	0	0	0	0	0	743
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8:15AM	46	236	0	0	282	0	222	0	0	0	222	0	9	170	0	0	179	0	0	0	0	0	0	0	683
Total	209	962	0	0	1171	0	879	0	10	0	889	0	56	774	0	0	830	0	0	0	0	0	0	0	2890
% Approach	17.8%	82.2%	0%	0%	-	-	98.9%	0%	1.1% (	)%	-	-	6.7%	93.3%	0%	0%	-	-	0%	0%	0%	0%	-	-	-
% Total	7.2%	33.3%	0%	0%	40.5%	-	30.4%	0%	0.3% (	)%	30.8%	-	1.9%	26.8%	0%	0%	28.7%	-	0%	0%	0%	0%	0%	-	-
PHF	0.857	0.978	-	-	0.960	-	0.908	-	0.357	-	0.907	-	0.824	0.880	-	-	0.876	-	-	-	-	-	-	-	0.961
Motorcycles	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0
% Motorcycles	0%	0%	0%	0%	0%	-	0%	0%	0% (	)%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	-	-	0%
Lights	165	885	0	0	1050	-	828	0	9	0	837	-	55	735	0	0	790	-	0	0	0	0	0	-	2677
% Lights	78.9%	92.0%	0%	0%	89.7%	-	94.2%	0%	90.0% (	)%	94.2%	-	98.2%	95.0%	0%	0%	95.2%	-	0%	0%	0%	0%	-	-	92.6%
Single-Unit Trucks	25	65	0	0	90	-	38	0	0	0	38	-	1	21	0	0	22	-	0	0	0	0	0	-	150
% Single-Unit Trucks	12.0%	6.8%	0%	0%	7.7%	-	4.3%	0%	0% (	)%	4.3%	-	1.8%	2.7%	0%	0%	2.7%	-	0%	0%	0%	0%	-	-	5.2%
Articulated Trucks	19	11	0	0	30	-	13	0	1	0	14	-	0	18	0	0	18	-	0	0	0	0	0	-	62
% Articulated Trucks	9.1%	1.1%	0%	0%	2.6%	-	1.5%	0%	10.0% (	)%	1.6%	-	0%	2.3%	0%	0%	2.2%	-	0%	0%	0%	0%	-	-	2.1%
Buses	0	1	0	0	1	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	1
% Buses	0%	0.1%	0%	0%	0.1%	-	0%	0%	0% (	)%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	-	-	0%
Bicycles on Road	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0
% Bicycles on Road	0%	0%	0%	0%	0%	-	0%	0%	0% (	)%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	-	-	0%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Wed Jan 16, 2019 AM Peak (7:30 AM - 8:30 AM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609037, Location: 42.585518, -71.156976



Out: 972 In: 830 Total: 1802 [S] Ballardvale Street (Route 125)

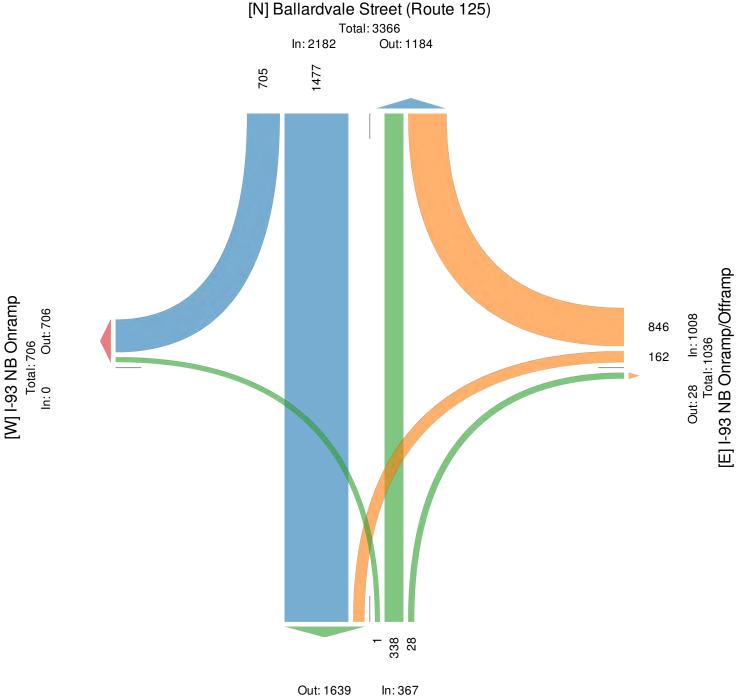
Wed Jan 16, 2019 PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609037, Location: 42.585518, -71.156976

Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

· · · · · · · · · · · · · · · · · · ·							1											1						
0			eet	(Rou	te 125)		I-93 NB			ffrai	пp				reet (Rout	te 125)			B NB		amp		I	1
Direction	Southb	ound					Westbo	und					Northb	oound				Eas	tbou	nd				
Time	R	Т	L	U	App I	Ped*	R	Т	L	U	Арр	Ped*	R	Т	L U	Арр	Ped*	F	τ	L	U.	App P	ed*	Int
2019-01-16 4:30PM	184	326	0	0	510	0	215	0	38	0	253	0	9	99	0 0	108	0	0	0	0	0	0	0	871
4:45PM	155	338	0	0	493	0	216	0	37	0	253	0	2	80	0 0	82	0	0	0	0	0	0	0	828
5:00PM	210	402	0	0	612	0	192	0	44	0	236	0	11	70	0 0	81	0	0	0	0	0	0	0	929
5:15PM	156	411	0	0	567	0	223	0	43	0	266	0	6	89	1 0	96	0	0	0	0	0	0	0	929
Total	705	1477	0	0	2182	0	846	0	162	0	1008	0	28	338	1 0	367	0	0	0	0	0	0	0	3557
% Approach	32.3%	67.7%	0%	0%	-	-	83.9%	0%	16.1%	0%	-	-	7.6%	92.1%	0.3% 0%			0%	0%	0%	0%	-	-	-
% Total	19.8%	41.5%	0%	0%	61.3%	-	23.8%	0%	4.6%	0%	28.3%	-	0.8%	9.5%	0% 0%	10.3%	) -	0%	0%	0%	0%	0%	-	-
PHF	0.839	0.898	-	-	0.891	-	0.948	-	0.920	-	0.947	-	0.636	0.854	0.250 -	0.850	) –			-	-	-	-	0.957
Mo to rc yc le s	0	0	0	0	0	-	1	0	0	0	1	-	0	0	0 0	0	) –	0	0	0	0	0	-	1
% Motorcycles	0%	0%	0%	0%	0%	-	0.1%	0%	0%	0%	0.1%	-	0%	0%	0% 0%	0 %	, -	0%	0%	0%	0%	-	-	0%
Lights	691	1461	0	0	2152	-	813	0	161	0	974	-	28	319	1 0	348	- 1	0	0	0	0	0	-	3474
% Lights	98.0%	98.9%	0%	0%	98.6%	-	96.1%	0%	99.4%	0%	96.6%	-	100%	94.4%	100% 0%	94.8%	) -	0%	0%	0%	0%	-	-	97.7%
Single-Unit Trucks	7	8	0	0	15	-	27	0	1	0	28	-	0	13	0 0	13	-	0	0	0	0	0	-	56
% Single-Unit Trucks	1.0%	0.5%	0%	0%	0.7%	-	3.2%	0%	0.6%	0%	2.8%	-	0%	3.8%	0% 0%	3.5%	) -	0%	0%	0%	0%	-	-	1.6%
Artic ulate d Truc ks	7	7	0	0	14	-	5	0	0	0	5	-	0	5	0 0	5		0	0	0	0	0	-	24
% Articulated Trucks	1.0%	0.5%	0%	0%	0.6%	-	0.6%	0%	0%	0%	0.5%	-	0%	1.5%	0% 0%	1.4 %	) –	0%	0%	0%	0%	-	-	0.7%
Buses	0	1	0	0	1	-	0	0	0	0	0	-	0	1	0 0	1	L -	0	0	0	0	0	-	2
% Buses	0%	0.1%	0%	0%	0%	-	0%	0%	0%	0%	0%	-	0%	0.3%	0% 0%	0.3%	) -	0%	0%	0%	0%	-	-	0.1%
Bicycles on Road	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0 0	0	) –	0	0	0	0	0	-	0
% Bicycles on Road	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	0%	-	0%	0%	0% 0%	0%	) –	0%	0%	0%	0%	-	-	0%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-			- 0			-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-						-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-			- 0			-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-						-	-	-	-	-



Wed Jan 16, 2019 PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609037, Location: 42.585518, -71.156976



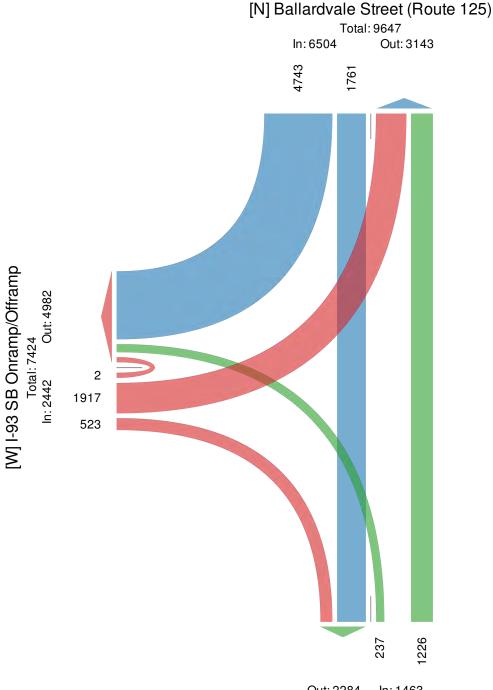
Total: 2006 [S] Ballardvale Street (Route 125)

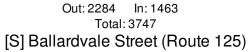
Wed Jan 16, 2019 Full Length (6 AM-9 AM, 3 PM-6 PM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609038, Location: 42.582744, -71.158304

#### Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

Leg	Ballardv	ale Stree	t (Ro	ute 125	)	Ballardv	ale Stree	et (Ro	oute 125	)	I-93 SB (	Onramp/0	Offramp	)		
Direction	Southbo	und				Northbo	und				Eastbour	nd				
Time	R	Т	U	Арр	Ped*	Т	L	U	Арр	Ped*	R	L	U	Арр	Ped*	Int
2019-01-16 6:00AN	1 137	12	0	14 9	0	23	9	0	32	0	17	88	0	105	0	286
6:15AN	1 150	17	0	167	0	45	7	0	52	0	21	95	0	116	0	335
6:30AN	1 158	18	0	176	0	45	10	0	55	0	16	113	0	129	0	360
6:45AN	1 152	27	0	179	0	51	19	0	70	0	19	130	0	149	0	398
Hourly Tota	l 597	74	0	671	0	164	45	0	209	0	73	426	0	499	0	1379
7:00A	4 201	19	0	220	0	58	13	0	71	0	14	142	0	156	0	447
7:15A	4 218	31	0	249	0	85	12	0	97	0	18	123	0	14 1	0	487
7:30AN	4 207	33	0	240	0	101	24	0	125	0	16	139	0	155	0	520
7:45AN	4 217	44	0	261	0	81	27	0	108	0	15	128	0	143	0	512
Hourly Tota	1 843	127	0	970	0	325	76	0	401	0	63	532	0	595	0	1966
8:00A	1 204	40	0	244	0	65	22	0	87	0	6	123	0	129	0	460
8:15A	4 191	44	0	235	0	59	16	0	75	0	13	131	0	144	0	454
8:30A	4 183	50	0	233	0	48	11	0	59	0	24	117	0	141	0	433
8:45A	4 198	43	0	241	0	55	5	0	60	0	21	125	0	146	0	447
Hourly Tota	1 776	177	0	953	0	227	54	0	281	0	64	496	0	560	0	1794
3:00PM	1 153	86	0	239	0	47	1	0	48	0	14	41	0	55	0	342
3:15PM	1 149	86	0	235	0	38	4	0	42	0	22	44	0	66	0	343
3:30PM	1 174	95	0	269	0	29	8	0	37	0	37	30	0	67	0	373
3:45PI	1 157	90	0	247	0	38	4	0	42	0	23	40	0	63	0	352
Hourly Tota	1 633	357	0	990	0	152	17	0	169	0	96	155	0	251	0	14 10
4:00PM	_	86	0	284	0	30	2	0	32	0	32	41	0	73	0	389
4:15PM	1 207	124	0	331	0	48	7	0	55	0	26	44	0	70	0	456
4:30PM	-	147	0	367	0	51	7	0	58	0	36	57	1	94	0	519
4:45PM		128	0	396	0		7	0	48	0	29	45	0	74	0	518
Hourly Tota	1 893	485	0	1378	0	170	23	0	193	0	123	187	1	311	0	1882
5:00PI	_	171	0	438	0	42	7	0	49	0	21	28	0	49	0	
5:15PM		122	0	449	0		7	0	61	0	28	46	0	74	0	
5:30PM		157	0	386	0		5	0	63	0	29	18	1	48	0	
5:45PM	-	91	0	269	0		3	0	37	0	26	29	0	55	0	
Hourly Tota		541	0	1542	0		22	0	210	0	104	121	1	226	0	
Tota		1761	0	6504	0		237	0	1463	0	523	1917	2	2442	0	
% Approac	-	27.1%	0%			83.8%	16.2%	0%	-	0	21.4%	78.5%	0.1%	2442	0	10403
% Tota			0%	62.5%		11.8%	2.3%		14.1%		5.0%	18.4%	0.170	23.5%		
Motorcycle		0	0 / 0	02.5 /0		0	0	0 /0	0		0	0	0 /0	20.0 /0		0
% Motorcycle		0%		0%		0%		0%	0%		0%	0%	0%	0%		0%
Light		1726	0 /0	6199		1207	234	0 /0	1441		516	1744	2	2262		9902
% Light		98.0%		95.3%		98.5%			98.5%		98.7%	91.0%	100%	92.6%		95.1%
Single-Unit Truck		27	0 /0	227		17	30.770		20		6	85	0	91		338
% Single-Unit Truck		1.5%		3.5%		1.4%	1.3%		1.4 %		1.1%	4.4%	0%	3.7%		3.2%
Articulated Truck		2	0 /0	67		1.470	1.5 /0	0 /0	1.4 /0		0	86	0 /0	86		154
% Articulated Truck	_	0.1%		1.0%	_	0.1%		0%	0.1%	_	0%	4.5%	0%	3.5%	_	1.5%
Buse		6	0 /0	1.0 70	_	0.170	0 /0	0 /0	1	_	1	4.370	0 /0	3.570	_	1.5 %
Buse % Buse	_	0.3%		0.2%	-	0.1%		0%	0.1%	-	0.2%	0.1%	0%	0.1%	-	0.1%
Bicycles on Roa		0.3%	0%	0.2%	-	0.1%	0%		0.1%	-	0.2%	0.1%	0%	0.1%	-	0.1%
% Bicycles on Roa		0%		0%		0%		0%	0%	-	0%	0%	0%	0%		0%
% BICycles on Roa Pedestrian					0					- 0	- 0%	- 0%	- 0%	- 0%	0	
							-			-						
% Pedestrian						-	-				-	-	-			
-	_		-	-	0					0						
Bicycles on Crosswal % Bicycles on Crosswal	k -	-	-	-	-	- -	-	-	-	0	-	-	-	-		

Wed Jan 16, 2019 Full Length (6 AM-9 AM, 3 PM-6 PM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609038, Location: 42.582744, -71.158304



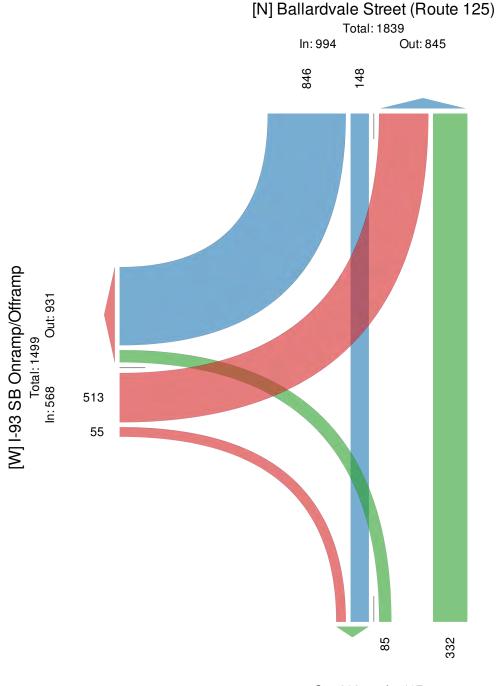


Wed Jan 16, 2019 AM Peak (7:15 AM - 8:15 AM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609038, Location: 42.582744, -71.158304

#### Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

Leg	Ballardva	ale Stree	t (Ro	ute 125)	)	Ballardva	ale Stree	t (Ro	ute 125)		I-93 SB (	Onramp/	Offrai	np		
Direction	Southbo	und				Northboy	ınd				Eastbour	nd				
Time	R	Т	U	Арр	Ped*	Т	L	U	Арр	Ped*	R	L	U	Арр	Ped*	Int
2019-01-16 7:15AM	218	31	0	249	0	85	12	0	97	0	18	123	0	14 1	0	487
7:30AM	207	33	0	240	0	101	24	0	125	0	16	139	0	155	0	520
7:45AM	217	44	0	261	0	81	27	0	108	0	15	128	0	143	0	512
8:00AM	204	40	0	244	0	65	22	0	87	0	6	123	0	129	0	460
Total	846	148	0	994	0	332	85	0	4 17	0	55	513	0	568	0	1979
% Approach	85.1%	14.9%	0%	-	-	79.6%	20.4%	0%	-	-	9.7%	90.3%	0%	-	-	
% Total	42.7%	7.5%	0%	50.2%	-	16.8%	4.3%	0%	21.1%	-	2.8%	25.9%	0%	28.7%	-	
PHF	0.970	0.841	-	0.952	-	0.822	0.787	-	0.834	-	0.764	0.923	-	0.916	-	0.951
Motorcycles	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% Motorcycles	0%	0%	0%	0%	-	0%	0%	0%	0 %	-	0%	0%	0%	0 %	-	0%
Lights	782	139	0	921	-	327	85	0	4 12	-	53	483	0	536	-	1869
% Lights	92.4%	93.9%	0%	92.7%	-	98.5%	100%	0%	98.8%	-	96.4%	94.2%	0%	94.4%	-	94.4%
Single -Unit Trucks	53	7	0	60	-	4	0	0	4	-	2	15	0	17	-	81
% Single-Unit Trucks	6.3%	4.7%	0%	6.0%	-	1.2%	0%	0%	1.0 %	-	3.6%	2.9%	0%	3.0%	-	4.1%
Articulated Trucks	10	2	0	12	-	1	0	0	1	-	0	15	0	15	-	28
% Articulated Trucks	1.2%	1.4%	0%	1.2%	-	0.3%	0%	0%	0.2%	-	0%	2.9%	0%	2.6%	-	1.4%
Buses	1	0	0	1	-	0	0	0	0	-	0	0	0	0	-	1
% Buses	0.1%	0%	0%	0.1%	-	0%	0%	0%	0 %	-	0%	0%	0%	0 %	-	0.1%
Bicycles on Road	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% Bicycles on Road	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	

Wed Jan 16, 2019 AM Peak (7:15 AM - 8:15 AM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609038, Location: 42.582744, -71.158304





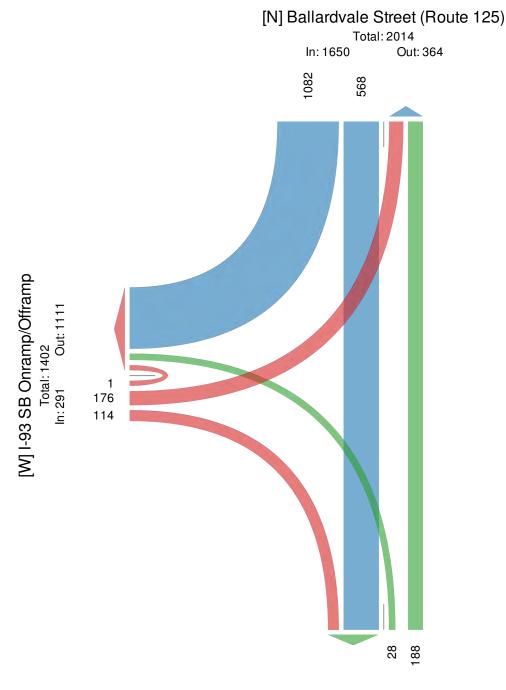
Wed Jan 16, 2019 PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609038, Location: 42.582744, -71.158304

#### Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

Leg	Ballardv	ale Stree	et (Ro	ute 125)	)	Ballardv	ale Stre	et (Ro	ute 125	)	I-93 SB (	) Onramp/	Offramp			
Direction	Southbo	und				Northbo	und				Eastboui	nd				
Time	R	Т	U	Арр	Ped*	Т	L	U	Арр	Ped*	R	L	U	Арр	Ped*	Int
2019-01-16 4:30PM	220	147	0	367	0	51	7	0	58	0	36	57	1	94	0	519
4:45PM	268	128	0	396	0	41	7	0	48	0	29	45	0	74	0	518
5:00PM	267	171	0	438	0	42	7	0	49	0	21	28	0	49	0	536
5:15PM	327	122	0	449	0	54	7	0	61	0	28	46	0	74	0	584
Total	1082	568	0	1650	0	188	28	0	216	0	114	176	1	291	0	2157
% Approach	65.6%	34.4%	0%	-	-	87.0%	13.0%	0%	-	-	39.2%	60.5%	0.3%	-	-	-
% Total	50.2%	26.3%	0%	76.5%	-	8.7%	1.3%	0%	10.0%	-	5.3%	8.2%	0%	13.5%	-	-
PHF	0.827	0.830	-	0.919	-	0.870	1.000	-	0.885	-	0.792	0.772	0.250	0.774	-	0.923
Motorcycles	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% Motorcycles	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%
Lights	1071	565	0	1636	-	186	28	0	214	-	114	159	1	274	-	2124
% Lights	99.0%	99.5%	0%	99.2%	-	98.9%	100%	0%	99.1%	-	100%	90.3%	100%	94.2%	-	98.5%
Single -Unit Trucks	3	2	0	5	-	2	0	0	2	-	0	13	0	13	-	20
% Single -Unit Trucks	0.3%	0.4%	0%	0.3%	-	1.1%	0%	0%	0.9%	-	0%	7.4%	0%	4.5%	-	0.9%
Artic ulate d Truc ks	7	0	0	7	-	0	0	0	0	-	0	3	0	3	-	10
% Artic ulate d Truc ks	0.6%	0%	0%	0.4%	-	0%	0%	0%	0%	-	0%	1.7%	0%	1.0 %	-	0.5%
Buses	1	1	0	2	-	0	0	0	0	-	0	1	0	1	-	3
% Buses	0.1%	0.2%	0%	0.1%	-	0%	0%	0%	0 %	-	0%	0.6%	0%	0.3%	-	0.1%
Bicycles on Road	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% Bicycles on Road	0%	0%	0%	0 %	-	0%	0%	0%	0 %	-	0%	0%	0%	0 %	-	0%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Wed Jan 16, 2019 PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609038, Location: 42.582744, -71.158304

Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US



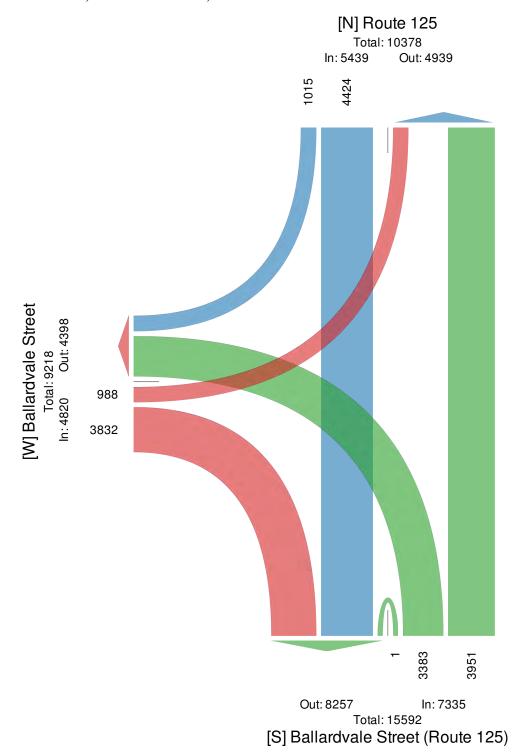
Out: 682 In: 216 Total: 898 [S] Ballardvale Street (Route 125)

Wed Jan 16, 2019 Full Length (6 AM-9 AM, 3 PM-6 PM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609040, Location: 42.587497, -71.155562

Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

Leg	Route 12	25				Ballardv	ale Stree	t (Rout	e 125)		Ballardv	ale Stree	e t			
Direction	Southbo					Northbo		. (	,		Eastboui					
Гime	R		U	Арр	Ped*	Т	L	U	Арр	Ped*	R	L	U	Арр	Ped*	Int
2019-01-16 6:00/		128	0	145	0	52	115	0	167	0	41	9	0	50	0	
6:15/	M 30	151	0	181	0	91	138	0	229	0	46	9	0	55	0	46
6:30/	M 39	144	0	183	0	98	180	0	278	0	47	8	0	55	0	
6:45/	M 40	185	0	225	0	118	194	0	312	0	43	7	0	50	0	
Hourly To	tal 126	608	0	734	0	359	627	0	986	0	177	33	0	210	0	193
7:004		189	0	229	0	135	203	0	338	0	55	13	0	68	0	
7:15/	M 70	235	0	305	0	168	195	0	363	0	65	17	0	82	0	75
7:30/		223	0	301	0	169	250	0	4 19	0	80	16	0	96	0	81
7:45/	M 88	230	0	318	0	161	314	0	475	0	66	13	0	79	0	87
Hourly To		877	0	1153	0	633	962	0	1595	0	266	59	0	325	0	307
8:004		209	0	301	0	130	261	1	392	0	87	12	0	99	0	79
8:15/		214	0	321	0	122	265	0	387	0	77	18	0	95	0	80
8:30/		179	0	263	0	118	249	0	367	0	79	17	0	96	0	72
8:45/		213	0	315	0	112	263	0	375	0	73	17	0	90	0	78
Hourly To		815	0	1200	0	482	1038	1	1521	0	316	64	0	380	0	
3:00		151	0	175	0	160	75	0	235	0	177	42	0	219	0	
3:15]	_		0	159	0	152	77	0	229	0	170	47	0	217	0	<u> </u>
3:30		181	0	206	0	169	69	0	238	0	220	53	0	273	0	71
3:45		131	0	151	0	200	58	0	258	0	187	45	0	232	0	64
Hourly To		601	0	691	0	681	279	0	960	0	754	187	0	941	0	
4:00		169	0	183	0	184	58	0	242	0	267	86	0	353	0	77
4:15		164	0	186	0	205	63	0	268	0	224	62	0	286	0	74
4:30		198	0	217	0	205	74	0	300	0	292	84	0	376	0	89
4:45		213	0	235	0	243	68	0	311	0	281	75	0	356	0	90
Hourly To		744	0	821	0	858	263	0	1121	0	1064	307	0	1371	0	331
5:00		222	0	240	0	213	52	0	265	0	405	117	0	522	0	
5:15		205	0	240	0	213	54	0	308	0	365	98	0	463	0	
5:30			0	210	0	242	49	0	291	0	284	75	0	359	0	
5:45	_		0	158	0	242	59	0	288	0	204	48	0	249	0	<u> </u>
Hourly To	_	779	0	840	0	938	214	0	1152	0	1255	338	0	1593	0	358
													-		_	
To	_	4424	0	5439	0	3951	3383	1 0%	7335	0	3832	988	0	4820	0	1759
% Approa % To		81.3%		- 30.9%	-	53.9%	46.1%		-	-	79.5%	20.5%		-	-	
			0%		-	22.5%	19.2%	0%	41.7%	-	21.8%	5.6%	0%	27.4%	-	
Motorcyc		0%	0	0%	-	0%	0%	0	0%	-	0	0	0	0%	-	09
% Motorcyc					-			0%		-	0%		0%		-	
		4262	0	5230	-	3798	3069	1000/	6868	-	3529	945	0	4474	-	1657
% Lig		96.3%		96.2%	-	96.1%	90.7%		93.6%	-	92.1%	95.6%		92.8%	-	94.29
Single-Unit Truc		108	0	142	-	114	185	0	299	-	200	31		231	-	67
% Single-Unit Truc Articulated Truc		2.4%		2.6%	-	2.9%	5.5% 129	0%	4.1%	-	5.2%	3.1%		4.8%	-	3.8%
		43	0		-			0		-	103	11		114	-	33
% Articulated True		1.0%		1.0%	-	0.8%	3.8%	0%	2.2%	-	2.7%	1.1%		2.4 %	-	1.9%
Bus 9/ Bus		11	0	11	-	6	0	0	6	-	0	0		0	-	1
% Bus		0.2%		0.2%	-	0.2%	0%	0%	0.1%	-	0%		0%	0%	-	0.19
Bicycles on Ro			0	1	-	0	0	0	0	-	0	1		1	-	
% Bicycles on Ro		0%		0%	-	0%	0%	0%	0 %	-	0%	0.1%		0%	-	09
Pe de stria				-		-	-	-	-	0	-	-		-	-	
% Pedestria		-		-		-	-	-	-	-	-	-		-		
Bicycles on Crossw		-	-	-	0	-	-	-	-	0	-	-		-	0	<u> </u>
% Bicycles on Crossw	alk -	-	-	-	-	-	-	-	-	-	-	-	-	-	-	

Wed Jan 16, 2019 Full Length (6 AM-9 AM, 3 PM-6 PM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609040, Location: 42.587497, -71.155562



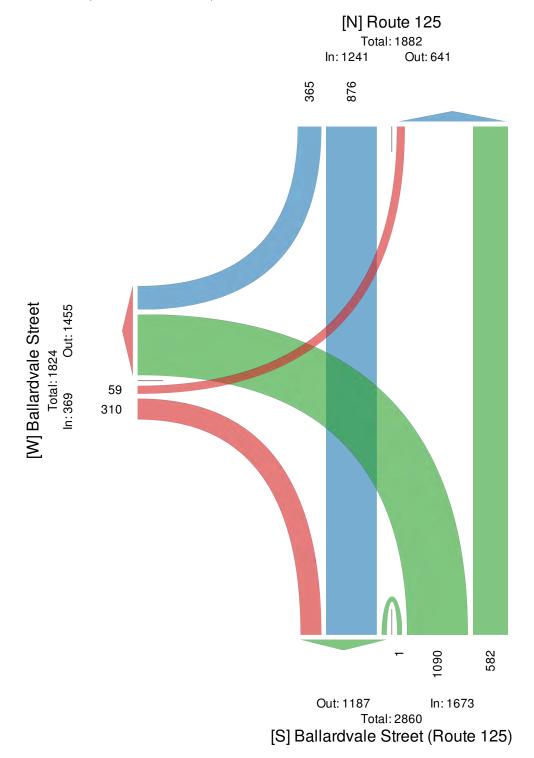
Wed Jan 16, 2019 AM Peak (7:30 AM - 8:30 AM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609040, Location: 42.587497, -71.155562

#### Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

10.

Leg	Route 12	5				Ballardv	ale Stree	et (Rout	e 125)		Ballardva	ale Stree	t			
Direction	Southbo	und				Northbo	und				Eastbour	nd				
Time	R	Т	U	Арр	Ped*	Т	L	U	Арр	Ped*	R	L	U	Арр	Ped*	Int
2019-01-16 7:30AM	78	223	0	301	0	169	250	0	4 19	0	80	16	0	96	0	816
7:45AM	88	230	0	318	0	161	314	0	475	0	66	13	0	79	0	872
8:00AM	92	209	0	301	0	130	261	1	392	0	87	12	0	99	0	792
8:15AM	107	214	0	321	0	122	265	0	387	0	77	18	0	95	0	803
Total	365	876	0	1241	0	582	1090	1	1673	0	310	59	0	369	0	3283
% Approach	29.4%	70.6%	0%	-	-	34.8%	65.2%	0.1%	-	-	84.0%	16.0%	0%	-	-	-
% Total	11.1%	26.7%	0%	37.8%	-	17.7%	33.2%	0%	51.0%	-	9.4%	1.8%	0%	11.2%	-	-
PHF	0.850	0.952	-	0.966	-	0.861	0.868	0.250	0.881	-	0.891	0.819	-	0.932	-	0.941
Mo to r c y c le s	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% Motorcycles	0%	0%	0%	0%	-	0%	0%	0%	0 %	-	0%	0%	0%	0 %	-	0%
Lights	349	833	0	1182	-	550	1029	1	1580	-	230	47	0	277	-	3039
% Lights	95.6%	95.1%	0%	95.2%	-	94.5%	94.4%	100%	94.4%	-	74.2%	79.7%	0%	75.1%	-	92.6%
Single -Unit Trucks	12	31	0	43	-	26	36	0	62	-	64	10	0	74	-	179
% Single -Unit Trucks	3.3%	3.5%	0%	3.5%	-	4.5%	3.3%	0%	3.7%	-	20.6%	16.9%	0%	20.1%	-	5.5%
Artic ulate d Trucks	3	10	0	13	-	6	25	0	31	-	16	2	0	18	-	62
% Artic ulate d Truc ks	0.8%	1.1%	0%	1.0 %	-	1.0%	2.3%	0%	1.9%	-	5.2%	3.4%	0%	4.9%	-	1.9%
Buses	0	2	0	2	-	0	0	0	0	-	0	0	0	0	-	2
% Buses	0%	0.2%	0%	0.2%	-	0%	0%	0%	0 %	-	0%	0%	0%	0%	-	0.1%
Bicycles on Road	1	0	0	1	-	0	0	0	0	-	0	0	0	0	-	1
% Bicycles on Road	0.3%	0%	0%	0.1%	-	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Wed Jan 16, 2019 AM Peak (7:30 AM - 8:30 AM) All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609040, Location: 42.587497, -71.155562



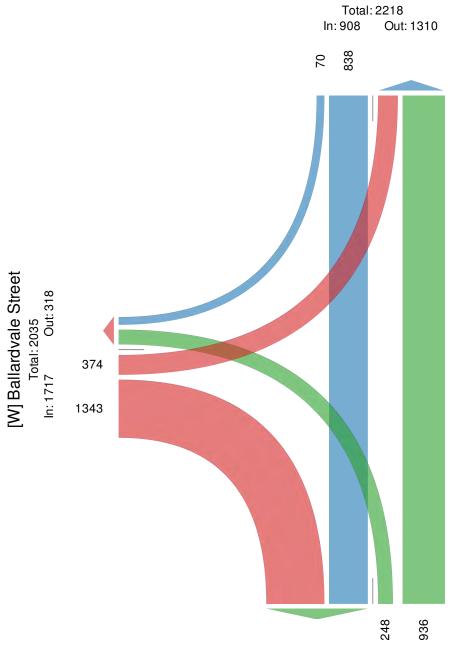
Wed Jan 16, 2019 PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609040, Location: 42.587497, -71.155562

#### Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US

Leg	Route 12	25				Ballardva	ale Stree	t (Ro	ute 125)		Ballardva	ale Stree	t			
Direction	Southbo	und				Northbo	und				Eastbour	nd				
Time	R	Т	U	Арр	Ped*	Т	L	U	Арр	Ped*	R	L	U	Арр	Ped*	Int
2019-01-16 4:30PM	19	198	0	217	0	226	74	0	300	0	292	84	0	376	0	893
4:45PM	22	213	0	235	0	243	68	0	311	0	281	75	0	356	0	902
5:00PM	18	222	0	240	0	213	52	0	265	0	405	117	0	522	0	1027
5:15PM	11	205	0	216	0	254	54	0	308	0	365	98	0	463	0	987
Total	70	838	0	908	0	936	248	0	1184	0	1343	374	0	1717	0	3809
% Approach	7.7%	92.3%	0%	-	-	79.1%	20.9%	0%	-	-	78.2%	21.8%	0%	-	-	
% Total	1.8%	22.0%	0%	23.8%	-	24.6%	6.5%	0%	31.1%	-	35.3%	9.8%	0%	45.1%	-	
PHF	0.795	0.944	-	0.946	-	0.921	0.838	-	0.952	-	0.829	0.804	-	0.823	-	0.928
Motorcycles	0	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0
% Motorcycles	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%	0%	0%	0 %	-	0%
Lights	64	827	0	891	-	920	209	0	1129	-	1327	368	0	1695	-	3715
% Lights	91.4%	98.7%	0%	98.1%	-	98.3%	84.3%	0%	95.4%	-	98.8%	98.4%	0%	98.7%	-	97.5%
Single -Unit Trucks	4	4	0	8	-	9	34	0	43	-	8	4	0	12	-	63
% Single-Unit Trucks	5.7%	0.5%	0%	0.9%	-	1.0%	13.7%	0%	3.6%	-	0.6%	1.1%	0%	0.7%	-	1.7%
Artic ulate d Truc ks	2	6	0	8	-	5	5	0	10	-	8	1	0	9	-	27
% Articulated Trucks	2.9%	0.7%	0%	0.9%	-	0.5%	2.0%	0%	0.8%	-	0.6%	0.3%	0%	0.5%	-	0.7%
Buses	0	1	0	1	-	2	0	0	2	-	0	0	0	0	-	3
% Buses	0%	0.1%	0%	0.1%	-	0.2%	0%	0%	0.2%	-	0%	0%	0%	0 %	-	0.1%
Bicycles on Road	0	0	0	0	-	0	0	0	0	-	0	1	0	1	-	1
% Bicycles on Road	0%	0%	0%	0%	-	0%	0%	0%	0%	-	0%	0.3%	0%	0.1%	-	0%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
Bicycles on Crosswalk	. –	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	. –	-	-	-	-	-	-	-	-	-	-	-	-	-	-	

Wed Jan 16, 2019 PM Peak (4:30 PM - 5:30 PM) - Overall Peak Hour All Classes (Motorcycles, Lights, Single-Unit Trucks, Articulated Trucks, Buses, Pedestrians, Bicycles on Road, Bicycles on Crosswalk) All Movements ID: 609040, Location: 42.587497, -71.155562

Provided by: Precision Data Industries, LLC (PDI) 46 Morton Street, Framingham, MA, MA, 01702, US



Out: 2181 In: 1184 Total: 3365 [S] Ballardvale Street (Route 125)

[N] Route 125

STA.4

# **Massachusetts Highway Department** S18-055-342-01 Weekly Volume Report - Mon 01/14/2019 - Sun 01/20/2019

Location ID:	S18-055-342-0	D1			Type:	SPOT	
Located On:	ON-RAMP FRC	OM RTE. 62 TC	1-93NB		1		
Direction	RAMP						
Community:	WILMINGTON				Period:	Mon 01/14/2	019 - Sun 01
AADT:							
Start Time	Mon	Tue	Wed	Thu	Fri	Sat	Sun
12:00 AM			18	18	16		T and
1:00 AM		· · · · · · · · · · · · · · · · · · ·	14	11	11	H. Press Press Press	19.5.°-
2:00 AM			12	16	24		
3:00 AM		State She was	14	18	14		1.1.7
4:00 AM			32	44	36		
5:00 AM	P F a		103	105	106	W The set of the set	Takan and a
6:00 AM		G	263	276	215		
7:00 AM			470	408	360	N. M. C.	19-10
8:00 AM			431	415	368		
9:00 AM		www.	328	334	291		-
10:00 AM		228	220	206			
11:00 AM		216	234	239			
12:00 PM		247	268	273			
1:00 PM	Caster and	290	303	283		The second second	
2:00 PM		314	334	302			
3:00 PM		378	331	337	all a supervile		
4:00 PM		321	346	283			
5:00 PM		300	320	258		al class of	1
6:00 PM		204	244	202			
7:00 PM		120	139	168		17.2.90	
8:00 PM		140	133	151			
9:00 PM	a de la compañía de la	80	76	97			Salar -
10:00 PM		68	71	97			
11:00 PM	an a	39	40	41			
Total	0	2945	4744	4582	1441	0	0
24HrTotal	· · · · · · · · · · · · · · · · · · ·	46	30 47	04	4378		
AM Pk Hr			7:00	8:00			
				1			

4:00

9.91%

8.16%

3:00

9.06%

9.48%

9.99%

AM Peak

PM Pk Hr

**PM Peak** 

% Peak Hr

% Peak Hr

1/20/2019

9.50%

9.21%

STA. 5

# Massachusetts Highway Department R12208 Weekly Volume Report - Mon 01/28/2019 - Sun 02/03/2019

Location ID:	R12208
Located On:	RAMP-RT 93 NB TO RT 125
Direction	RAMP
Community:	Wilmington
AADT:	

Type: SPOT

Period: Mon 01/28/2019 - Sun 02/03/2019

Start Time	Mon	Tue	Wed	Thu	Fri	Sat	Sun	Avg
12:00 AM		46	56	62			1	55
1:00 AM		33	43	44	a the second second		state - state	40
2:00 AM		48	41	62			11	50
3:00 AM	Carl Works	75	64	65		Non Your		68
4:00 AM		107	114	111				111
5:00 AM		303	282	285	1.		1	290
6:00 AM		471	400	431				434
7:00 AM	in the	802	660	730	1	1. A.	1	731
8:00 AM		909	762	831				834
9:00 AM		638	684	591			181	638
10:00 AM		428	449	426				434
11:00 AM		473	433	452				453
12:00 PM		538	487	515				513
1:00 PM	608	565	637	100 m 314		Street in the		603
2:00 PM	713	709	702					708
3:00 PM	724	802	779			51	Serie - The s	768
4:00 PM	945	771	917	2				878
5:00 PM	1037	1000	1015			10 - <sup>20</sup> -	1	1017
6:00 PM	724	866	665					752
7:00 PM	485	455	418			A. C. S. S.		453
8:00 PM	342	317	408	Second Street				356
9:00 PM	221	287	263	And the second				257
10:00 PM	184	221	226		1			210
11:00 PM	89	138	120	8-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1		$(1-1)^{-1} = \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum_{j=1}^{n-1} \sum_{j=1}^{n-1} \sum_{j=1}^{n-1} \sum_{i=1}^{n-1} \sum_{j=1}^{n-1} \sum$		116
Total	6072	11002	10625	4605	0	0	0	
24HrTotal	10	943 10	606 107	755				10768
AM Pk Hr		8:00	8:00					
AM Peak		909	762	(				836
PM Pk Hr		5:00	5:00	0				1
PM Peak		1000	1015					1008
% Peak Hr		9.09%	9.55%		1			9.50%
% Peak Hr	9.4	18% 9.4	3% 9.4	4%			19	9.45%

STA.6

# Massachusetts Highway Department S18-055-342-03 Weekly Volume Report - Mon 01/14/2019 - Sun 01/20/2019

Location ID:S18-055-342-03Located On:ON-RAMP FROM RTE.125 TO I-93NBDirectionRAMPCommunity:WILMINGTONAADT:

Туре:	SPOT
1997 - 24-20 1997 - 24-20 1997 - 24-20	
Period:	Mon 0

Mon 01/14/2019 - Sun 01/20/2019

Start Time	Mon	Tue	Wed	Thu	Fri	Sat	Sun	Avg
12:00 AM		-	1	2				2
1:00 AM	のないな地方		5	0				3
2:00 AM			0	2	1	C		1
3:00 AM		and and	2	0				1
4:00 AM			4	3				4
5:00 AM			11	11				11
6:00 AM			37	40			1	39
7:00 AM		and the second	57	57			Non Star of	57
8:00 AM			48	44	5			46
9:00 AM	the state		37.	51	4			44
10:00 AM			36	31				34
11:00 AM	et	29	36			Maria Alla - A		33
12:00 PM		31	32		· · · · · · · · · · · · · · · · · · ·			32
1:00 PM		30	25				$\beta_n^{(1)} =$	28
2:00 PM		40	33					37
3:00 PM		36	25	4			Alla	31
4:00 PM	1. In 1997	20	25			· · · · · · · ·		23
5:00 PM		43	42	the second of the			3	43
6:00 PM		30	38		×			34
7:00 PM	See a second	27	39					33
8:00 PM		15	24					20
9:00 PM	and a state of the	17	15					16
10:00 PM		7	5			/		6
11:00 PM		1	3		And we have	W.P	et per desta	2
Total	0	326	580	241	0	0	0	
24HrTotal		5	64 5	33				574
AM Pk Hr			7:00					1.00
AM Peak			57			1		57
PM Pk Hr			5:00					
PM Peak			42					42
% Peak Hr			9.83%					10.00%
% Peak Hr		7.	52% 9.7	8%				8.70%

# I-93 Southbound between Exit 7 (Furnace Brook Parkway) and Exit 8 (Route 3 Southbound)



MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 6/16/2019

County: Ramp ID # 12076

File: 1.prn

City: Quincy

5TA.1

Site Reference: 190020000141 Site ID: Station 1 Location: Furnace Brook Pkwy. on-ramp to I-93 SB Direction: SOUTH

TIME MON TUE WED THU FRI WKDAY SAT SUN WEEK TOTAL AVG AVG -------..... ----01:00 02:00 03:00 04:00 35 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 418 427 16:00 17:00 18:00 19:00 20:00 .217 21:00 22:00 23:00 24:00 TOTALS 7437 7031 6908 6754 6950 7007 5996 6298 6756 96.3 98.5 99.1 % AVG WKDY 106.1 100.3 85.5 89.8 % AVG WEEK 110 104 102.2 99.9 102.8 88.7 93.2 09:00 09:00 09:00 09:00 09:00 09:00 11:00 12:00 09:00 AM Times AM Peaks 18:00 18:00 17:00 18:00 16:00 18:00 15:00 16:00 18:00 PM Times PM Peaks 622 755 

MassDOT Highway Division WEEKLY SUMMARY FOR LANE Starting: 6/16/2019

STA.2

TOTAL

Page: 3

File: comb..prn City: Quincy County: Ramp ID # 12077

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: ROAD TOTAL

TIME	MON 17	TUE 18	WED 19	THU 20	FRI 21	WKDAY AVG	SAT 22	SUN 16	WEEK AVG	TOTA
01:00	811	915	933	1213	1203	1015	1674	1585	1190	8334
02:00	556	616	709	786	814	696	1172	1245	842	5898
03:00	368	443	434	534	724	500	943	980	632	4426
04:00	324	361	396	400	435	383	640	980 607	451	3163
05:00	623	728	677	744	593	673	582	445	627	4392
06:00	1521	1494	1500	1583	1340	1487	927	563	1275	8928
07:00	2580	2661	2581	2553	2338	2542	1680	943	2190	15336
08:00	2768	3003	2988	2790	2769	2863	2331	1494	2591	18143
09:00	2857	2938	2812	2703	2756	2813	2999	1877	2706	18942
10:00	2745	2887	2768	2641	2452	2698	3153	2641	2755	19287
11:00	2885	2855	2972	2840	2980	2906	3430	3370	3047	21332
12:00	3239	3313	3304	3033	3365	3250	3608	3618		23480
13:00	3403	3004	3619	3238	3441	3341	3622	3619	3420	23946
14:00	3533	2702	3689	3092	2975	3198	3427	3522	3277	22940
15:00	3436	2631	3337	3189	2786	3075	3489	3106	3139	21974
16:00	2828	2291	2695	2165	2497	2495	3154	2815	2635	18445
17:00		2409	2679		2314	2357	3130	3477	2627	18392
18:00	2655	2625	2693	2932	2215	2624	2663	3113	2699	18896
		3112	3277	3074	2996	3076	3311	3023	3102	21718
20:00	3514	3600	3594	3174	3280	3432	3579	2891	3376	23632
21:00	2788	2984	3350	3125	2928	3035		2600	3002	
22:00	2487	2492	2705	2502	2770	2591	3041	2343	2620	18340
23:00	2487 2005	1871		2327	2752	2169	2937		2206	15442
24:00	1667	1700		1971		1985	2897	1376		14199
TOTALS	55136	53635	57523	54374	55391	55204	61628	52912	55791	390599
% AVG WKDY	99.8	07 1	104.2	98.4	100.3		111.0	95.8		
% AVG WEEK	99.0	96.1	104.2	97.4	99.2			95.8		
6 AVG WEEK	90.0	90.1	103.1	97.4	99.2		110.4	94.8		
AM Times	12:00	12:00	12:00	12:00	12:00	12:00	12:00	12:00	12:00	
AM Peaks	3239	3313	3304	3033	3365	3250	3608	3618	3354	
PM Times	14:00	20:00		13:00		20:00			13:00	
PM Peaks	3533	3600	3689	3238	3441	3432	3622	3619	3420	
D%	55	55	55	55	55		55	55		
K%	6	7	6	6	6		55	7		

#### MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 6/16/2019

STA.2

LNII

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH

TIME	MON	TUE 18			FRI 21	WKDAY AVG		SUN 16	WEEK AVG	TOTAL
						AVG			AVG	
						144				
01:00		399				433				3518
	226	242	262			286				2413
03:00	147	186	186	220	294	206	395	399	261	1827
04:00	125	145 359	153	164	177 300	152 325	265	245	182	1274
05:00	288	359	324	358	300	325	239	199		2067
	696	695	677			687		218	573	4011
07:00		1203	1204			1186			993	6955
08:00	1367	1431	1446	1365	1351			616	1222	8558
09:00	1397 1376	1404	1360	1321	1343	1365			1263	8847
10:00	1376	1418	1388		1215	1354				9201
11:00	1334	1330	1353	1357	1362	1347		1437		9769
12:00	1495	1476	1487	1418	1567	1488	1701	1629		10773
13:00	1509	1415	1618	1529	1582	1530	1710	1694		11057
14:00	1581	1279	1685	1468	1465		1568	1659		10705
15:00	1616	1282	1589	1524	1349		1638	1433		10431
16:00	1385	1105	1327	1087	1218		1500	1330		8952
17:00		1169	1423	875	1128	1190	1501	1591		9046
18:00	1444	1371	1451	1488	1095	1369	1354	1394	1371	9597
19:00	1522 1574 1207	1531	1606	1520	1452	1526		1344		10484
20:00	1574	1635	1618			1558		1275		
81.00	2201	10/0		1390	1178			1139		9121
	1067	1079	1149	1032		1101	1320	1006		7833
23:00	865	738	864	897	1176	908 845	1241	727	929	6508
24:00	681	660	954	835	1098	845	1221	563	858	6012
TOTALS	25828	24822	26988	25549	25616			23234	25649	179608
<pre>% AVG WKDY</pre>	100 3	96.3	104 8	99.2	99.4		107	90.2		
<pre>% AVG WEEK</pre>		96.7					107.4	90.5		
	12:00		12:00			12:00		12:00	[1] A. B. M.	
AM Peaks	1495	1476	1487	1418	1567	1488	1701	1629	1539	
PM Times			14:00			20:00			13:00	
PM Peaks	1616	1635	1685	1529	1582	1558	1710	1694	1579	

File: comb..prn

City: Quincy County: Ramp ID # 12077 MassDOT Highway Division WEEKLY SUMMARY FOR LANE 2 Starting: 6/16/2019

5TA . 2 LN . 2 Page: 2

File: comb..prn

County: Ramp ID # 12077

City: Quincy

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH

TIME	MON 17	TUE 18	WED 19	THU 20	FRI 21	WKDAY AVG	SAT 22	SUN 16	WEEK AVG	TOTAL
********	هم بالمحمد المحم				الرباب وججو					
									12.2.2	6676
	477				692					4816
02:00		374	447		466	409	697	740		3485
03:00	221	257 216	248	314	430	294	548	581	371	2599
04:00	199	216	243	236	258	294	375	362	269	1889
05:00	335	369	353		293	347	343		332	2325
06:00		799	823	859	693	799	573	345	702	4917
		1458	1377	1372		1356		592	1197	8381
08:00		1572	1542	1425	1418	1471	1349	878	1369	9585
09:00	1460	1534	1452	1382	1413	1448	1714	1140	1442	10095
10:00		1469	1380	1200	1237	1344	1798	1565	1440	10086
11:00		1525	1619	1483		1559			1651	11563
12:00	1744	1837	1817	1615	1798	1762	1907	1989	1815	12707
13:00	1894	1589		1709	1859	1810	1912	1925	1841	12889
14:00	1952	1423	2004	1624	1510	1702	1859	1863	1747	12235
15:00	1820	1349	1748	1665	1437	1603	1851	1673	1649	11543
16:00	1443	1186	1368	1078	1279	1270	1654	1485	1356	9493
17:00	1259	1240	1256	890	1186	1166	1629	1886	1335	9346
18:00	1211	1254	1242	1444	1120	1254	1309	1719	1328	9299
19:00	1403	1581	1671	1554	1544	1550	1802	1679	1604	11234
20:00	1940	1965	1976	1678	1809	1873	1999	1616	1854	12983
21:00	1581	1714	1836	1735		1723	1816	1461	1699	11893
		1413		1470	1590	1489	1721	1337	1501	10507
23:00	1140	1133	1027			1261	1696	932	1276	8934
24:00		1040	966		1570	1139		813		8187
TOTALS	29308	28813	30535	28825	29775	29440	34057	29678	30132	210991
% AVG WKDY	99.5	97.8	103.7				115.6	100.8		
% AVG WEEK	97.2	95.6	101.3	95.6	98.8		113	98.4		
	12:00			12:00		12:00		the second second second	12:00	
AM Peaks	1744	1837	1817	1615	1798	1762	1907	1989	1815	
PM Times	14:00		14:00	the second se	13:00				20:00	
PM Peaks	1952	1965	2004	1735	1859	1873	1999	1925	1854	

MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 6/16/2019

Page: 1

57A.3

LN.1

File: comb..prn City: Quincy County: Ramp ID # 12032

Site Reference: 190020000042 Site ID: Station 3 Location: I-93 SB ramp to Route 3 SB Direction: SOUTH

TIME	MON 17	TUE 18	WED 19	THU 20	FRI	WKDAY AVG		SUN 16	WEEK AVG	TOTAL
01:00	279	351	305	721	453	421	643	497	464	3249
02:00	187	198				233		463	293	2052
03:00	121	151	126	209	250	171	311	315	211	1483
04:00	104		143	170	194	153	243		175	1227
05:00	124 268	276	272	281	226	264	234	130	241	1687
06:00	621	616	631	628	587	616	378	213	524	3674
07:00	1054	1020	1047	981	966		756		885	6201
08:00	1238	1281	1274	1197	1121	1222	1114	625	1121	7850
09:00	1201	1174	1188	1211	1039	1162	1340		1132	7929
10:00	1161	1180	1175	1213	1045	1154	1229	1091	1156	8094
11:00	1106	1129	1148	1079	1279	1148	1285	1220	1178	8246
12:00	1167	1248	1203	1146	1392	1231	1376	1245	1253	8777
13:00	1147	1043	1277	1164	1338	1193	1357	1384	1244	8710
14:00	1315	1068	1334	1270	1252	1247	1263	1166	1238	8668
15:00	1470		1569	1353	1316	1391	1304	1122	1340	9382
16:00	1408	1207	1498	1166	1302	1316	1153	1026	1251	8760
17:00	1406	1176	1402	1052	1325	1272	1120	1215	1242	8696
18:00		1398	1460	1499	1355	1419	983	1120	1314	9201
19:00		1502	1579	1583	1543	1547		1057	1420	9944
20:00	1313	1414	1414	1454	1262	1371	1120	1048	1289	9025
21:00		1094	1218	1192	1211	1155	1037	904	1102	7720
22:00	901	843	987	1024	1029	956	1006	785	939	6575
23:00	685	646	1063	892		845	945	604	825	5775
24:00	553							438		5115
TOTALS	22706	21910	24722	23434	23594	23262	22634	19040	22567	158040
% AVG WKDY	97.6	94.1	106.2	100.7	101.4		97.3	81.8		
<pre>% AVG WEEK</pre>	100.6	97	109.5	103.8	104.5		100.2	84.3		
	08:00		08:00	10:00	12:00	12:00	12:00	12:00	12:00	
AM Peaks	1238	1281	1274	1213	1392	1231	1376	1245	1253	
PM Times	19:00	19:00	19:00	19:00	19:00		13:00			
PM Peaks	1531	1502	1579	1583	1543	1547	1357	1384	1420	

STA. 3 LN12 NO PATA

.

#### MassDOT Highway Division SPEED SUMMARY Sun 6/16/2019

STA.Z

TOTAL

File: comb..prn
City: Quincy
County: Ramp ID # 12077

Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: ROAD TOTAL

Site Reference: 190020000034

TIME	30	35	40	45	50	55	60	65	70 '	75	80	85	90	91+	Total
01:00	1	0	3	22	91	423	512	360	116	40	13	2		1	1585
02:00	1	1	5	7	72	261	390	285	140	65	15	3	0	0	1245
03:00	1	ō	2	9	42	115	281	263	161	74	23	3	1	5	980
04:00	2	1	3	9	23	88	155	150	104	57	11	1	1	2	607
05:00	0	0	1	2	35	87	124	105	62	19	9	õ	1	0	445
06:00	1	0	1	2	6	45	127	141	135	79	20	6	ō	0	563
07:00	3	1	0	14	36	60	126	285	224	130	48	12	3	1	943
08:00	2	0	1	1	14	106	293	476	325	196	60	11	7	2	1494
09:00	0	4	3	2	30	223	528	582	331	143	29	2	Ó	0	1877
10:00	18	4	2	14	154	637	785	645	274	89	17	1	0	1	2641
11:00	. 7	32	80	248	601	1058	798	362	135	39	7	3	0	0	3370
12:00	199	228	488	894	842	656	243	54	13	0	1	0	0	0	3618
13:00	262	145	333	938	1159	668	104	4	2	3	0	0	0	1	3619
14:00	871	671	662	701	400	188	23	2	0	0	0	2	0	2	3522
15:00	421	361	299	514	582	590	272	56	6	4	0	1	0	0	3106
16:00	494	238	267	352	350	477	359	213	45	16	3	0	0	1	2815
17:00	102	183	409	899	991	651	182	46	8	4	1	1	0	0	3477
18:00	4	2	110	305	687	1118	653	185	34	8	5	0	0	2	3113
19:00	34	48	143	416	702	984	502	156	28	6	3	1	0	0	3023
20:00	10	33	92	246	587	1099	576	189	43	14	1	0	0	1	2891
21:00	5	9	41	222	534	947	570	210	43	12	6	1	0	0	2600
22:00	4	0	16	113	383	831	647	264	71	12	0	0	0	2	2343
23:00	2	1	5	20	142	401	542	369	125	37	13	1	0	1	1659
24:00	2	0	1	26	72	317	434	330	146	37	10	1	0	0	1376
DAY TOTAL	2446	1962	2967	5976	9525	12030	9226	5732	2571	1084	295				52912
PERCENTS	4.7%	3.8%				22.8%			4.8%	2.0%	0.5%	0.0%	0.0%	0.08	100%

Statistical Information ...

15th Percentile Speed 40.5 mph

Median Speed 51.9 mph

10 MPH Pace Speed 50 mph to 60 mph 21256 vehicles in pace Representing 40.1% of the total vehicles 85th Percentile Speed 61.6 mph

Average Speed 50.5 mph

Vehicles > 65 MPH 4038 7.6%

# MassDOT Highway Division SPEED SUMMARY Sun 6/16/2019

File: comb..prn

# 57A.3

Site ID: Station 3 City: Quincy LN, 1 Location: I-93 SB ramp to Route 3 SB County: Ramp ID # 12032 Direction: SOUTH Lane: 1 TIME 40 45 50 55 60 80 85 86+ Total 65 70 75 149 167 85 30 8 1 1 50 01:00 0 6 0 497 1 139 145 88 14 14 2 75 99 75 27 8 1 02:00 16 44 0 463 0 15 99 46 67 75 7 23 7 21 
 99
 75
 27
 8
 1
 0
 0
 315

 67
 50
 19
 8
 0
 0
 219
 03:00 0 1 04:00 1 0 12 43 34 30 7 3 0 0 0 130 0 4 9 43 55 68 27 7 0 0 0 213 05:00 7 0 0 0 213 06:00 1 7 0 2 0 2 16 49 07:00 106 112 68 16 2 0 377 5 80 193 200 103 34 5 08:00 0 0 14 263 255 63 14 3 09:00 164 10:00 283 443 223 61 16 3 33 9 9 1 0 0 2 0 1220 11:00 488 338 133 6 108 12 136 37 218 12 136 12:00 526 9 0 0 1245 0 1384 0 1166 180 33 Ő 516 3 0 24 18 0 0 13:00 556 137 400 394 390 445 206 311 6 1 14:00 92 0 
 92
 18
 6

 106
 27
 5

 164
 71
 11
 0 0 5 1 11 1 0 1122 15:00 0 16:00 15 147 0 0 1026 0 1215 

 511
 104
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 11
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 178
 530
 299
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 12
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 0 17:00 178 530 299 63 13 3 1 0 0 1120 18:00 3 26 194 546 233 48 6 1 0 0 0 1057 19:00 

 3
 20
 194
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 233
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 6
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 18
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 212
 505
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 27
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 418
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 73
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 26
 96
 288
 258
 85
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 20:00 0 1048 0 904 0 785 21:00 
 85
 21
 3
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 133
 27
 4
 0
 22:00 0 0 15 46 181 198 23:00 4 0 0 604 0 14 52 132 129 82 20 9 0 0 24:00 0 438 \_\_\_\_\_ DAY TOTAL12211503749686942982050611164215119040PERCENTS0.7%6.1%19.7%36.1%22.6%10.7%3.2%0.8%0.1%0.0%0.0%100%

Statistical Information...

Site Reference: 190020000042

- 15th Percentile Speed 47.1 mph
- Median Speed 53.3 mph

10 MPH Pace Speed 50 mph to 60 mph 11167 vehicles in pace Representing 58.6% of the total vehicles 85th Percentile Speed 60.0 mph

Average Speed 53.6 mph

Vehicles > 65 MPH 802 4.2%

#### MassDOT Highway Division SPEED SUMMARY Mon 6/17/2019

File: comb..prn

County: Ramp ID # 12032

City: Quincy

Site Reference: 190020000042 Site ID: Station 3 Location: I-93 SB ramp to Route 3 SB Direction: SOUTH Lane: 1

1.0

40 45 50 55 TIME 60 65 70 75 80 85 86+ Total \_\_\_\_\_ 0 8 0 4 75 92 61 23 4 42 57 49 19 3 01:00 02:00 23 4 0 0 16 0 279 13 0 187 0 0 2 1 0 1 03:00 10 29 0 121 10 29 32 7 22 31 0 124 04:00 1 0 10 44 8 3 0 1 268 76 41 05:00 84 0 0 5 60 199 207 100 42 8 0 0 621 06:00 
 100
 12
 8
 0
 0
 621

 269
 53
 17
 0
 0
 0
 1054

 255
 60
 6
 0
 0
 0
 1238

 170
 25
 9
 3
 0
 0
 1201

 07:00
 3
 4
 30
 237
 441
 269
 53
 17
 0
 0
 0
 1054

 08:00
 1
 7
 73
 336
 500
 255
 60
 6
 0
 0
 0
 1238

 09:00
 13
 29
 134
 416
 402
 170
 25
 9
 3
 0
 0
 1201

 10:00
 3
 13
 93
 375
 442
 183
 38
 10
 3
 0
 1
 1161

 11:00
 10
 51
 134
 462
 350
 76
 16
 6
 1
 0
 0
 1106

 07:00 09:00 7 83 11 80 12:00 242 453 278 82 18 1 3 0 0 1167 
 13:00
 11
 80
 254
 447

 14:00
 4
 39
 247
 644

 15:00
 201
 231
 514
 432

 16:00
 289
 589
 396
 108
 1 0 0 1147 0 1315 6 2 252 70 24 296 67 13 5 0 82 8 18 5 0 0 0 2 0 1470 0 1408 1 1 0 1 1361 3 0 0 1406 0 17:00 28 12 0 0 28 3 0 0 0 0 1386 1378 18:00 0 1 0 0 0 0 0 1386 0 0 0 0 0 0 1531 19:00 1359 104 53 13 97 110 213 444 322 101 24 2 0 5 94 418 375 138 23 10 2 0 0 0 1313 20:00 0 0 3 0 1 1064 0 901 0 5 94 418 375 138 23 8 28 109 359 290 88 11 21:00 22:00 5 8 2 0 0 685 8 12 58 214 241 114 28 23:00 0 4 23 138 211 125 32 16 3 1 0 553 24:00 4756 1435 2743 5771 4999 2216 578 171 32 2 3 22706 DAY TOTAL 21.0% 6.4% 12.1% 25.5% 22.0% 9.7% 2.5% 0.7% 0.1% 0.0% 0.0% 100% PERCENTS

Statistical Information...

15th Percentile Speed 28.7 mph

Median Speed 52.1 mph

10 MPH Pace Speed 50 mph to 60 mph 10770 vehicles in pace Representing 47.4% of the total vehicles. 85th Percentile Speed 59.6 mph

Average Speed 47.1 mph

Vehicles > 65 MPH 786 3.5%

#### MassDOT Highway Division SPEED SUMMARY Tue 6/18/2019

Location: I-93 SB ramp to Route 3 SB County: Ramp ID # 12032 Direction: SOUTH Lane: 1 40 45 50 55 60 65 70 75 80 85 86+ Total TIME ------1226930165653181042451273537176 4 01:00 02:00 0 93 122 69 4 0 0 351 13 4 13 56 53 10 2 0 0 42 0 198 0 151 0 134 0 03:00 32 1 1 20 04:00 1 3 3 0 4 81 73 37 16 0 0 0 276 05:00 7 56 8 
 71
 198
 204
 91
 31
 7
 1
 1
 616

 239
 421
 234
 62
 14
 3
 0
 0
 1020
 06:00 1 10 36 239 421 234 1 10 56 362 556 234 6 30 95 402 421 182 07:00 50 9 3 08:00 0 0 1281 

 6
 30
 95
 402
 421
 182
 33
 4
 1
 0
 0
 1174

 0
 39
 148
 447
 412
 113
 19
 1
 1
 0
 0
 1180

 1
 26
 147
 102
 112
 113
 19
 1
 1
 0
 0
 1180

 09:00 10:00 372 11:00 1 26 147 439 117 24 3 0 0 0 1129 4 12:00 297 74 20 5 0 0 0 1248 28 263 557 23 54 250 395 61 0 257 2 13:00 1 0 0 1043 9 4 7 1 1 0 0 0 1 0 317 462 1 0 1068 14:00 217 57 653 125 0 15:00 427 34 0 1248 1 0 816 522 0 0 0 1207 16:00 255 91 31 10 2 3 438 159 44 1 17:00 10 0 0 0 0 1176 1 0 0 0 0 0 0 0 0 1398 1389 18:00 4 1 4 1 ı 1 1 1477 16 5 1 0 0 0 0 1502 19:00 185 343 491 329 53 10 0 0 0 1 2 1414 20:00 1 22 149 453 353 92 17 2 2 1 0 0 1094 0 843 21:00 3 22:00 14 63 138 304 228 74 16 5 0 0 1 646 23:00 1 11 38 253 231 81 25 5 24:00 1 4 17 113 184 145 31 15 3 0 0 513 DAY TOTAL 5447 2470 2630 4633 4160 1871 506 152 31 6 4 21910 PERCENTS 24.9% 11.3% 12.1% 21.2% 19.0% 8.5% 2.3% 0.6% 0.1% 0.0% 0.0% 100%

Statistical Information ...

Site Reference: 190020000042

Site ID: Station 3

15th Percentile Speed 24.1 mph

Median Speed 50.5 mph

10 MPH Pace Speed 50 mph to 60 mph 8793 vehicles in pace Representing 40.1% of the total vehicles 85th Percentile Speed 59.1 mph

Average Speed 45.0 mph

Vehicles > 65 MPH 699 3.2%

Page: 3

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File: comb..prn

City: Quincy

# MassDOT Highway Division SPEED SUMMARY Wed 6/19/2019

Site Reference: 190020000042 Site ID: Station 3 Location: I-93 SB ramp to Route 3 SB Direction: SOUTH Lane: 1 File: comb..prn City: Quincy County: Ramp ID # 12032

TIME	40	45	50	55	60	65	70	75	80	85	86+	Total	
		99999						******	229922				
01:00	6	0	13	67	94	75	30	17	2	1	0	305	
02:00	0	4	18	50	61	60	20	11	1	0	. 0	225	
03:00	0	7	13	24	33	37	6	6	0	0	0	126	
04:00	0	6	10	26	38	37	20	6	0	0	0	143	
05:00	0	0	10	43	58	78	56	23	4	0	0	272	
06:00	0	1	14	91	190	179	113	35	8	0	0	631	
07:00	0	12	41	232	416	260	66	16	4	0	0	1047	
08:00	2	42	155	383	447	192	42	10	0	1	0	1274	
09:00	5	31	127	425	402	142	48	8	0	0	0	1188	
10:00	0	7	105	462	381	197	21	1	1	0	0	1175	
11:00	1	15	128	460	399	123	18	3	1	0	0	1148	
12:00	9	134	263	501	231	53	8	3	0	0	1	1203	
13:00	2	56	232	551	326	94	10	5	1	0	0	1277	
14:00	10	79	291	611	276	52	10	5	0	0	0	1334	
15:00	810	267	267	184	34	3	1	1	1	0	1	1569	
16:00	1404	74	15	3	1	1	0	0	0	0	0	1498	
17:00	1398	4	0	0	0	0	0	0	0	0	0	1402	
18:00	1450	7	1	1	0	0	1	0	0	0	0	1460	
19:00	1325	93	114	41	6	0	0	0	0	0	0	1579	
20:00	34	324	578	368	82	23	1	2	0	0	2	1414	
21:00	14	60	246	571	272	41	11	2	1	0	0	1218	
22:00	6	25	144	431	268	90	16	6	1	0	0	987	
23:00	22	131	228	399	203	61	15	3	1	0	0	1063	
24:00	701	90	106	163	87	34	3	0	0	0	0	1184	
AY TOTAL		1469			4305		516	163	26	2		24722	
ERCENTS	29.28	6.08	12.78	24.68	17.4%	1.48	2.0%	0.6%	0.1%	0.0%	0.0%	100%	
catistical	Inform	ation	255										
15th T	Percenti	le Sn	heed								\$	S5th Der	centile Speed

15th Percentile Speed 20.6 mph

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Median Speed 50.5 mph

10 MPH Pace Speed 50 mph to 60 mph 10392 vehicles in pace Representing 42.0% of the total vehicles 85th Percentile Speed 58.7 mph

Average Speed 43.9 mph

Vehicles > 65 MPH 711 2.9%

# MassDOT Highway Division SPEED SUMMARY Thu 6/20/2019

Site Reference: 190020000042 Site ID: Station 3 Location: I-93 SB ramp to Route 3 SB Direction: SOUTH Lane: 1 File: comb..prn City: Quincy County: Ramp ID # 12032

TIME	40	45	50	55	60	65	70	75	80	85	86+	Total	
				2433355									
01:00	3	58	132	266	185	62	11	3	1	0	0	721	
02:00	0	0	9	81	92	68	21	6	0	0	0	277	
03:00	1	3	21	46	52	53	22	10	0	1	0	209	
04:00	0	4	8	32	47	49	23	5	2	0	0	170	
05:00	2	1	8	51	84	80	39	13	3	0	0	281	
06:00	0	3	13	70	177	203	111	45	6	0	0	628	
07:00	0	14	47	240	373	231	63	12	1	0	0	981	
08:00	2	35	155	445	388	150	18	3	1	0	0	1197	
09:00	5	56	234	480	324	92	17	2	0	0	1	1211	
10:00	10	141	378	491	169	20	3	1	0	0	0	1213	
11:00	10	87	263	496	174	41	7	0	1	0	0	1079	
12:00	47	204	498	324	57	11	4	1	0	0	0	1146	
13:00	45	308	478	275	41	12	3	1	1	0	0	1164	
14:00	131	436	442	215	38	7	0	0	0	0	1	1270	
15:00	250	360	469	215	44	12	3	0	0	0	0	1353	
16:00	1074	68	15	8	1	0	0	0	0	0	0	1166	
17:00	328	451	170	68	17	12	5	1	0	0	0	1052	
18:00	1409	72	13	2	2	0	0	1	0	0	0	1499	
19:00	1512	52	8	6	4	1	0	0	0	0	0	1583	
20:00	386		411	87	9	3	1	0	0	0	0	1454	
21:00	12	104	329	499	205	32	10	1	0	0	0	1192	
22:00	13	80	185	398	248	75	19	6	0	0	0	1024	
23:00	3	23	107	398	271	68	15	3	3	0	1	892	
24:00	3	3	26	147	280	149	47	10	3	1	3	672	
DAY TOTAL	5246					1431	442	124	22	2		23434	
PERCENTS	22.4%	13.4%	18.9%	22.8%	14.1%	6.1%	1.8%	0.5%	0.0%	0.0%	0.0%	100%	
tatistica	Infor	mation											

Statistical Information...

15th Percentile Speed 26.8 mph

Median Speed 48.8 mph

10 MPH Pace Speed 45 mph to 55 mph 9759 vehicles in pace Representing 41.6% of the total vehicles 85th Percentile Speed 57.7 mph

Average Speed 44.7 mph

Vehicles > 65 MPH 596 2.5%

# MassDOT Highway Division SPEED SUMMARY Fri 6/21/2019

4

Site Reference: 190020000042 Site ID: Station 3 Location: I-93 SB ramp to Route 3 SB Direction: SOUTH Lane: 1 File: comb..prn City: Quincy County: Ramp ID # 12032

TIME	40	45	50	55	60	65	70	75	80	85	86+	Total	
01:00	5	4	21	109	153	108	34	13	0	0	6	453	
02:00	1	5	20	50	81	72	26	21	3	1	1	281	
03:00	2	4	14	66	75	55	23	8	0	2	1	250	
04:00	3	6	17	38	69	43	14	2	1	0	1	194	
05:00	3	10	40	74	51	34	12	1	0	0	1	226	
06:00	3	34	136	255	115	36	6	1	0	0	1	587	
07:00	2	35	172	436	251	61	5	1	0	1	2	966	
08:00	1	53	217	449	314	69	13	2	0	1	2	1121	
09:00	5	56	151	439	285	85	13	2	2	0	1	1039	
10:00	10	56	180	444	279	61	8	3	0	0	4	1045	
11:00	5	105	293	580	228	51	7	4	0	1	5	1279	
12:00	19	167	495	598	96	9	3	0	0	0	5	1392	
13:00	247	181	403	376	90	16	4	1	1	3	16	1338	
14:00	432	318	341	113	16	9	2	0	4	5	12	1252	
15:00	1009	217	64	13	3	4	0	1	0	2	3	1316	
16:00	1166	109	23	2	1	1	0	0	0	0	0	1302	
17:00	1294	26	4	1	0	0	0	0	0	0	0	1325	
18:00	1320	33	1	1	0	0	0	0	0	0	0	1355	
19:00	1196	198	105	38	4	1	0	0	0	0	1	1543	
20:00	189	261	376	329	84	17	4	2	0	0	0	1262	
21:00	1	57	202	544	318	70	11	5	2	0	1	1211	
22:00	3	12	139	392	361	96	17	8	0	0	1	1029	
23:00	4	21	104	328	320	128	29	4	0	1	1	940	
24:00	1	13	51	274	337	163	33	14	2	0	0	888	
AY TOTAL	6921	1981				1189	264	93	15	17		23594	
ERCENTS	29.4%	8.4%	15.2%	25.3%	15.0%	5.1%	1.1%	0.3%	0.0%	0.0%	0.2%	100%	
tatistical	Inform	ation											

15th Percentile Speed 20.5 mph

Median Speed 49.1 mph

10 MPH Pace Speed 45 mph to 55 mph 9518 vehicles in pace Representing 40.3% of the total vehicles 85th Percentile Speed 57.3 mph

Average Speed 42.9 mph

Vehicles > 65 MPH 454 1.9%

# MassDOT Highway Division SPEED SUMMARY Sat 6/22/2019

Site Reference: 190020000042 Site ID: Station 3 Location: I-93 SB ramp to Route 3 SB Direction: SOUTH Lane: 1 File: comb..prn City: Quincy County: Ramp ID # 12032

	40	45	50	55	60	65	70	75	80	85	86+	Total	
	******												
01:00	1	4	43	134	253	160	34	10	3	1	0	643	
02:00	1	3	16	95	141	117	37	9	2	0	0	421	
03:00	0	1	7	58	107	85	37	9	5	1	1	311	
04:00	0	1	5	37	79	68	32	17	4	0	0	243	
05:00	0	1	8	26	66	74	36	16	6	1	0	234	
06:00	0	0	6	26	106	126	75	28	11	0	0	378	
07:00	2	4	14	101	246	238	106	32	11	2	0	756	
08:00	2	7	63	246	449	266	54	20	6	0	1	1114	
09:00	3	27	98	399	547	212	44	9	1	0	0	1340	
10:00	1	21	89	416	490	183	22	5	2	0	0	1229	
11:00	6	132	367	532	190	45	12	0	1	0	0	1285	
12:00	19	135	479	584	138	18	3	0	0	0	0	1376	
13:00	15	141	413	625	139	16	4	4	0	0	0	1357	
14:00	2	80	378	555	214	29	5	0	0	0	0	1263	
15:00	5	111	372	557	210	36	10	3	0	0	0	1304	
16:00	34	293	473	278	53	13	8	0	1	0	0	1153	
17:00	38	220	398	357	86	14	5	2	0	0	0	1120	
18:00	106	206	303	247	67	33	18	2	1	0	0	983	
19:00	6	27	127	439	371	134	36	8	1	0	0	1149	
20:00	4	20	119	417	384	138	25	10	2	1	0	1120	
21:00	8	21	90	428	347	109	21	11	2	0	0	1037	
22:00	1	30	124	433	316	75	21	3	3	0	0	1006	
23:00	7	24	124	377	287	93	24	7	1	1	0	945	
24:00	7	8	79	315	306	103	38	9	1	1	0	867	
AY TOTAL	268	1517	4195	7682	5592	2385	707	214		8		22634	******
PERCENTS						10.5%						100%	
Statistical	Inform	ation											
15th 5	ercenti	le Spe	eed								8	35th Per	centile Speed

46.9 mph

Median Speed 53.5 mph

10 MPH Pace Speed 50 mph to 60 mph 13274 vehicles in pace Representing 58.6% of the total vehicles 85th Percentile Speed 60.0 mph

Average Speed 53.6 mph

Vehicles > 65 MPH 995 4.4%

STA.3 LN.2 NO DATA

4

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# MassDOT Highway Division SPEED SUMMARY Mon 6/17/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: ROAD TOTAL File: comb..prn City: Quincy County: Ramp ID # 12077

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Tota
										******					
01:00	2	1	0	4	20	101	241	241	138	51	10	2	0	0	811
02:00	0	0	2	2	6	73	167	151	100	43	7	3	1	1	556
03:00	3	0	0	6	19	56	93	102	51	25	7	5	1	0	368
04:00	0	0	1	2	16	45	54	90	76	28	8	3	0	1	324
05:00	0	0	0	1	18	- 39	96	171	165	86	36	9	0	2	623
06:00	2	0	4	2	21	97	298	513	353	170	46	11	2	2	1521
07:00	6	1	0	10	94	564	890	658	263	76	17	0	1	0	2580
08:00	17	17	35	71	189	757	898	519	184	62	19	0	0	0	2768
09:00	360	144	81	182	353	728	597	315	82	12	1	0	0	2	2857
10:00	4	2	8	150	424	852	754	395	124	23	7	1	0	1	2745
11:00	72	55	109	216	493	956	635	259	68	18	1	0	0	3	2885
12:00	137	141	224	563	570	796	493	236	59	17	3	0	0	0	3239
13:00	187	207	330	620	756	704	355	160	70	11	1	1	1	0	3403
14:00	160	180	416	694	727	839	383	91	26	11	2	2	0	2	3533
15:00	1031	587	551	643	397	181	30	7	3	0	3	0	1	2	3436
16:00	1960	446	235	131	37	8	0	3	1	2	3	0	0	2	2828
17:00	767	235	297	602	430	216	52	9	3	0	3	1	1	2	2618
18:00	21	58	274	1060	803	340	73	20	2	2	2	0	0	0	2655
19:00	5	37	364	1287	802	355	61	10	1	0	3	0	0	0	2925
20:00	3	2	81	441	768	1134	725	277	63	17	2	1	0	0	3514
21:00	2	2	9	89	325	953	887	392	102	21	3	2	0	1	2788
22:00	6	2	14	105	318	871	784	276	84	18	8	· 0	0	1	2487
23:00	5	2	14	42	206	691	576	332	96	35	5	0	1	0	2005
24:00	5	0	0	29	91	358	517	415	175	63	11	1	0	2	1667
			2040				0000								
DAY TOTAL	4755	2119	3049	6952	1.10.00.00	11714	9659	5642	2289	791	208	42	9		55136
PERCENTS	8.7%	3.9%	5.68	12.78	14.38	21.3%	17.5%	10.28	4.1%	1.4%	0.3%	0.0%	0.0%	0.08	100%

Statistical Information ...

15th Percentile Speed 37.3 mph

Median Speed 51.2 mph

10 MPH Pace Speed 50 mph to 60 mph 21373 vehicles in pace Representing 38.7% of the total vehicles 85th Percentile Speed 60.7 mph

Average Speed 48.6 mph

Vehicles > 65 MPH 3363 6.1%

#### MassDOT Highway Division SPEED SUMMARY Tue 6/18/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: ROAD TOTAL File: comb..prn City: Quincy County: Ramp ID # 12077

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total
										مربع مربع مربع مربع				*****	
01:00	1	0	1	6	29	169	265	271	109	52	11	0	0	1	915
02:00	2	0	1	4	22	78	163	183	104	37	20	1	0	1	616
03:00	0	0	1	0	19	61	110	129	78	32	10	2	0	1	443
04:00	2	0	1	0	12	35	90	104	59	38	17	2	0	1	361
05:00	0	0	5	8	22	73	162	206	141	73	28	7	3	0	728
06:00	0	0	0	14	12	128	360	450	327	152	42	6	1	2	1494
07:00	8	0	6	59	211	631	820	586	242	73	21	3	0	1	2661
08:00	11	0	1	38	252	1003	987	521	146	36	7	1	0	0	3003
09:00	12	3	21	128	457	990	854	353	98	20	2	0	0	0	2938
10:00	9	12	34	190	458	1025	714	333	88	21	1	1	0	1	2887
11:00	10	0	8	148	542	1041	739	276	79	.9	2	0	0	1	2855
12:00	11	18	130	468	881	983	549	229	31	12	1	0	0	0	3313
13:00	1441	710	386	244	89	58	42	22	5	2	0	0	0	5	3004
14:00	1956	278	259	152	44	7	0	1	3	0	0	0	0	2	2702
15:00	2347	189	69	16	2	3	1	2	0	0	1	1	0	0	2631
16:00	2174	76	25	4	2	1	3	1	0	0	1	3	1	0	2291
17:00	2234	93	29	27	9	8	4	0	0	2	0	1	1	1	2409
18:00	668	187	562	809	306	83	8	1	0	0	0	0	1	0	2625
19:00	24	190	896	1417	446	118	15	1	2	0	1	1	1	0	3112
20:00	8	64	587	1460	824	503	119	18	9	4	1	2	0	1	3600
21:00	7	7	34	197	583	1142	676	254	74	8	2	0	0	0	2984
22:00	8	5	162	454	386	674	511	227	46	16	1	0	0	2	2492
23:00	4	2	8	47	209	614	587	295	77	20	8	0	0	0	1871
24:00	· 3	1	10	17	75	347	538	454	167	71	13 ,	4	0	0	1700
DAY TOTAL	10940	1835	3236	5907	5892	9775	8317	4917	1885	678	190	35	8	20	53635
PERCENTS	20.4%	3.5%	6.1%	11.1%	11.0%	18.3%	15.5%	9.1%	3.5%	1.2%	0.3%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 22.1 mph

Median Speed 49.2 mph

10 MPH Pace Speed 50 mph to 60 mph 18092 vehicles in pace Representing 33.7% of the total vehicles 85th Percentile Speed 59.8 mph

Average Speed 44.2 mph

Vehicles > 65 MPH 2816 5.3%

# MassDOT Highway Division SPEED SUMMARY Wed 6/19/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: ROAD TOTAL File: comb..prn City: Quincy County: Ramp ID # 12077

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Tota
									******			रत्त्रं रंग्स	*****		
01:00	1	0	0	0	36	137	274	258	139	68	17	2	1	0	933
02:00	0	1	2	2	19	122	191	201	106	45	14	5	1	0	709
03:00	0	0	0	4	20	57	112	124	69	31	14	1	0	2	434
04:00	0	0	2	5	13	51	88	122	58	43	11	2	1	0	396
05:00	1	0	0	9	14	63	127	206	132	79	38	6	2	0	677
06:00	4	0	1	6	21	102	348	478	346	156	29	9	0	0	1500
07:00	0	0	0	16	107	614	944	617	209	57	15	2	0	0	2581
08:00	240	47	94	139	325	849	809	351	102	27	4	0	0	1	2988
09:00	449	105	117	197	329	747	591	203	53	10	4	2	1	4	2812
10:00	31	15	12	126	437	960	779	306	79	20	2	0	0	1	2768
11:00	54	46	54	233	474	1045	711	262	73	15	3	1	0	1	2972
12:00	450	385	388	528	554	644	242	86	16	7	3	0	0	1	3304
13:00	163	166	233	633	825	983	452	121	33	3	2	1	0	4	3619
14:00	27	57	300	951	1020	989	260	65	14	2	1	2	1	0	3689
15:00	13	35	348	1102	1161	576	77	20	2	0	0	0	1	2	3337
16:00	932	97	345	729	381	169	26	10	2	2	0	0	2	0	2695
17:00	215	119	363	906	654	319	70	26	6	0	1	0	0	0	2679
18:00	13	20	205	895	910	484	126	30	7	3	0	0	0	0	2693
19:00	35	134	559	1380	844	259	49	10	2	0	3	0	1	1	3277
20:00	16	70	426	1332	1038	543	119	34	12	1	2	0	0	1	3594
21:00	38	91	143	513	990	1033	372	141	20	6	1	1	0	1	3350
22:00	2	6	34	221	511	979	592	266	68	20	3	1	0	2	2705
23:00	2	1	19	64	234	597	575	303	79	13	3	0	1	0	1891
24:00	6	4	34	153	356	654	438	211	47	14	1	0	1	1	1920
DAY TOTAL	2692	1399	3679	10144	11273	12976	8372	4451	1674	622	171	35	13	22	57523
PERCENTS	4.7%	2.5%	2.2.5.5			22.6%		7.8%	2.9%	1.0%	0.28	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 40.4 mph

Median Speed 49.8 mph

10 MPH Pace Speed 45 mph to 55 mph 24249 vehicles in pace Representing 42.1% of the total vehicles 85th Percentile Speed 59.0 mph

Average Speed 48.8 mph

Vehicles > 65 MPH 2537 4.4%

# MassDOT Highway Division SPEED SUMMARY Thu 6/20/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: ROAD TOTAL File: comb..prn City: Quincy County: Ramp ID # 12077

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Tota
01:00	1		0	4	45	300	400	310	112	34	6	1	0	0	1213
02:00	1	0	0	3	32	130	224	225	87	63	17	ĩ	õ	3	786
03:00	4	2	0	7	19	66	155	150	79	36	13	2	Ő	1	534
04:00	0	0	0	5	9	74	102	98	60	42	8	2	Ő	0	400
05:00	1	o	3	6	20	85	155	200	147	86	35	5	ĩ	0	744
06:00	1	1	4	13	31	183	372	418	337	156	56	9	ī	1	1583
07:00	8	1	19	56	178	680	853	492	182	71	11	2	0	0	2553
08:00	2	0	7	88	396	1107	804	286	80	18	2	0	0	0	2790
09:00	147	29	122	334	545	833	482	172	33	5	0	0	0	1	2703
10:00	4	8	51	340	823	920	372	92	22	5	0	4	0	0	2641
11:00	10	11	78	433	829	989	362	100	23	4	0	0	0	1	2840
12:00	242	203	499	962	686	331	79	26	3	0	0	0	0	2	3033
13:00	801	696	638	679	307	99	13	2	0	1	1	0	0	1	3238
14:00	1470	638	407	335	153	69	12	2	0	2	0	1	0	3	3092
15:00	1427	583	417	408	236	93	20	4	0	0	0	1	0	0	3189
16:00	1562	178	213	152	40	13	1	1	0	1	2	0	1	1	2165
17:00	1603	92	34	31	2	1	1	0	1	0	0	0	0	0	1765
18:00	227	241	610	1153	513	159	21	4	1	0	0	1	0	2	2932
19:00	70	173	735	1337	564	165	23	3	1	0	0	1	0	2	3074
20:00	14	82	603	1331	840	263	27	12	0	1	0	1	0	0	3174
21:00	3	15	126	562	833	979	452	120	28	4	2	1	0	0	3125
22:00	165	34	94	360	488	674	419	192	64	9	2	0	0	1	250:
23:00	212	7	35	137	382	745	520	195	81	12	1	0	0	0	232
24:00	82	2	9	23	135	454	628	449	132	45	10	1	0	1	1973
AY TOTAL	8057	2996	4704	8759	8106	9412	6497	3553	1473	595	166			20	54374
ERCENTS	14.9%	5.6%			14.9%	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		6.5%	2.7%	1.0%	0.3%	0.0%	0.0%	0.0%	

Statistical Information ...

15th Percentile Speed 30.2 mph

Median Speed 46.7 mph

10 MPH Pace Speed 45 mph to 55 mph 17518 vehicles in pace Representing 32.2% of the total vehicles 85th Percentile Speed 58.2 mph

Average Speed 44.2 mph

Vehicles > 65 MPH 2290 4.2%

#### MassDOT Highway Division SPEED SUMMARY Fri 6/21/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: ROAD TOTAL

File: comb..prn City: Quincy County: Ramp ID # 12077

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total
														66699	
01:00	15	0	3	9	54	225	406	276	147	51	13	2	0	2	1203
02:00	0	0	0	6	34	119	245	198	126	66	13	6	0	1	814
03:00	1	1	4	13	38	136	223	176	82	39	9	1	0	1	724
04:00	2	0	0	2	28	96	122	106	57	19	2	0	0	1	435
05:00	3	0	4	31	54	160	140	118	47	31	5	0	0	0	593
06:00	1	4	2	65	203	485	377	144	50	9	0	0	0	0	1340
07:00	15	18	51	115	489	891	532	166	47	11	1	2	0	0	2338
08:00	13	11	52	230	615	1091	543	183	26	4	0	0	0	1	2769
09:00	19	5	50	222	599	1095	572	148	26	15	3	0	0	2	2756
10:00	13	7	44	226	598	901	443	162	45	8	1	0	0	4	2452
11:00	15	10	91	576	930	870	379	87	20	1	1	0	0	0	2980
12:00	25	62	214	904	1160	772	200	22	4	1	1	0	0	0	3365
13:00	231	126	498	1067	845	520	132	17	3	0	1	1	0	0	3441
14:00	1760	562	396	169	49	26	6	1	2	1	2	0	0	1	2975
15:00	1782	338	329	231	80	16	6	1	0	0	0	1	0	2	2786
16:00	2296	165	21	7	1	1	1	0	1	0	0	2	1	1	2497
17:00	2233	51	12	6	2	4	2	0	1	1	1	1	0	0	2314
18:00	2041	104	42	17	4	3	2	0	1	0	1	0	0	0	2215
19:00	466	129	439	1091	642	189	30	6	0	0	2	2	0	0	2996
20:00	33	37	313	1000	962	732	164	32	3	1	1	1	0	1	3280
21:00	156	4	92	231	596	1033	551	197	48	17	0	0	0	3	2928
22:00	39	7	16	108	483	1127	639	269	69	12	0	0	0	1	2770
23:00	19	23	58	190	462	938	707	263	70	20	2	0	0	0	2752
24:00	5	0	7	76	272	887	880	367	139	31	3	1	0	0	2668
DAY TOTAL	11183	1664	2738	6592	9200	12317	7302	2939	1014	338	62	20	1	21	55391
PERCENTS	20.2%	3.1%				22.28		5.3%	1.8%	0.6%	0.1%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 22.3 mph

Median Speed 48.0 mph

10 MPH Pace Speed 45 mph to 55 mph 21517 vehicles in pace Representing 38.8% of the total vehicles 85th Percentile Speed 57.3 mph

Average Speed 43.2 mph

Vehicles > 65 MPH 1456 2.6%

#### MassDOT Highway Division SPEED SUMMARY Sat 6/22/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: ROAD TOTAL File: comb..prn City: Quincy County: Ramp ID # 12077

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Tota
		*****											******	****	
01:00	5	0	7	27	78	400	519	388	175	57	16	0	0	2	1674
02:00	3	0	1	16	65	186	316	308	180	85	8	2	1	1	1172
03:00	1	0	1	4	17	97	249	285	164	73	40	8	2	2	943
04:00	0	0	0	4	13	81	137	180	122	64	24	10	2	3	640
05:00	0	0	0	4	13	52	104	185	115	75	30	4	0	0	582
06:00	1	0	1	1	7	51	118	227	250	191	62	15	2	1	927
07:00	5	0	1	7	25	126	355	526	345	201	69	16	2	2	1680
08:00	6	0	0	17	67	.337	660	668	359	170	40	. 4	1	2	2331
09:00	6	1	8	60	211	786	, 941	667	247	58	13	1	0	0	2999
10:00	6	. 2	4	62	272	1003	1107	508	157	25	5	1	0	1	3153
11:00	14	71	222	697	1109	898	289	95	25	7	0	0	0	3	3430
12:00	16	47	311	1020	1327	757	99	25	2	2	0	1	0	1	3608
13:00	104	109	252	848	1208	908	163	22	3	0	0	2	1	2	3622
14:00	62	94	222	736	1015	994	245	48	5	3	0	0	1	2	3427
15:00	61	128	241	779	1037	911	249	56	16	6	0	2	0	3	3489
16:00	1541	813	492	218	66	14	3	2	2	0	1	0	0	2	3154
17:00	1188	440	684	593	161	48	6	5	1	1	1	1	0	1	3130
18:00	234	299	486	734	436	256	135	61	14	8	0	0	0	0	2663
19:00	22	34	71	334	663	1185	691	242	56	7	2	2	0	2	3311
20:00	85	126	173	424	747	1170	601	195	47	8	0	2	1	0	3579
21:00	7	10	46	274	647	1199	717	243	65	27	2	0	0	2	3239
22:00	201	67	153	351	619	956	477	165	37	10	4	0	0	1	3041
23:00	8	24	35	220	597	1072	670	247	52	6	3	1	1	1	2937
24:00	4	9	34	183	487	1018	771	291	76	16	6	1	0	1	2897
DAY TOTAL	3580	2274	3445	7613	10887	14505	9622	5639	2515	1100	326		14		61628
PERCENTS	5.9%	3.78				23.6%		9.1%	4.0%	1.7%	0.5%	0.1%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 39.9 mph

Median Speed 51.0 mph

10 MPH Pace Speed 45 mph to 55 mph 25392 vehicles in pace Representing 41.2% of the total vehicles 85th Percentile Speed 60.4 mph

Average Speed 49.5 mph

Vehicles > 65 MPH 4063 6.6%

-					Ma	SI	Highwa PEED SU n 6/16/	MMARY	sion					F	age:
Site Refere Site ID: St Location: I Direction: Lane: 1	ation 2 -93 SB			5			A. V. I	2		File: City: County	Quincy		12077	÷	
TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total
**********										299449					
01:00	1	0	0	7	41	224	202	141	34	11	2	2	1	1	667
02:00	1	0	0	3	35	116	163	105	56	23	3	0	0	0	505
03:00	1	0	2	8	17	64	130	85	59	27	3	1	1	1	399
04:00	1	1	3	5	14	46	72	49	36	12	5	0	0	1	245
05:00	0	0	0	2	20	44	59	44	22	7	0	0	1	0	199
06:00	1	0	1	2	5	28	67	49	43	17	4	1	0	0	218
07:00	0	1	0	5	23	30	78	102	65	36	7	3	1	0	351
08:00	0	0	1	1	13	67	154	197	115	56	11	1	0	0	616
09:00	0	4	3	2	21	159	234	201	81	29	3	0	0	0	737
10:00	6	2	2	11	71	356	336	206	60	21	3	1	0	1	1076
11:00	2	5	37	133	325	481	290	118	32	11	3	0	0	0	1437
12:00	84	98	275	506	368	217	59	15	6	0	1	0	0	0	1629
13:00	132	68	186	594	524	182	5	1	0	1	0	0	0	1	1694
14:00	417	359	350	329	145	52	5	0	0	0	0	1	0	1	1659
15:00	199	193	153	311	278	224	61	9	4	0 3	0	1	0	0	1433
16:00	243	113 100	154	204 501	165 434	226 177	141 42	74 14	6	3	0	0 1	0	1	1330
17:00	42 0	100	272 53	175	434	514	179	14 59	5 9	3	2	0	0	2	1591 1394
18:00	2	26	53 64	228	334	457	179	47	13	1	0	1	0	0	1394
19:00 20:00	. 4	26	49	110	280	496	239	47	13	6	0	0	0	0	1275
20:00	5	0	14	117	256	490	218	66	14	4	3	0	0	0	1139
22:00	2	0	5	44	182	385	247	108	29	4	0	0	0	0	1006
23:00	2	0	3	8	80	195	217	155	44	19	4	0	0	0	727
24:00	2	0	õ	4	33	141	183	130	51	14	5	0	Ő	0	563
	1147	070	1607	2210	4064	5323	2552	2046	705	206			4		22224
DAY TOTAL PERCENTS	5.0%	979 4.3%	1627	3310 14.3%	4064		3552	8.8%	795 3.4%	306 1.3%	59 0.2%	13	0.0%	0.0%	23234 100%

Statistical Information...

- 15th Percentile Speed 39.2 mph
- Median Speed 50.5 mph
- 10 MPH Pace Speed 45 mph to 55 mph 9387 vehicles in pace Representing 40.4% of the total vehicles

85th Percentile Speed 59.7 mph

Average Speed 49.0 mph

Vehicles > 65 MPH 1186 5.1%

#### MassDOT Highway Division SPEED SUMMARY Mon 6/17/2019

File: comb..prn

County: Ramp ID # 12077

City: Quincy

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH

Lane: 1															
TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total
		*****													
01:00	1	0	0	3	10	58	108	86	45	18	3	2	0	0	334
02:00	0	0	2	2	5	39	73	56	38	10	0	0	1	0	226
03:00	3	0	0	1	12	31	41	34	12	9	2	2	0	0	147
04:00	0	0	1	1	12	26	25	24	23	10	2	1	0	0	125
05:00	0	0	0	1	14	23	54	83	67	37	8	1	0	0	288
06:00	2	0	4	2	16	75	183	239	123	45	4	2	0	1	696
07:00	6	1	0	9	71	362	463	225	79	14	2	0	1	0	1233
08:00	15	12	23	43	145	527	419	126	43	11	3	0	0	0	1367
09:00	184	78	37	122	251	416	234	59	11	3	1	0	0	1	1397
10:00	4	2	6	103	289	522	330	92	23	3	2	0	0	0	1376
11:00	36	30	75	153	298	475	192	65	9	0	0	0	0	1	1334
12:00	67	72	117	310	273	370	209	64	8	3	2	0	0	0	1495
13:00	91	78	182	378	384	230	109	36	18	3	0	0	0	0	1509
14:00	63	80	207	373	353	368	116	17	0	2	0	1	0	1	1581
15:00	494	299	308	296	161	48	3	3	0	0	1	0	1	2	1616
16:00	977	229	88	68	13	1	0	2	1	2	3	0	0	1	1385
17:00	411	97	167	379	218	71	9	1	1	0	3	0	1	1	1359
18:00	15	34	186	663	419	104	20	3	0	0	0	0	0	0	1444
19:00	5	15	210	800	377	99	12	2	1	0	1	0	0	0	1522
20:00	2	1	42	256	440	467	264	74	22	5	0	1	0	0	1574
21:00	1	0	2	41	175	472	328	138	46	4	0	0	0	0	1207
22:00	3	0	4	37	167	404	325	89	31	4	2	0	0	1	1067
23:00	2	0	6	20	111	306	257	115	31	14	3	0	0	0	865
24:00	4	0	0	15	52	165	209	157	58	15	4	1	0	1	681
DAY TOTAL	2386	1028	1667	4076				1790	690	212	46	11	4		25828
PERCENTS	9.38	4.0%	6.5%	15.8%	16.68	22.0%	15.4%	6.9%	2.6%	0.8%	0.1%	0.0%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 36.4 mph

Median Speed 49.4 mph

10 MPH Pace Speed 45 mph to 55 mph 9925 vehicles in pace Representing 38.4% of the total vehicles 85th Percentile Speed 58.6 mph

Average Speed 47.0 mph

Vehicles > 65 MPH 973 3.8%

#### MassDOT Highway Division SPEED SUMMARY Tue 6/18/2019

File: comb..prn

County: Ramp ID # 12077

City: Quincy

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH Lane: 1

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Tota
		******													
01:00	1	0	0	2	17	90	111	120	36	20	2	0	0	0	399
02:00	2	0	0	3	12	38	66	66	33	17	5	0	0	0	242
03:00	0	0	1	0	13	33	44	57	27	7	2	1	0	1	186
04:00	1	0	0	0	10	21	39	37	22	11	4	0	0	0	145
05:00	0	0	5	7	16	55	81	95	68	22	8	2	0	0	359
06:00	0	0	0	13	12	91	234	198	103	34	10	0	0	0	695
07:00	8	0	6	49	149	374	360	196	50	7	4	0	0	0	1203
08:00	8	0	1	25	205	637	412	123	16	4	0	0	0	0	1431
09:00	12	1	13	97	366	535	284	82	13	1	0	0	0	0	1404
10:00	6	10	24	142	301	606	247	66	12	2	1	0	0	1	1418
11:00	10	0	5	96	362	537	252	54	11	2	0	0	0	1	1330
12:00	8	8	75	262	431	420	195	62	12	3	0	0	0	0	1476
13:00	696	332	209	98	33	16	14	10	1	1	0	0	0	5	1415
14:00	941	143	114	56	17	3	0	1	2	0	0	0	0	2	1279
15:00	1159	89	24	3	1	2	0	2	0	0	1	1	0	0	1282
16:00	1054	35	5	1	1	1	2	1	0	0	1	3	1	0	1105
17:00	1065	56	20	12	3	5	3	0	0	2	0	1	1	1	1169
18:00	343	90	327	440	145	24	1	0	0	0	0	0	1	0	1371
19:00	7	121	538	688	141	30	3	1	1	0	1	0	0	0	1531
20:00	3	36	271	750	343	179	41	6	2	1	1	1	0	1	1635
21:00	4	0	17	108	300	502	237	86	15	1	0	0	0	0	1270
22:00	1	4	72	214	186	286	208	88	13	5	0	0	0	2	1079
23:00	2	0	3	29	97	235	221	121	23	5	2	0	0	0	738
24:00	2 3	0	4	10	29	129	201	182	72	24	3	3	0	0	660
	*******														
AY TOTAL	5334	925	1734	3105	3190		3256	1654	532	169	45	12	3		24822
ERCENTS	21,5%	3.8%	7.08	12.6%	12.98	19.68	13.2%	6.6%	2.18	0.6%	0.1%	0.0%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 21.0 mph

Median Speed 47.1 mph

10 MPH Pace Speed 50 mph to 60 mph 8105 vehicles in pace Representing 32.6% of the total vehicles 85th Percentile Speed 58.0 mph

Average Speed 42.6 mph

Vehicles > 65 MPH 775 3.1%

#### MassDOT Highway Division SPEED SUMMARY Wed 6/19/2019

Page: 4

Site Reference: 190020000034 File: comb.,prn Site ID: Station 2 City: Quincy Location: I-93 SB ramp to I-95 County: Ramp ID # 12077 Direction: SOUTH Lane: 1 30 35 40 45 50 55 60 65 70 75 80 85 90 91+ Total TIME \_\_\_\_\_ 0 5 0 0 0 0 0 0 11 - 66 107 95 47 19 01:00 350 0 1 0 0 0 0 1 0 0 0 56 60 70 02:00 2 0 16 37 15 4 262 0 50 23 2 16 33 47 12 3 03:00 0 186 5 0 1 0 3 4 0 0 7 0 0 6 2 0 2 3 0 9 45 04:00 3 24 45 14 13 0 153 10 0 45 78 05:00 65 100 58 29 324 0 677 06:00 208 212 107 38 0 07:00 0 75 50 3 0 0 0 1204 0 370 481 7 211 7 
 0
 0
 0

 08:00
 124
 21

 09:00
 224
 46

 10:00
 7
 2
 69 4 1 0 0 201 1 1446 502 332 91 20 80 1 0 0 0 0 0 1 1360 70 118 230 448 180 32 9 1 7 0 1388 309 96 584 278 85 19 30 19 1 0 0 0 11:00 23 509 239 63 15 1 1353 158 292 4 235 173 173 280 257 264 67 30 5 2 0 0 0 1 1487 12:00 76 32 115 0 0 0 0 0 0 4 1618 0 1685 430 140 30 8 13:00 337 446 0 13 29 2 14:00 167 521 498 385 57 13 0 0 0 0 2 1589 505 161 11 4 1 0 15:00 13 22 214 656 3 1 5 0 1327 461 65 200 363 175 54 0 0 0 0 16:00 2 0 1423 122 19 0 130 1 0 17:00 55 226 2 0 533 333 0 1451 0 1606 1 1618 4 9 6 57 6 27 12 46 0 15 5 146 601 505 166 0 0 0 18:00 õ 0 1 19:00 315 775 366 77 1 1 2 0 0 223 20:00 726 412 185 0 1 1514 4 60 447 133 34 2 1 0 0 0 21:00 13 46 235 537 3 0 2 5 1 0 95 421 247 101 19 1 1149 0 22:00 18 237 3 0 - 0 18 3 226 96 11 39 144 324 1 1 864 23:00 4 0 18 6 0 0 0 0 954 202 347 19 94 196 68 24:00 \_\_\_\_\_ DAY TOTAL 1353 609 2061 5732 5798 6098 3190 1450 479 160 40 3 2 13 26988 5.1% 2.3% 7.7% 21.3% 21.5% 22.6% 11.9% 5.3% 1.7% 0.5% 0.1% 0.0% 0.0% 0.0% 100% PERCENTS

Statistical Information ...

15th Percentile Speed 40.0 mph

Median Speed 48.2 mph

10 MPH Pace Speed 45 mph to 55 mph 11896 vehicles in pace Representing 44.0% of the total vehicles 85th Percentile Speed 57.0 mph

Average Speed 47.4 mph

Vehicles > 65 MPH 697 2.6%

#### MassDOT Highway Division SPEED SUMMARY Thu 6/20/2019

Site Reference: 190020000034 File: comb..prn Site ID: Station 2 City: Quincy Location: I-93 SB ramp to I-95 County: Ramp ID # 12077 Direction: SOUTH 30 35 40 45 50 55 70 75 80 85 90 91+ Total 60 65 7 0 4 35 1 0 0 0 0 173 181 132 39 0 572 2 79 6 5 1 0 2 0 21 100 103 24 17 0 0 0 355 0 1 0 6 3 10 42 75 54 18 9 0 220 0 0 0 0 1 0 0 3 6 44 43 32 19 13 3 1 0 164 1 0 1 0 2 0 6 14 0 28 63 95 87 9 0 358 51 235 194 11 18 32 5 0 724 128 95 32 361 14 1 0 0 1 1 0 0 0 1181 53 126 397 169 1 0 0 0 0 0 0 1365 76 294 628 289 59 9 1 1 26 1 1321 216 345 418 136 2 0 0 4 0 261 562 401 0 1373 84 14 4 0 0 0 0 7 285 453 418 113 25 0 1357

11:00 12:00 133 105 265 524 260 102 20 5 2 0 0 0 0 2 1418 1 0 0 4 0 403 361 320 325 99 12 2 1 0 1 1529 13:00 70 17 7 2 0 1 0 0 1 0 703 326 186 153 0 0 3 1468 14:00 0 699 298 221 189 92 18 4 2 0 0 1524 15:00 68 0 1 1 0 0 0 1 1087 1 16:00 790 83 121 16 3 1 2 0 17:00 1 0 
 807
 47
 13
 132

 132
 123
 385
 608

 38
 100
 457
 676

 36
 731
 0 0 807 47 13 5 1 0 0 0 875 0 875 2 1488 2 1520 0 1496 0 0 1 0 0 1 0 192 41 3 0 18:00 1 201 41 288 58 4 0 0 2 0 19:00 0 0 366 9 1 0 7 20:00 35 731 0 0 292 2 0 0 1390 6 6 4 400 422 152 21:00 60 42 11 2 0 52 154 257 134 57 2 0 0 0 0 1 1032 21 0 22:00 159 189 0 897 7 36 4 142 59 209 239 163 0 34 23:00 0 835 61 42 14 2 0 0 1 4 81 1 183 275 171 24:00 \_\_\_\_\_ 4267 1521 2646 4688 3894 4185 2486 1236 412 148 38 12 1 15 25549 DAY TOTAL 16.8% 6.0% 10.4% 18.4% 15.3% 16.4% 9.7% 4.8% 1.6% 0.5% 0.1% 0.0% 0.0% 0.0% 100% PERCENTS

Statistical Information ...

Lane: 1

TIME

01:00

02:00

03:00

04:00

05:00

06:00

07:00

08:00

09:00

10:00

15th Percentile Speed 27.0 mph

Median Speed 44.6 mph

10 MPH Pace Speed 40 mph to 50 mph 8582 vehicles in pace Representing 33.5% of the total vehicles 85th Percentile Speed 56.0 mph

Average Speed 42.3 mph

Vehicles > 65 MPH 626 2.5%

#### MassDOT Highway Division SPEED SUMMARY Fri 6/21/2019

File: comb..prn

County: Ramp ID # 12077

City: Quincy

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH Lane: 1

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total
01:00	13		0	7	33	112	178	110	42	14	1		0	1	511
02:00	0	0	0	5	15	66	117	69	42 51	19	2	4	0	0	348
02:00	1	0	2	6	19	58	103	54	36	14	0	1	0	0	294
04:00	2	0	0	2	17	47	48	40	17	3	1	0	0	0	177
05:00	3	0	3	17	35	97	68	52	15	10	0	0	0	0	300
06:00	- 1	4	2	40	134	255	159	40	11	1	0	0	0	0	647
07:00	13	12	30	74	317	426	175	43	16	2	0	1	0	0	1109
08:00	12	11	40	153	409	523	157	37	7	1	0	0	õ	1	1351
09:00	18	5	45	179	411	511	140	24	5	2	3	0	0	ō	1343
10:00	10	2	32	172	418	460	94	19	7	0	0	0	0	1	1215
11:00	12	2	40	357	503	354	76	15	2	0	1	0	0	0	1362
12:00	12	31	133	556	565	229	35	4	2	0	0	0	0	0	1567
13:00	133	75	261	554	381	151	22	2	1	0	1	1	0	0	1582
14:00	865	300	183	79	16	10	5	1	2	1	2	0	0	1	1465
15:00	881	170	153	107	27	4	4	. 0	0	0	0	1	0	2	1349
16:00	1141	59	6	5	1	0	1	0	- 1	0	0	2	1	1	1218
17:00	1095	17	5	2	2	2	1	0	1	1	1	1	0	0	1128
18:00	1021	46	12	6	3	3	2	0	1	0	1	0	0	0	1095
19:00	267	49	242	560	267	61	3	1	0	0	0	2	0	0	1452
20:00	31	22	173	525	439	231	41	6	1	0	0	1	0	1	1471
21:00	154	1	20	95	239	406	179	66	13	2	0	0	0	3	1178
22:00	29	2	5	60	239	488	235	101	19	2	0	0	0	0	1180
23:00	9	4	17	62	214	447	306	90	22	5	0	0	0	0	1176
24:00	3	0	4	25	119	380	344	164	53	6	0	0	0	0	1098
DAY TOTAL	5726	812	1408	3648	4823	5321	2493	938	325	83	13	14	1	11	25616
PERCENTS	22.4%	3.2%			18.9%		9.8%	3.6%	1.28	0.3%	0.08	0.08	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 20.1 mph

Median Speed 46.3 mph

10 MPH Pace Speed 45 mph to 55 mph 10144 vehicles in pace Representing 39.6% of the total vehicles 85th Percentile Speed 55.1 mph

Average Speed 41.4 mph

Vehicles > 65 MPH 447 1.7%

Page: 6

#### MassDOT Highway Division SPEED SUMMARY Sat 6/22/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH

Lane: 1

File: comb..prn City: Quincy County: Ramp ID # 12077

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Tota
						assa.									
01:00	4	0	0	4	45	196	204	137	67	22	5	0	0	1	685
02:00	2	0	1	8	26	95	130	118	71	20	3	1	0	0	475
03:00	1	0	0	3	12	50	118	114	53	25	16	2	0	1	395
04:00	0	0	0	3	5	46	66	70	32	29	7	3	1	3	265
05:00	0	0	0	4	8	22	56	74	39	24	10	2	0	0	239
06:00	1	0	1	1	6	29	65	94	82	58	15	2	0	0	354
07:00	4	0	0	4	19	85	159	214	105	56	23	4	0	1	674
08:00	3	0	0	8	39	180	310	264	120	49	7	1	0	1	982
09:00	4	1	7	26	103	437	356	236	93	18	4	0	0	0	1285
10:00	2	2	3	39	138	490	457	175	40	7	1	0	0	1	1355
11:00	6	29	131	417	543	321	108	34	4	2	0	0	0	1	1596
12:00	9	24	150	598	637	260	15	5	0	2	0	1	0	0	1701
13:00	53	70	149	480	620	305	23	7	1	0	0	0	1	1	1710
14:00	16	43	108	436	496	387	71	8	2	1	0	0	0	0	1568
15:00	20	68	132	472	561	305	58	17	4	1	0	0	0	0	1638
16:00	706	435	229	100	20	4	1	1	1	0	1	0	0	2	1500
17:00	561	260	335	284	47	9	2	0	1	1	0	0	Ó	1	1501
18:00	114	163	265	429	224	97	36	19	5	2	0	0	0	0	1354
19:00	4	11	31	180	374	558	277	65	7	1	1	0	0	0	1509
20:00	40	52	82	235	401	495	214	50	9	0	0	1	1	0	1580
21:00	3	3	21	111	348	539	265	93	28	11	1	0	0	0	1423
22:00	61	39	70	169	289	405	207	62	13	3	1	0	0	1	1320
23:00	4	0	3	84	302	473	260	83	26	3	0	1	1	1	1241
24:00	2	0	7	82	246	428	299	118	29	8	1	0	0	1	1221
DAY TOTAL	1620	1200	1725	4177	5509	6216	3757	2058	832	343	96		4	16	27571
PERCENTS	5.9%	4.4%		15.2%				2058	3.0%	1.2%	0.3%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 38.8 mph

Median Speed 49.6 mph

10 MPH Pace Speed 45 mph to 55 mph 11725 vehicles in pace Representing 42.5% of the total vehicles 85th Percentile Speed 59.0 mph

Average Speed 48.2 mph

Vehicles > 65 MPH 1309 4.7%

# MassDOT Highway Division SPEED SUMMARY Sun 6/16/2019

STA.2 Site Reference: 190020000034 File: comb..prn Site ID: Station 2 LNI.2 City: Quincy Location: I-93 SB ramp to I-95 County: Ramp ID # 12077 Direction: SOUTH 30 35 40 45 50 55 60 65 70 75 80 85 90 91+ Total 50 82 29 0 0 0 3 15 199 310 219 11 0 0 918 3 0 0 918 3 0 0 740 2 0 4 581 1 1 362 5 4 0 1 0 4 
 84
 42

 102
 47

 68
 45

 40
 12
 180 84 178 102 0 37 145 1 227 12 151 20 68 6 1 1 1 9 0 0 0 101 61 92 246 0 9 13 2 0 0 0 0 0 12 2 0 43 115 276 577 508 244 103 28 4 3 0 5 27 0 1933 

 12:00
 115
 130
 213
 388
 474
 439
 184
 39
 7
 0
 0
 0
 0
 1933

 12:00
 115
 130
 213
 388
 474
 439
 184
 39
 7
 0
 0
 0
 0
 1933

 13:00
 130
 77
 147
 344
 635
 486
 99
 3
 2
 2
 0
 0
 0
 1933

 14:00
 454
 312
 312
 372
 255
 136
 18
 2
 0
 0
 1
 0
 1
 1863

0

2

3

47

218 139 39

32

57 10 57 10 1 7 0 1719 0 1679 1 1616 0 1461 474 18:00 4 2 287 604 126 25 0 3 0 7 5 8 8 8 0 109 57 130 79 188 0 527 
 3
 0
 0

 1
 0
 0

 3
 1
 0

 0
 0
 0
 19:00 368 331 15 118 603 .337 20:00 136 307 32 105 278 201 0 9 2 0 0 1 0 0 505 352 21:00 144 29 156 - 69 2 2 1337 1 932 446 400 22:00 42 9 62 18 0 23:00 206 325 214 1 81 1 22 176 251 200 95 23 5 1 0 0 813 39 24:00 DAY TOTAL 1299 983 1340 2666 4471 6707 5674 3686 1776 778 236 39 10 13 29678 PERCENTS 4.4% 3.4% 4.6% 9.0% 15.1% 22.6% 19.2% 12.4% 5.9% 2.6% 0.7% 0.1% 0.0% 0.0% 100%

140

366 211

251

474

Statistical Information...

Lane: 2

TIME

01:00

02:00

03:00

04:00

05:00

06:00

07:00

08:00 09:00 10:00

11:00

17:00

15th Percentile Speed 41.6 mph

45431231215:0022216814616:00251105

60 83

Median Speed 53.1 mph

10 MPH Pace Speed 50 mph to 60 mph 12381 vehicles in pace Representing 41.7% of the total vehicles

203

148

137

304

185

557

85th Percentile Speed 62.8 mph

Average Speed 51.7 mph

2 0 0 0 0 0 1 0 4 0 0 0 13 3 0 0

1 0 0

Vehicles > 65 MPH 2852 9.68

Page: 8

0 1673

0 1485

0 1886

# MassDOT Highway Division SPEED SUMMARY Mon 6/17/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH

Lane: 2

File: comb..prn City: Quincy County: Ramp ID # 12077

					201		22				1.1	2.5	14			
TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total	ł
01:00	1	1	0	1	10	43	133	155	93	33	7	0	0	0	477	
02:00	0	0	0	0	1	34	94	95	62	33	7	3	0	1	330	
03:00	0	0	0	5	7	25	52	68	39	16	5	3	1	0	221	
04:00	0	0	0	1	4	19	29	66	53	18	6	2	0	1	199	
05:00	0	0	0	0	4	16	42	88	98	49	28	8	0	2	335	
06:00	0	0	0	0	5	22	115	274	230	125	42	9	2	· 1	825	
07:00	0	0	0	1	23	202	427	433	184	62	15	0	0	0	1347	
08:00	2	5	12	28	44	230	479	393	141	51	16	0	0	0	1401	
09:00	176	66	44	60	102	312	363	256	71	9	0	0	0	1	1460	
10:00	0	0	2	47	135	330	424	303	101	20	5	1	0	1	1369	
11:00	36	25	34	63	195	481	443	194	59	18	1	0	0	2	1551	
12:00	70	69	107	253	297	426	284	172	51	14	1	0	0	0	1744	
13:00	96	129	148	242	372	474	246	124	52	8	1	1	1	0	1894	
14:00	97	100	209	321	374	471	267	74	26	9	2	1	0	1	1952	
15:00	537	288	243	347	236	133	27	4	3	0	2	0	0	0	1820	
16:00	983	217	147	63	24	7	0	1	0	0	0	0	0	1	1443	
17:00	356	138	130	223	212	145	43	8	2	0	0	1	0	1	1259	
18:00	. 6	24	88	397	384	236	53	17	2	2	2	0	0	0	1211	
19:00	0	22	154	487	425	256	49	8	0	0	2	0	0	0	1403	
20:00	1	1	39	185	328	667	461	203	41	12	2	0	0	0	1940	
21:00	1	2	7	48	150	481	559	254	56	17	3	2	0	1	1581	
22:00	3	2	10	68	151	467	459	187	53	14	6	0	0	0	1420	
23:00	3	2	8	22	95	385	319	217	65	21	2	0	1	0	1140	
24:00	1	0	0	14	39	193	308	258	117	48	7	0	0	1	986	
DAY TOTAL	2369	1091	1382	2876	3617	6055	5676	3852	1599	579	162	31	 5		29308	2
PERCENTS	8.1%	3.8%	4.8%		12.4%		19.3%		5.48	1.9%	0.5%	0.1%	0.0%	0.0%	100%	

Statistical Information ...

15th Percentile Speed 38.4 mph

Median Speed 52.8 mph

10 MPH Pace Speed 50 mph to 60 mph 11731 vehicles in pace Representing 40.0% of the total vehicles 85th Percentile Speed 62.4 mph

Average Speed 50.1 mph

Vehicles > 65 MPH 2390 8.2%

#### MassDOT Highway Division SPEED SUMMARY Tue 6/18/2019

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH Lane: 2 File: comb..prn City: Quincy County: Ramp ID # 12077

TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total
	ويوفي ومرو			******						*****			******		
01:00	0	0	1	4	12	79	154	151	73	32	9	0	0	1	516
02:00	0	0	1	1	10	40	97	117	71	20	15	1	0	1	374
03:00	0	0	0	0	6	28	66	72	51	25	8	1	0	0	257
04:00	1	0	1	0	2	14	51	67	37	27	13	2	0	1	216
05:00	0	0	0	1	6	18	81	111	73	51	20	5	3	0	369
06:00	0	0	0	1	0	37	126	252	224	118	32	6	1	2	799
07:00	0	0	0	10	62	257	460	390	192	66	17	3	0	1	1458
08:00	3	0	0	13	47	366	575	398	130	32	7	1	0	0	1572
09:00	0	2	8	31	91	455	570	271	85	19	2	0	0	0	1534
10:00	3	2	10	. 48	157	419	467	267	76	19	0	1	0	0	1469
11:00	0	0	3	52	180	504	487.	222	68	7	2	0	0	0	1525
12:00	3	10	55	206	450	563	354	167	19	9	1	0	0	0	1837
13:00	745	378	177	146	56	42	28	12	4	1	0	0	0	0	1589
14:00	1015	135	145	96	27	4	0	0	1	0	0	0	0	0	1423
15:00	1188	100	45	13	1	1	1	0	0	0	0	0	0	0	1349
16:00	1120	41	20	3	1	0	1	0	0	0	0	0	0	0	1186
17:00	1169	37	9	15	6	3	1	0	0	0	0	0	0	0	1240
18:00	325	97	235	369	161	59	7	1	0	0	0	0	0	0	1254
19:00	17	69	358	729	305	88	12	0	1	0	0	1	1	0	1581
20:00	5	28	316	710	481	324	78	12	7	3	0	1	0	0	1965
21:00	3	7	17	89	283	640	439	168	59	7	2	0	0	0	1714
22:00	7	1	90	240	200	388	303	139	33	11	1	0	0	0	1413
23:00	• 2	2	5	18	112	379	366	174	54	15	6	0	0	0	1133
24:00	0	1	6	7	46	218	337	272	95	47	10	1	0	0	1040
DAY TOTAL	5606	910	1502	2802	2702	4926	5061	3263	1353	509	145	23	5	6	28813
PERCENTS	19.5%	3.2%	5.3%	9.8%		17.1%			4.6%	1.7%	0.5%	0.0%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 23.1 mph

Median Speed 50.9 mph

10 MPH Pace Speed 50 mph to 60 mph 9987 vehicles in pace Representing 34.6% of the total vehicles 85th Percentile Speed 61.5 mph

Average Speed 45.6 mph

Vehicles > 65 MPH 2041 7.1%

#### MassDOT Highway Division SPEED SUMMARY Wed 6/19/2019

File: comb..prn

County: Ramp ID # 12077

City: Quincy

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH

Lane: 2

Lane: 2															
TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total
									وم م م م	بدمجم					
01:00	1	0	0	0	25	71	167	163	92	49	12	2	1	0	583
02:00	0	0	0	2	3	66	131	131	69	30	10	4	1	0	447
03:00	0	0	0	2	4	24	65	74	46	19	11	1	0	2	248
04:00	0	0	0	2	10	27	43	77	44	30	7	2	1	0	243
05:00	0	0	0	0	4	18	62	106	74	50	31	6	-2	0	353
06:00	1	0	0	2	3	24	140	266	239	118	23	7	0	0	823
07:00	0	0	0	9	32	244	463	406	159	50	12	2	0	0	1377
08:00	116	26	25	59	124	347	477	260	82	23	3	0	0	0	1542
09:00	225	59	47	79	99	299	411	171	44	9	3	2	1	3	1452
10:00	24	13	5	30	128	376	501	221	60	19	2	0	0	1	1380
11:00	24	27	31	75	182	536	472	199	58	11	3	1	0	0	1619
12:00	215	212	215	248	297	380	175	56	11	5	3	0	0	0	1817
13:00	87	134	118	296	379	553	312	91	25	3	2	1	0	0	2001
14:00	14	28	133	430	522	604	203	52	12	2	1	2	1	0	2004
15:00	0	13	134	446	656	415	66	16	1	0	0	0	1	0	1748
16:00	471	32	145	366	206	115	21	7	1	2	0	0	2	0	1368
17:00	85	64	137	373	321	197	51	24	4	0	0	0	0	0	1256
18:00	9	11	59	294	405	318	111	25	7	3	0	0	. 0	0	1242
19:00	29	77	244	605	478	182	43	8	2	0	2	0	0	1	1671
20:00	10	43	203	606	626	358	93	26	10	1	0	0	0	0	1976
21:00	25	45	83	278	453	586	239	107	16	3	0	1	0	0	1836
22:00	0	1	16	126	274	558	345	165	49	17	3	1	0	1	1556
23:00	1	1	8	25	90	273	349	207	61	10	2	0	0	0	1027
24:00	2	4	15	59	154	307	242	143	29	8	1	0	1	1	966
DAY TOTAL	1339	790	1618	4412	5475	6878	5182	3001	1195	462	131				30535
PERCENTS	4.4%	2.6%		14.5%				9.8%	3.9%	1.5%	0.4%	0.1%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 41.0 mph

Median Speed 51.2 mph

10 MPH Pace Speed 45 mph to 55 mph 12353 vehicles in pace Representing 40.4% of the total vehicles 85th Percentile Speed 60.4 mph

Average Speed 50,1 mph

Vehicles > 65 MPH 1840 6.0%

#### MassDOT Highway Division SPEED SUMMARY Thu 6/20/2019

File: comb..prn

County: Ramp ID # 12077

City: Quincy

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH Lane: 2

35 TIME 30 40 45 50 55 60 65 70 75 80 85 90 91+ Total 27 5 1 0 0 10 127 01:00 1 0 0 219 178 73 0 641 0 0 1 11 51 0 1 9 24 0 2 2 1 431 1 314 124 122 63 02:00 46 3 2 0 0 80 96 61 27 59 66 41 29 03:00 30 59 0 236 04:00 0 0 0 1 0 1 0 0 386 6 22 60 113 96 58 26 05:00 2 13 55 137 224 242 124 51 7 1 1 859 06:00 0 1372 52 283 492 323 150 57 10 2 0 07:00 1 0 0 0 0 0 0 08:00 0 1425 09:00 0 1382 

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 261
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 376
 571
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 98
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 200
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 208
 87
 9

 0 1268 79 261 519 288 78 18 5 0 0 0 10:00 11:00 249 75 16 4 0 0 0 1 1483 12:001099823443813:0039833531835414:0076731222118215:00728285196219 0 21 1 0 0 0 0 1615 9 0 0 0 0 0 0 0 0 0 1709 0 1624 208 87 2 2 0 0 83 52 5 0 15:00 144 0 0 0 0 75 16 0 0 0 1665 24 0 1078 0 772 95 796 45 10 1 0 0 0 16:00 0 92 84 21 0 0 0 0 0 0 0 0 890 0 1444 17:00 26 2 0 0 0 0 225 545 321 73 278 661 363 47 237 95 32 5 2 5 95 118 225 0 118 18 4 18:00 

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 0 1554 19:00 124 0 1334 0 1678 0 1735 0 1470 47 237 600 552 20:00 205 66 270 433 42 206 299 9 557 21:00 22:00 6 28 417 3 28 101 240 506 0 1430 23:00 357 136 47 8 1 0 0 3 1 1 8 19 74 271 353 278 90 31 8 1 0 1 1136 24:00 \_\_\_\_\_ 3790 1475 2058 4071 4212 5227 4011 2317 1061 447 128 21 2 5 28825 DAY TOTAL 13.2% 5.2% 7.2% 14.2% 14.7% 18.1% 13.9% 8.0% 3.6% 1.5% 0.4% 0.0% 0.0% 0.0% 100% PERCENTS

Statistical Information ....

15th Percentile Speed 31.8 mph

Median Speed 48.6 mph

10 MPH Pace Speed 45 mph to 55 mph 9439 vehicles in pace Representing 32.7% of the total vehicles 85th Percentile Speed 59.6 mph

Average Speed 45.8 mph

Vehicles > 65 MPH 1664 5.8%

# MassDOT Highway Division SPEED SUMMARY Fri 6/21/2019

File: comb..prn

County: Ramp ID # 12077

City: Quincy

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH Lane: 2

Danc. 2															
TIME	30	35	40	45	50	55	60	65	70	75	80	85	90	91+	Total
				83889.	******										
01:00	2	0	3	2	21	113	228	166	105	37	12	2	0	1	692
02:00	0	0	0	1	19	53	128	129	75	47	11	2	0	1	466
03:00	0	1	2	7	19	78	120	122	46	25	9	0	0	1	430
04:00	0	0	0	0	11	49	74	66	40	16	1	0	0	- 1	258
05:00	0	0	1	14	19	63	72	66	32	21	5	0	0	0	293
06:00	0	0	0	25	69	230	218	104	39	8	0	0	0	0	693
07:00	2	6	21	41	172	465	357	123	31	9	1	1	0	0	1229
08:00	1	0	12	77	206	568	386	146	19	3	0	0	0	0	1418
09:00	1	0	5	43	188	584	432	124	21	13	0	0	0	2	1413
10:00	3	5	12	54	180	441	349	143	38	8	1	0	0	3	1237
11:00	3	8	51	219	427	516	303	72	18	1	0	0	0	0	1618
12:00	13	31	81	348	595	543	165	18	2	1	1	0	0	0	1798
13:00	98	51	237	513	464	369	110	15	2	0	0	0	0	0	1859
14:00	895	262	213	90	33	16	1	0	0	0	0	0	0	0	1510
15:00	901	168	176	124	53	12	2	1	0	0	0	0	0	0	1437
16:00	1155	106	15	2	0	1	0	0	0	0	0	0	0	0	1279
17:00	1138	34	7	4	0	2	1	0	0	0	0	0	0	0	1186
18:00	1020	58	30	11	1	0	0	0	0	0	0	0	0	0	1120
19:00	199	80	197	531	375	128	27	5	0	0	2	0	0	0	1544
20:00	2	15	140	475	523	501	123	26	2	1	1	0	0	0	1809
21:00	2	3	72	136	357	627	372	131	35	15	0	0	0	0	1750
22:00	10	5	11	48	244	639	404	168	50	10	0	0	0	1	1590
23:00	10	19	41	128	248	491	401	173	48	15	2	0	0	0	1576
24:00	2	0	3	51	153	507	536	203	86	25	3	1	0	0	1570
DAY TOTAL	5457	852	1330	2944	4377	6996	4809	2001	689	255	49	6	0	10	29775
PERCENTS	18.4%	2.9%	4.5%		14.8%			6.78	2.3%	0.8%	0.1%	0.0%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 24.6 mph

Median Speed 49.9 mph

10 MPH Pace Speed 50 mph to 60 mph 11805 vehicles in pace Representing 39.6% of the total vehicles 85th Percentile Speed 58.5 mph

Average Speed 44.7 mph

Vehicles > 65 MPH 1009 3.4%

# MassDOT Highway Division SPEED SUMMARY Sat 6/22/2019

File: comb..prn

County: Ramp ID # 12077

City: Quincy

Site Reference: 190020000034 Site ID: Station 2 Location: I-93 SB ramp to I-95 Direction: SOUTH Lane: 2

TIME 30 35 40 45 50 55 60 65 70 75 80 85 90 91+ Total 35 1 0 35 65 11 0 0 1 989 01:00 204 315 251 108 39 91 5 47 190 109 171 111 1 697 02:00 186 5 1 6 2 1 1 48 1 548 03:00 131 24 110 90 35 111 76 51 7 1 8 35 71 0 375 17 04:00 

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 06:00 2 1 1006 07:00 3 1 1 0 08:00 1 1349 0 1714 9 1 0 4 1 0 34 108 349 585 431 154 40 09:00 0 1798 23 134 513 650 333 117 18 10:00 8 42 91 280 566 577 181 61 21 5 0 0 0 2 1834 11:00 

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Statistical Information ...

15th Percentile Speed 40.5 mph

Median Speed 52.1 mph

10 MPH Pace Speed 50 mph to 60 mph 14154 vehicles in pace Representing 41.5% of the total vehicles 85th Percentile Speed 61.7 mph

Average Speed 50.5 mph

Vehicles > 65 MPH 2754 8.1%

# APPENDIX C

1. Crash tables

	A	В	G	Н	J	К	М	N	0	Q	Т	U
1	Study Location	Area of Crash	Crash Time	ls Peak?	Road Surface Conditions	Ambient Light Conditions	Manner of Collision	Crash Severity	Weather Conditions	Crash Numbe	r Crash Year	Crash Date
2	Wilmington	I-93 Segment 7 (after second Exit 41 on-ramp)	1:40 PM	Off-peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	3752697	2014	2014-03-04
3	Wilmington	I-93 Segment 7 (after second Exit 41 on-ramp)	12:25 PM	Off-peak	Unknown	Not reported	Not reported	Property damage only (none injured)	Unknown	3792552	2014	2014-03-21
4	Wilmington	I-93 Segment 7 (after second Exit 41 on-ramp)	8:45 AM	Peak	Wet	Daylight	Sideswipe, same direction	Property damage only (none injured)	Snow	3680696	2013	2013-12-09
5	Wilmington	Exit 40 merge	9:33 AM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Cloudy	4038992	2015	2015-05-02
6	Wilmington	Exit 40 merge	10:00 PM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Non-fatal injury	Clear	4058519	2015	2015-05-22
7	Wilmington	Exit 40 merge	3:20 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4061157	2015	2015-07-07
8	Wilmington	Exit 40 merge	12:51 PM	Off-peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	4164171	2016	2016-03-07
9	Wilmington	Exit 40 merge	12:50 PM	Off-peak	Dry	Daylight	Single vehicle crash	Non-fatal injury	Clear	3667330	2013	2013-11-26
10	Wilmington	Route 125 at Ballardville St	12:08 PM	Off-peak	Wet	Daylight	Angle	Property damage only (none injured)	Cloudy	4154894	2016	2016-02-16
11	Wilmington	Route 125 at Ballardville St	2:41 PM	Off-peak	Wet	Daylight	Unknown	Property damage only (none injured)	Clear	4301334	2016	2016-12-12
12	Wilmington	Route 125 at Ballardville St	2:39 PM	Off-peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Rain	3999257	2015	2015-01-15
13	Wilmington	Route 125 at Ballardville St	8:26 AM	Peak	Wet	Daylight	Rear-end	Non-fatal injury	Cloudy	3367293	2012	2012-12-05
14	Wilmington	Route 125 at Ballardville St	3:38 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4096845	2015	2015-10-08
15	Wilmington	Route 125 at Ballardville St	9:09 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4141225	2016	2016-01-21
16	Wilmington	Route 125 at Ballardville St	8:04 AM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	4187516	2016	2016-04-29
17	Wilmington	Route 125 at Ballardville St	5:25 PM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	4245639	2016	2016-08-30
18	Wilmington	Route 125 at Ballardville St	8:41 AM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	4284384	2016	2016-11-14
19	Wilmington	Route 125 at Ballardville St	1:58 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	3367318	2013	2013-01-21
20	Wilmington	Route 125 at Ballardville St	1:11 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3829970	2014	2014-06-01
21	Wilmington	Route 125 at Ballardville St	9:20 AM	Peak	Dry	Daylight	Sideswipe, opposite direction	Property damage only (none injured)	Cloudy	4191147	2016	2016-05-13
22	Wilmington	Route 125 at Ballardville St	9:40 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4218786	2016	2016-06-30
23	Wilmington	Route 125 at Ballardville St	8:40 AM	Peak	Wet	Daylight	Single vehicle crash	Property damage only (none injured)	Cloudy	2934893	2012	2012-02-17
24	Wilmington	Route 125 at Ballardville St	10:23 AM	Off-peak	Wet	Daylight	Angle	Property damage only (none injured)	Rain	3116421	2012	2012-05-08
25	Wilmington	Route 125 at Ballardville St	11:03 AM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Cloudy	3412964	2013	2013-03-18
26	Wilmington	Route 125 at Ballardville St	6:21 AM	Peak	Dry	Dawn	Sideswipe, same direction	Property damage only (none injured)	Clear	3705609	2013	2013-10-28
	Wilmington	Route 125 at Ballardville St	8:00 AM	Peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Clear	3671711	2013	2013-12-04
28	Wilmington	Route 125 at Ballardville St	5:05 PM	Peak	Dry	Dusk	Rear-end	Non-fatal injury	Clear	3730942	2014	2014-02-04
	Wilmington	Route 125 at Ballardville St	4:46 PM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Unknown	3743372	2014	2014-02-21
30	Wilmington	Route 125 at Ballardville St	8:49 AM	Peak	Dry	Daylight	Angle	Non-fatal injury	Clear	3818287	2014	2014-04-24
	Wilmington	Route 125 at Ballardville St	8:55 AM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	3949949	2014	2014-09-09
	Wilmington	Route 125 at Ballardville St	8:00 AM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	4041862	2015	2015-05-14
	Wilmington	Route 125 at Ballardville St	9:25 PM	Off-peak	Dry	Dark - unknown roadway	Rear-end	Non-fatal injury	Clear	4058693	2015	2015-06-23
	Wilmington	Route 125 at Ballardville St	1:50 PM	Off-peak	Dry	lighting Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	4061355	2015	2015-07-10
	Wilmington	Route 125 at Ballardville St	8:11 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4132764	2016	2016-01-05
	Wilmington	Route 125 at Ballardville St	2:12 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4149330	2016	2016-01-27
	Wilmington	Route 125 at Ballardville St	7:25 PM	Off-peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	4219262	2016	2016-06-29

	А	В	G	н	J	К	М	N	0	Q	Т	U
1	Study Location	Area of Crash	Crash Time	Is Peak?	Road Surface Conditions	Ambient Light Conditions	Manner of Collision	Crash Severity	Weather Conditions	Crash Numbe	Crash Year	Crash Date
38	Wilmington	Route 125 at Ballardville St	4:00 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4226863	2016	2016-07-23
39	Wilmington	Route 125 at Ballardville St	12:12 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4245641	2016	2016-08-31
40	Wilmington	Route 125 at Ballardville St	5:04 PM	Peak	Dry	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Clear	4301328	2016	2016-12-09
41	Wilmington	Route 125 at Ballardville St	9:22 AM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	4284379	2016	2016-11-10
42	Wilmington	Route 125 at Ballardville St	4:57 PM	Peak	Dry	Dusk	Rear-end	Non-fatal injury	Clear	4288113	2016	2016-11-18
43	Wilmington	Route 125 at Ballardville St	1:48 PM	Off-peak	Wet	Daylight	Rear-end	Non-fatal injury	Cloudy	3968377	2014	2014-10-01
44	Wilmington	Route 125 at Ballardville St	7:55 AM	Peak	Wet	Daylight	Rear-end	Non-fatal injury	Clear	3298967	2012	2012-10-05
45	Wilmington	Route 125 at Ballardville St	8:35 PM	Off-peak	Dry	Dark - roadway not lighted	Head-on	Fatal injury	Cloudy	3374720	2013	2013-03-03
46	Wilmington	Route 125 at I-93 NB ramps	3:10 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3829341	2014	2014-02-14
47	Wilmington	Route 125 at I-93 NB ramps	4:07 AM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Property damage only (none injured)	Clear	3509772	2013	2013-06-26
48	Wilmington	Route 125 at I-93 NB ramps	5:34 PM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	3412999	2013	2013-04-24
49	Wilmington	I-93 Segment 6 (before second Exit 41 on-ramp)	9:20 PM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Non-fatal injury	Unknown	3729200	2014	2014-01-16
50	Wilmington	Route 125 at I-93 NB ramps	5:13 PM	Peak	Dry	Dark - roadway not lighted	Rear-end	Non-fatal injury	Clear	4127100	2015	2015-12-16
51	Wilmington	I-93 Segment 5 (before first Exit 41 on-ramp)	4:30 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4010731	2015	2015-02-20
52	Wilmington	Route 125 at I-93 NB ramps	8:35 AM	Peak	Wet	Daylight	Single vehicle crash	Property damage only (none injured)	Unknown	3414235	2013	2013-05-11
53	Wilmington	Route 125 at I-93 NB ramps	5:45 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4259043	2016	2016-10-04
54	Wilmington	Route 125 at I-93 NB ramps	6:00 PM	Peak	Dry	Dusk	Angle	Property damage only (none injured)	Clear	3984649	2014	2014-12-03
55	Wilmington	I-93 Segment 5 (before first Exit 41 on-ramp)	6:40 PM	Peak	Dry	Dark - roadway not lighted	Rear-end	Property damage only (none injured)	Cloudy	4109972	2015	2015-11-13
56	Wilmington	I-93 Segment 5 (before first Exit 41 on-ramp)	1:33 PM	Off-peak	Wet	Daylight	Single vehicle crash	Property damage only (none injured)	Unknown	3606315	2013	2013-10-04
57	Wilmington	I-93 Segment 5 (before first Exit 41 on-ramp)	3:08 AM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Non-fatal injury	Clear	3274436	2012	2012-10-12
58	Wilmington	Exit 40 off-ramp	8:51 AM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Cloudy	3375560	2012	2012-04-02
59	Wilmington	Exit 40 off-ramp	8:30 AM	Peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Cloudy	3279863	2012	2012-10-03
60	Wilmington	I-93 Segment 4 (after Exit 41 off-ramp)	3:15 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Unknown	2869268	2012	2012-01-20
61	Wilmington	I-93 Segment 4 (after Exit 41 off-ramp)	4:30 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	2914945	2012	2012-02-01
62	Wilmington	Exit 41 diverge	3:50 AM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Property damage only (none injured)	Clear	3868054	2014	2014-06-07
63	Wilmington	Exit 41 diverge	2:34 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4263092	2016	2016-10-14
64	Wilmington	Exit 41 diverge	3:30 PM	Peak	Snow/Ice	Daylight	Single vehicle crash	Non-fatal injury	Snow	3375544	2012	2012-01-21
65	Wilmington	Exit 41 diverge	4:28 PM	Peak	Dry	Daylight	Single vehicle crash	Not Reported	Unknown	3101966	2012	2012-05-17
66	Wilmington	Exit 41 diverge	12:45 PM	Off-peak	Dry	Daylight	Single vehicle crash	Non-fatal injury	Clear	3210661	2012	2012-07-17
67	Wilmington	Exit 41 diverge	4:06 PM	Peak	Dry	Daylight	Single vehicle crash	Non-fatal injury	Clear	3266952	2012	2012-09-15
68	Wilmington	Exit 41 diverge	4:15 PM	Peak	Dry	Dark - roadway not lighted	Rear-end	Property damage only (none injured)	Clear	3290863	2012	2012-11-15
69	Wilmington	Exit 41 diverge	8:25 PM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Property damage only (none injured)	Cloudy	3317430	2012	2012-12-18
70	Wilmington	Exit 41 diverge	2:09 PM	Off-peak	Dry	Daylight	Single vehicle crash	Non-fatal injury	Clear	3452992	2013	2013-05-18
71	Wilmington	Exit 41 diverge	1:30 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3584857	2013	2013-09-08
72	Wilmington	Exit 41 diverge	3:10 PM	Peak	Snow/Ice	Daylight	Single vehicle crash	Non-fatal injury	Snow	3371847	2013	2013-03-19
73	Wilmington	Exit 41 diverge	3:36 PM	Peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Cloudy	3372959	2013	2013-03-06

	А	В	G	н	J	К	М	N	0	Q	Т	U
1	Study Location	Area of Crash	Crash Time	Is Peak?	Road Surface Conditions	Ambient Light Conditions	Manner of Collision	Crash Severity	Weather Conditions	Crash Number	Crash <sup>r</sup> Year	Crash Date
74	Wilmington	Exit 41 diverge	9:00 AM	Peak	Snow/Ice	Daylight	Angle	Property damage only (none injured)	Snow	3371276	2013	2013-03-08
75	Wilmington	Exit 41 diverge	2:15 AM	Off-peak	Dry	Dark - roadway not lighted	Rear-end	Non-fatal injury	Clear	3369869	2013	2013-03-17
76	Wilmington	Exit 41 diverge	4:47 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Cloudy	3379666	2013	2013-03-29
77	Wilmington	Exit 41 diverge	2:10 PM	Off-peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	3394427	2013	2013-04-10
78	Wilmington	Exit 41 diverge	2:10 PM	Off-peak	Dry	Daylight	Angle	Non-fatal injury	Clear	3425153	2013	2013-05-07
79	Wilmington	Exit 41 diverge	2:21 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3439969	2013	2013-05-27
80	Wilmington	Exit 41 diverge	5:20 PM	Peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Rain	3501963	2013	2013-07-01
81	Wilmington	Exit 41 diverge	5:05 PM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Cloudy	3502057	2013	2013-07-07
82	Wilmington	Exit 41 diverge	8:03 AM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Cloudy	3543034	2013	2013-07-22
83	Wilmington	Exit 41 diverge	10:19 AM	Off-peak	Snow/Ice	Daylight	Single vehicle crash	Non-fatal injury	Unknown	3685118	2013	2013-12-09
84	Wilmington	Exit 41 diverge	2:50 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3887785	2014	2014-07-24
85	Wilmington	Exit 41 diverge	3:22 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3795756	2014	2014-04-29
86	Wilmington	Exit 41 diverge	6:25 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3887782	2014	2014-07-17
87	Wilmington	Exit 41 diverge	2:35 PM	Off-peak	Wet	Daylight	Rear-end	Non-fatal injury	Rain	3907428	2014	2014-08-13
88	Wilmington	Exit 41 diverge	11:20 AM	Off-peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	3959500	2014	2014-10-06
89	Wilmington	Exit 41 diverge	3:03 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	3991837	2014	2014-12-30
90	Wilmington	Exit 41 diverge	5:14 PM	Peak	Dry	Dusk	Rear-end	Property damage only (none injured)	Clear	3997768	2015	2015-01-20
91	Wilmington	Exit 41 diverge	4:37 PM	Peak	Wet	Daylight	Single vehicle crash	Property damage only (none injured)	Cloudy	4012925	2015	2015-02-10
92	Wilmington	Exit 41 diverge	3:30 PM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	4008577	2015	2015-02-16
93	Wilmington	Exit 41 diverge	5:20 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4030660	2015	2015-03-26
94	Wilmington	Exit 41 diverge	7:50 PM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Non-fatal injury	Clear	4033921	2015	2015-04-15
95	Wilmington	Exit 41 diverge	7:50 AM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4058514	2015	2015-05-06
96	Wilmington	Exit 41 diverge	2:45 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4041332	2015	2015-05-12
97	Wilmington	Exit 41 diverge	5:55 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	4047250	2015	2015-05-29
98	Wilmington	Exit 41 diverge	9:37 AM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	4063712	2015	2015-06-26
99	Wilmington	Exit 41 diverge	3:45 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Unknown	4076568	2015	2015-08-11
100	Wilmington	Exit 41 diverge	2:30 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4079535	2015	2015-08-17
101	Wilmington	Exit 41 diverge	5:45 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4082424	2015	2015-08-25
102	Wilmington	Exit 41 diverge	2:25 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4086715	2015	2015-09-18
	Wilmington	Exit 41 diverge	6:25 PM	Peak	Dry	Dusk	Sideswipe, same direction	Property damage only (none injured)	Clear	4095280	2015	2015-09-30
	Wilmington	Exit 41 diverge	1:02 AM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Non-fatal injury	Clear	4118675	2015	2015-11-19
	Wilmington	Exit 41 diverge	9:00 AM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Cloudy	4118695	2015	2015-12-06
	Wilmington	Exit 41 diverge	6:12 AM	Peak	Dry	Dark - roadway not lighted	Rear-end	Non-fatal injury	Clear	4124993	2015	2015-12-17
	Wilmington	Exit 41 diverge	3:10 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4146377	2016	2016-02-04
	Wilmington	Exit 41 diverge	2:30 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4155360	2016	2016-02-26
	Wilmington	Exit 41 diverge	3:00 AM	Off-peak	Dry	Dark - roadway not lighted	Rear-end	Non-fatal injury	Clear	4170933	2016	2016-03-12

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1	Study Location	Area of Crash	Crash Time	Is Peak?	Road Surface Conditions	Ambient Light Conditions	Manner of Collision	Crash Severity	Weather Conditions	Crash Number	Crash Year	Crash Date
110	Wilmington	Exit 41 diverge	6:28 PM	Peak	Wet	Dark - roadway not lighted	Rear-end	Property damage only (none injured)	Rain	4277199	2016	2016-10-21
111	Wilmington	Exit 41 diverge	3:14 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4225013	2016	2016-07-25
112	Wilmington	Exit 41 diverge	3:45 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Unknown	4225876	2016	2016-07-29
113	Wilmington	Exit 41 diverge	11:06 PM	Off-peak	Dry	Dark - roadway not lighted	Rear-end	Property damage only (none injured)	Clear	4231281	2016	2016-08-05
114	Wilmington	Exit 41 diverge	2:30 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4232428	2016	2016-08-12
115	Wilmington	Exit 41 diverge	1:23 AM	Off-peak	Wet	Dark - roadway not lighted	Single vehicle crash	Not Reported	Rain	4246651	2016	2016-08-22
116	Wilmington	Exit 41 diverge	3:25 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4250351	2016	2016-09-15
117	Wilmington	Exit 41 diverge	1:10 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4251667	2016	2016-09-17
118	Wilmington	Exit 41 diverge	3:00 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4259039	2016	2016-09-20
119	Wilmington	Exit 41 diverge	2:49 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4285466	2016	2016-10-12
120	Wilmington	Exit 41 diverge	12:09 AM	Off-peak	Dry	Dark - roadway not lighted	Sideswipe, same direction	Property damage only (none injured)	Clear	4264687	2016	2016-10-15
121	Wilmington	Exit 41 diverge	8:16 AM	Peak	Dry	Daylight	Angle	Non-fatal injury	Clear	4280713	2016	2016-11-02
122	Wilmington	Exit 41 diverge	8:20 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4286590	2016	2016-11-02
123	Wilmington	Exit 41 diverge	9:00 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4286635	2016	2016-11-08
124	Wilmington	Exit 41 diverge	2:50 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4282098	2016	2016-11-10
125	Wilmington	Exit 41 diverge	8:45 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4311477	2016	2016-12-06
126	Wilmington	Exit 41 diverge	3:45 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3959836	2014	2014-09-17
127	Wilmington	Exit 41 diverge	8:58 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3340523	2013	2013-01-11
128	Wilmington	Exit 41 diverge	10:33 AM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3783775	2014	2014-04-01
129	Wilmington	Exit 41 diverge	7:09 AM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	4094461	2015	2015-09-21
130	Wilmington	Exit 41 diverge	4:00 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Unknown	4058716	2015	2015-06-16
131	Wilmington	I-93 Segment 3 (between Exit 40 and Exit 41)	5:40 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Cloudy	4070516	2015	2015-08-04
132	Wilmington	I-93 Segment 3 (between Exit 40 and Exit 41)	8:20 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Unknown	3664712	2013	2013-11-21
133	Wilmington	I-93 Segment 3 (between Exit 40 and Exit 41)	11:02 PM	Off-peak	Wet	Dark - lighted roadway	Sideswipe, same direction	Non-fatal injury	Rain	3427876	2013	2013-05-24
134	Wilmington	I-93 Segment 3 (between Exit 40 and Exit 41)	3:25 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Cloudy	3795758	2014	2014-04-30
135	Wilmington	I-93 Segment 3 (between Exit 40 and Exit 41)	5:01 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3870855	2014	2014-06-20
136	Wilmington	Exit 40 merge	2:50 AM	Off-peak	Dry	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Clear	4311467	2016	2016-12-24
137	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	2:50 PM	Off-peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	4132767	2016	2016-01-06
138	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	7:59 AM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	4248720	2016	2016-09-11
139	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	5:28 AM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Property damage only (none injured)	Clear	3298840	2012	2012-12-09
140	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	5:45 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3588308	2013	2013-09-13
141	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	2:50 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Unknown	3372288	2013	2013-03-20
142	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	2:45 PM	Off-peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	3481928	2013	2013-06-23
143	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	4:19 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3799235	2014	2014-01-20
144	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	3:39 PM	Peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Clear	3730081	2014	2014-01-27
	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	2:53 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4141204	2016	2016-01-12

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1	Study Location	Area of Crash	Crash Time	Is Peak?	Road Surface Conditions	Ambient Light Conditions	Manner of Collision	Crash Severity	Weather Conditions	Crash Number	Crash Year	Crash Date
146	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	6:03 PM	Peak	Dry	Daylight	Angle	Non-fatal injury	Cloudy	4228564	2016	2016-07-28
147	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	5:20 AM	Off-peak	Dry	Dark - roadway not lighted	Rear-end	Property damage only (none injured)	Unknown	3252543	2012	2012-09-16
148	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	12:08 PM	Off-peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	3602528	2013	2013-09-19
149	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	7:54 PM	Off-peak	Dry	Dark - lighted roadway	Angle	Non-fatal injury	Cloudy	3941716	2014	2014-08-22
150	Wilmington	I-93 Segment 2 (before Exit 40 on-ramp)	3:34 PM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	3720524	2014	2014-01-20
151	Wilmington	Exit 40 on-ramp	11:37 AM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3168166	2012	2012-02-23
152	Wilmington	Exit 40 off-ramp	11:40 PM	Off-peak	Snow/Ice	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Unknown	3367261	2013	2013-03-07
153	Wilmington	Exit 40 off-ramp	11:29 PM	Off-peak	Dry	Dark - unknown roadway	Single vehicle crash	Property damage only (none injured)	Cloudy	3850241	2014	2014-06-12
154	Wilmington	Exit 40 off-ramp	9:40 AM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Unknown	3215949	2012	2012-07-04
155	Wilmington	Exit 40 diverge	6:11 AM	Peak	Dry	Dark - roadway not lighted	Single vehicle crash	Non-fatal injury	Cloudy	2894051	2012	2012-01-22
156	Wilmington	Exit 40 diverge	2:17 PM	Off-peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Unknown	3604395	2013	2013-10-04
157	Wilmington	Exit 40 diverge	12:18 PM	Off-peak	Snow/Ice	Daylight	Single vehicle crash	Property damage only (none injured)	Snow	3362802	2013	2013-02-17
158	Wilmington	Exit 40 diverge	3:55 PM	Peak	Wet	Dusk	Rear-end	Property damage only (none injured)	Rain	3713445	2014	2014-01-06
159	Wilmington	Exit 40 diverge	11:11 PM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Unknown	3794740	2014	2014-04-24
160	Wilmington	Exit 40 diverge	9:02 AM	Peak	Dry	Daylight	Single vehicle crash	Non-fatal injury	Unknown	3867488	2014	2014-06-27
161	Wilmington	Exit 40 diverge	2:35 PM	Off-peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	3869351	2014	2014-06-28
162	Wilmington	Exit 40 diverge	9:35 PM	Off-peak	Wet	Dark - roadway not lighted	Single vehicle crash	Property damage only (none injured)	Rain	3987436	2014	2014-12-23
163	Wilmington	Exit 40 diverge	3:00 PM	Peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Rain	3999799	2015	2015-01-12
164	Wilmington	Exit 40 diverge	4:40 AM	Off-peak	Dry	Dark - roadway not lighted	Single vehicle crash	Property damage only (none injured)	Clear	4024118	2015	2015-03-20
165	Wilmington	Exit 40 diverge	9:08 AM	Peak	Dry	Daylight	Angle	Non-fatal injury	Unknown	4028915	2015	2015-03-20
166	Wilmington	Exit 40 diverge	9:35 PM	Off-peak	Dry	Dark - roadway not lighted	Angle	Non-fatal injury	Unknown	4030736	2015	2015-03-21
167	Wilmington	Exit 40 diverge	3:55 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4027365	2015	2015-03-31
168	Wilmington	Exit 40 diverge	4:45 PM	Peak	Dry	Dark - roadway not lighted	Single vehicle crash	Property damage only (none injured)	Clear	4119723	2015	2015-12-09
169	Wilmington	Exit 40 diverge	6:55 PM	Peak	Dry	Dark - unknown roadway	Single vehicle crash	Non-fatal injury	Cloudy	4131981	2015	2015-12-26
170	Wilmington	Exit 40 diverge	3:00 PM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	4165145	2016	2016-03-16
171	Wilmington	Exit 40 diverge	4:30 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4219952	2016	2016-06-28
172	Wilmington	Exit 40 diverge	7:50 PM	Off-peak	Dry	Dark - roadway not lighted	Rear-end	Property damage only (none injured)	Clear	4191975	2016	2016-05-06
173	Wilmington	Exit 40 diverge	8:40 PM	Off-peak	Dry	Dark - roadway not lighted	Sideswipe, same direction	Non-fatal injury	Clear	4193354	2016	2016-05-11
174	Wilmington	Exit 40 diverge	5:55 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4195143	2016	2016-05-12
175	Wilmington	Exit 40 diverge	7:00 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4218755	2016	2016-06-10
176	Wilmington	Exit 40 diverge	5:35 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4203970	2016	2016-06-13
177	Wilmington	Exit 40 diverge	3:00 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4225021	2016	2016-07-28
178	Wilmington	Exit 40 diverge	5:55 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4227763	2016	2016-08-02
179	Wilmington	Exit 40 diverge	10:20 AM	Off-peak	Dry	Daylight	Angle	Non-fatal injury	Clear	4291906	2016	2016-11-01
180	Wilmington	Exit 40 diverge	8:36 PM	Off-peak	Dry	Dark - roadway not lighted	Rear-end	Property damage only (none injured)	Clear	4285953	2016	2016-11-13
	Wilmington	Exit 40 diverge	9:26 PM	Off-peak	Snow/Ice	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Snow	4324755	2016	2016-12-29

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182	Wilmington	Exit 40 diverge	4:39 PM	Peak	Dry	Dusk	Sideswipe, same direction	Non-fatal injury	Clear	4132039	2016	2016-01-07
183	Wilmington	Exit 40 diverge	7:46 PM	Off-peak	Dry	Daylight	Sideswipe, same direction	Non-fatal injury	Clear	4218777	2016	2016-06-25

Α	В	G	н	J	К	М	Ν	0	Q	Т	U
Study Location	Area of Crash	Crash Time	Is Peak?	Road Surface Conditions	Ambient Light Condition	s Manner of Collision	Crash Severity	Weather Conditions	Crash Numbe	r Crash Year	Crash Date
184 Quincy	I-93 Segment 1 (over traffic circle)	4:25 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Unknown	4149066	2016	2016-01-26
185 Quincy	I-93 Segment 1 (over traffic circle)	7:40 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3484089	2013	2013-06-14
186 Quincy	Exit 8 on-ramp	12:19 PM	Off-peak	Dry	Daylight	Angle	Non-fatal injury	Clear	4160084	2016	2016-03-02
187 Quincy	I-93 Segment 2 (ahead of Exit 8 on-ramp)	11:14 PM	Off-peak	Wet	Dark - lighted roadway	Angle	Property damage only (none injured)	Rain	3863457	2014	2014-06-13
188 Quincy	I-93 Segment 2 (ahead of Exit 8 on-ramp)	9:03 PM	Off-peak	Dry	Dark - lighted roadway	Sideswipe, same direction	Non-fatal injury	Clear	4203426	2016	2016-05-25
189 Quincy	Exit 8 merge	9:05 AM	Peak	Snow/Ice	Daylight	Single vehicle crash	Non-fatal injury	Cloudy	3730358	2014	2014-01-04
190 Quincy	Exit 8 merge	3:10 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4039055	2015	2015-04-15
191 Quincy	Exit 8 merge	8:07 AM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	4220945	2016	2016-05-31
192 Quincy	Exit 8 merge	6:23 PM	Peak	Wet	Dark - lighted roadway	Angle	Property damage only (none injured)	Rain	3384663	2013	2013-03-06
193 Quincy	I-93 Segment 3 (after Exit 8 on-ramp)	1:05 PM	Off-peak	Wet	Daylight	Single vehicle crash	Non-fatal injury	Rain	3491318	2013	2013-06-13
194 Quincy	I-93 Segment 3 (after Exit 8 on-ramp)	8:35 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4124999	2015	2015-12-16
195 Quincy	I-93 Segment 3 (after Exit 8 on-ramp)	5:52 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3893451	2014	2014-07-31
196 Quincy	I-93 Segment 4 (near HOV lane merge)	3:00 PM	Peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Clear	4237590	2016	2016-08-04
197 Quincy	I-93 Segment 4 (near HOV lane merge)	7:10 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4235196	2016	2016-08-23
198 Quincy	HOV lane merge	10:10 AM	Off-peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	3793362	2014	2014-04-16
199 Quincy	I-93 Segment 5 (ahead of Exit 7 diverge)	2:13 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Clear	4134751	2016	2016-01-09
200 Quincy	I-93 Segment 5 (ahead of Exit 7 diverge)	8:36 AM	Peak	Wet	Daylight	Rear-end	Non-fatal injury	Rain	3475968	2013	2013-06-11
201 Quincy	I-93 Segment 5 (ahead of Exit 7 diverge)	3:20 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	3384447	2012	2012-09-20
202 Quincy	I-93 Segment 5 (ahead of Exit 7 diverge)	5:33 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	3162992	2012	2012-07-08
203 Quincy	Exit 7 diverge	12:25 AM	Off-peak	Dry	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Clear	4155214	2016	2016-02-23
204 Quincy	Exit 7 diverge	7:19 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3863449	2014	2014-05-13
205 Quincy	Exit 7 diverge	4:17 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4062538	2015	2015-05-27
206 Quincy	Exit 7 diverge	4:10 PM	Peak	Dry	Daylight	Single vehicle crash	Not Reported	Clear	4048083	2015	2015-05-29
207 Quincy	Exit 7 diverge	3:00 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Non-fatal injury	Clear	4108224	2015	2015-10-18
208 Quincy	Exit 7 diverge	3:30 PM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	4250337	2016	2016-08-31
209 Quincy	Exit 7 diverge	2:40 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Non-fatal injury	Clear	3381453	2012	2012-02-01
210 Quincy	Exit 7 diverge	8:17 AM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3381575	2012	2012-04-11
211 Quincy	Exit 7 diverge	7:21 PM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Unknown	3647651	2013	2013-10-19
212 Quincy	Exit 7 diverge	8:38 AM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3208033	2012	2012-05-15
213 Quincy	Exit 7 diverge	7:50 AM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	3421265	2013	2013-04-26
214 Quincy	Exit 7 diverge	7:10 PM	Off-peak	Dry	Dark - lighted roadway	Angle	Non-fatal injury	Clear	3963794	2014	2014-09-26
215 Quincy	Exit 7 diverge	4:00 PM	Peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Clear	3985720	2014	2014-12-02
216 Quincy	Exit 7 diverge	3:25 AM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Non-fatal injury	Cloudy	3541450	2013	2013-07-27
217 Quincy	Exit 7 diverge	1:50 AM	Off-peak	Snow/Ice	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Snow	3384348	2012	2012-03-03
218 Quincy	Exit 7 diverge	1:44 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	2854032	2012	2012-01-02
219 Quincy	Exit 7 diverge	11:44 AM	Off-peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	2900658	2012	2012-01-08

	A	В	G	Н	J	К	М	Ν	0	Q	Т	U
1	Study Location	Area of Crash	Crash Time	Is Peak?	Road Surface Conditions	Ambient Light Conditions	Manner of Collision	Crash Severity	Weather Conditions	Crash Number	r Crash Year	Crash Date
220	Quincy	Exit 7 diverge	12:51 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	2890194	2012	2012-01-25
221	Quincy	Exit 7 diverge	11:00 PM	Off-peak	Dry	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Clear	3061894	2012	2012-03-16
222	Quincy	Exit 7 diverge	6:39 AM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	3389298	2012	2012-07-15
223	Quincy	Exit 7 diverge	1:40 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	2939680	2012	2012-02-20
224	Quincy	Exit 7 diverge	12:00 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Not Reported	Cloudy	3000832	2012	2012-02-25
225	Quincy	Exit 7 diverge	5:10 PM	Peak	Dry	Daylight	Rear-end	Not Reported	Cloudy	3102080	2012	2012-05-05
226	Quincy	Exit 7 diverge	9:19 AM	Peak	Wet	Daylight	Single vehicle crash	Not Reported	Rain	3113847	2012	2012-05-16
227	Quincy	Exit 7 diverge	5:05 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Cloudy	3154606	2012	2012-06-23
228	Quincy	Exit 7 diverge	9:40 PM	Off-peak	Dry	Dark - lighted roadway	Angle	Property damage only (none injured)	Unknown	3207191	2012	2012-07-20
229	Quincy	Exit 7 diverge	2:07 PM	Off-peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	3289050	2012	2012-10-26
230	Quincy	Exit 7 diverge	12:00 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Clear	3289110	2012	2012-11-12
231	Quincy	Exit 7 diverge	11:50 PM	Off-peak	Dry	Dark - roadway not lighted	Angle	Property damage only (none injured)	Clear	3292059	2012	2012-11-14
232	Quincy	Exit 7 diverge	3:30 PM	Peak	Dry	Daylight	Single vehicle crash	Non-fatal injury	Clear	3330247	2012	2012-12-31
233	Quincy	Exit 7 diverge	3:30 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3607513	2013	2013-09-27
234	Quincy	Exit 7 diverge	10:30 AM	Off-peak	Dry	Daylight	Sideswipe, same direction	Not Reported	Clear	3655412	2013	2013-10-03
235	Quincy	Exit 7 diverge	8:50 AM	Peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	3362870	2013	2013-02-22
236	Quincy	Exit 7 diverge	11:30 PM	Off-peak	Snow/Ice	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Snow	3390860	2013	2013-03-27
237	Quincy	Exit 7 diverge	9:52 AM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	3359757	2013	2013-02-07
238	Quincy	Exit 7 diverge	4:30 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3376175	2013	2013-03-10
239	Quincy	Exit 7 diverge	1:41 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3391936	2013	2013-03-21
240	Quincy	Exit 7 diverge	2:16 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3377191	2013	2013-03-24
241	Quincy	Exit 7 diverge	12:00 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Non-fatal injury	Clear	3381638	2013	2013-04-05
242	Quincy	Exit 7 diverge	8:45 AM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3430374	2013	2013-05-03
243	Quincy	Exit 7 diverge	9:00 PM	Off-peak	Dry	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Clear	3430681	2013	2013-05-04
	Quincy	Exit 7 diverge	3:28 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3453240	2013	2013-05-30
	Quincy	Exit 7 diverge	4:39 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Unknown	3510837	2013	2013-07-03
	Quincy	Exit 7 diverge	6:25 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3556656	2013	2013-08-05
247	Quincy	Exit 7 diverge	3:20 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Unknown	3580257	2013	2013-08-20
248	Quincy	Exit 7 diverge	11:30 AM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3611135	2013	2013-09-22
	Quincy	Exit 7 diverge	11:15 AM	Off-peak	Dry	Daylight	Angle	Property damage only (none injured)	Clear	3611145	2013	2013-10-03
	Quincy	Exit 7 diverge	4:42 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Clear	3623834	2013	2013-10-26
	Quincy	Exit 7 diverge	11:20 AM	Off-peak	Wet	Daylight	Not reported	Property damage only (none injured)	Clear	3710829	2013	2013-12-19
	Quincy	Exit 7 diverge	11:32 PM	Off-peak	Wet	Dark - lighted roadway	Angle	Property damage only (none injured)	Rain	3726184	2014	2014-01-10
	Quincy	Exit 7 diverge	10:50 PM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Clear	3772811	2014	2014-03-07
	Quincy	Exit 7 diverge	8:34 AM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3772813	2014	2014-03-08
		Exit 7 diverge	4:05 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3974039	2014	2014-11-14

	А	В	G	Н	J	К	М	N	0	Q	Т	U
1	Study Location	Area of Crash	Crash Time	Is Peak?	Road Surface Conditions	Ambient Light Conditions	Manner of Collision	Crash Severity	Weather Conditions	Crash Number	Crash Year	Crash Date
256	Quincy	Exit 7 diverge	2:10 AM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Clear	3792026	2014	2014-04-21
257	Quincy	Exit 7 diverge	6:40 PM	Peak	Wet	Daylight	Single vehicle crash	Non-fatal injury	Rain	3902045	2014	2014-07-14
258	Quincy	Exit 7 diverge	10:58 AM	Off-peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	3908557	2014	2014-08-04
259	Quincy	Exit 7 diverge	5:12 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3909153	2014	2014-08-07
260	Quincy	Exit 7 diverge	6:45 AM	Peak	Dry	Daylight	Single vehicle crash	Property damage only (none injured)	Clear	3909154	2014	2014-08-08
261	Quincy	Exit 7 diverge	11:37 PM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Non-fatal injury	Unknown	3935855	2014	2014-08-17
262	Quincy	Exit 7 diverge	2:08 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3922844	2014	2014-08-21
263	Quincy	Exit 7 diverge	6:20 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3928331	2014	2014-09-05
264	Quincy	Exit 7 diverge	10:48 PM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Clear	3962816	2014	2014-10-09
265	Quincy	Exit 7 diverge	12:24 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3977440	2014	2014-11-22
266	Quincy	Exit 7 diverge	12:00 AM	Off-peak	Wet	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Rain	3987225	2014	2014-12-07
267	Quincy	Exit 7 diverge	9:45 PM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Clear	4011973	2014	2014-12-14
268	Quincy	Exit 7 diverge	10:35 AM	Off-peak	Dry	Daylight	Sideswipe, same direction	Non-fatal injury	Cloudy	4024370	2015	2015-03-20
269	Quincy	Exit 7 diverge	9:05 PM	Off-peak	Snow/Ice	Dark - lighted roadway	Rear-end	Non-fatal injury	Snow	4012964	2015	2015-02-08
270	Quincy	Exit 7 diverge	4:50 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4037235	2015	2015-04-06
271	Quincy	Exit 7 diverge	1:35 AM	Off-peak	Dry	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Clear	4031392	2015	2015-04-11
272	Quincy	Exit 7 diverge	4:35 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4037452	2015	2015-04-13
273	Quincy	Exit 7 diverge	11:50 AM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4046705	2015	2015-05-23
274	Quincy	Exit 7 diverge	12:50 AM	Off-peak	Dry	Dark - lighted roadway	Angle	Non-fatal injury	Clear	4085525	2015	2015-08-26
275	Quincy	Exit 7 diverge	1:35 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4099952	2015	2015-10-12
276	Quincy	Exit 7 diverge	6:20 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4065328	2015	2015-07-07
277	Quincy	Exit 7 diverge	3:06 PM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4065876	2015	2015-07-16
278	Quincy	Exit 7 diverge	5:45 PM	Peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Clear	4084975	2015	2015-08-29
279	Quincy	Exit 7 diverge	11:29 PM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Cloudy	4099176	2015	2015-09-21
280	Quincy	Exit 7 diverge	3:10 AM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Non-fatal injury	Clear	4089288	2015	2015-09-25
281	Quincy	Exit 7 diverge	1:32 AM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Clear	4104890	2015	2015-10-09
282	Quincy	Exit 7 diverge	7:15 PM	Off-peak	Wet	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Clear	4121124	2015	2015-11-28
283	Quincy	Exit 7 diverge	5:20 PM	Peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Clear	4135117	2015	2015-12-28
284	Quincy	Exit 7 diverge	6:43 PM	Peak	Dry	Dark - lighted roadway	Rear-end	Property damage only (none injured)	Clear	4134715	2015	2015-12-30
285	Quincy	Exit 7 diverge	1:55 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4141149	2016	2016-01-16
	Quincy	Exit 7 diverge	4:11 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4143218	2016	2016-01-02
	Quincy	Exit 7 diverge	2:15 PM	Off-peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Snow	4152580	2016	2016-02-08
	Quincy	Exit 7 diverge	6:47 AM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	4164398	2016	2016-03-12
	Quincy	Exit 7 diverge	11:50 AM	Off-peak	Wet	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4169147	2016	2016-03-15
	Quincy	Exit 7 diverge	7:30 PM	Off-peak	Dry	Dark - lighted roadway	Rear-end	Non-fatal injury	Cloudy	4175021	2016	2016-04-11
		Exit 7 diverge	7:38 PM	Off-peak	Wet	Dark - lighted roadway	Sideswipe, same direction	Non-fatal injury	Rain	4186159	2016	2016-04-12

	А	В	G	Н	J	К	М	Ν	0	Q	Т	U
1	Study Location	Area of Crash	Crash Time	Is Peak?	Road Surface Conditions	Ambient Light Conditions	Manner of Collision	Crash Severity	Weather Conditions	Crash Number	Crash Year	Crash Date
292	Quincy	Exit 7 diverge	4:00 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4177270	2016	2016-04-14
293	Quincy	Exit 7 diverge	8:20 AM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4220955	2016	2016-06-08
294	Quincy	Exit 7 diverge	1:18 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4220974	2016	2016-06-19
295	Quincy	Exit 7 diverge	6:19 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	4226497	2016	2016-07-06
296	Quincy	Exit 7 diverge	8:02 PM	Off-peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Clear	4224577	2016	2016-07-07
297	Quincy	Exit 7 diverge	11:15 PM	Off-peak	Dry	Dark - lighted roadway	Sideswipe, same direction	Property damage only (none injured)	Unknown	4246657	2016	2016-08-30
298	Quincy	Exit 7 diverge	12:00 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4245884	2016	2016-09-04
299	Quincy	Exit 7 diverge	5:39 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Cloudy	4245601	2016	2016-09-06
300	Quincy	Exit 7 diverge	5:05 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4250342	2016	2016-09-08
301	Quincy	Exit 7 diverge	10:45 AM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4276584	2016	2016-11-01
302	Quincy	Exit 7 diverge	12:58 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Cloudy	4277246	2016	2016-11-03
303	Quincy	Exit 7 diverge	2:10 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Cloudy	4291883	2016	2016-11-19
304	Quincy	Exit 7 diverge	1:45 PM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4313065	2016	2016-12-03
305	Quincy	Exit 7 diverge	11:56 AM	Off-peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	4313019	2016	2016-12-14
306	Quincy	Exit 7 diverge	10:47 PM	Off-peak	Dry	Dark - lighted roadway	Single vehicle crash	Property damage only (none injured)	Unknown	4313119	2016	2016-12-24
307	Quincy	Exit 7 diverge	9:00 PM	Off-peak	Wet	Dark - lighted roadway	Single vehicle crash	Non-fatal injury	Unknown	3984905	2014	2014-12-10
308	Quincy	Exit 7 diverge	9:56 PM	Off-peak	Wet	Dark - lighted roadway	Rear-end	Non-fatal injury	Rain	4062116	2015	2015-06-27
309	Quincy	Exit 7 diverge	1:40 PM	Off-peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	3545935	2013	2013-07-17
310	Quincy	Exit 7 diverge	4:54 AM	Off-peak	Wet	Dark - lighted roadway	Single vehicle crash	Non-fatal injury	Rain	4262971	2016	2016-10-01
311	Quincy	I-93 Segment 6 (shortly after Exit 7 split)	3:33 PM	Peak	Unknown	Not reported	Not reported	Property damage only (none injured)	Unknown	3826940	2014	2014-05-27
312	Quincy	I-93 Segment 6 (shortly after Exit 7 split)	8:00 AM	Peak	Dry	Daylight	Rear-end	Non-fatal injury	Clear	4160026	2016	2016-02-17
313	Quincy	I-93 Segment 6 (shortly after Exit 7 split)	5:15 PM	Peak	Dry	Daylight	Sideswipe, same direction	Property damage only (none injured)	Unknown	3376197	2013	2013-03-25
314	Quincy	I-93 Segment 6 (shortly after Exit 7 split)	7:00 PM	Peak	Dry	Dark - lighted roadway	Rear-end	Non-fatal injury	Clear	3354879	2013	2013-02-14
315	Quincy	I-93 Segment 6 (shortly after Exit 7 split)	4:45 PM	Peak	Dry	Daylight	Rear-end	Property damage only (none injured)	Clear	3391082	2013	2013-02-06

# APPENDIX D

1. HCS printouts

#### Intersection Capacity Analysis 1: Route 125 & I-93 NB Off-Ramp

07/17/2019

Lane Configurations         FBI		۶	+	*	4	ł	×	<	1	1	×	Ŧ	~
Lane Configurations         r         r         r         r         r         r         r           traftic Volume (vph)         0         0         0         184         0         893         0         276         41         0         1373         639           titure Volume (vph)         1900         110	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)         0         0         0         184         0         893         0         276         41         0         1373         639           Iduar Volume (vph)         1900         1000 <td< td=""><td>Lane Configurations</td><td></td><td></td><td></td><td>٦ ۲</td><td></td><td>11</td><td></td><td><b>4</b>1a</td><td></td><td></td><td>**</td><td>1</td></td<>	Lane Configurations				٦ ۲		11		<b>4</b> 1a			**	1
Future Volume (vph)         0         0         0         184         0         893         0         276         41         0         1373         639           ideal How (vph)         1900		0	0	0		0		0		41	0		
Ideal flow (spip)         1900 <td></td> <td></td> <td></td> <td></td> <td></td> <td>0</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						0							
Storage Lanes         0         0         0         1         1         0         0         0         1           Taper Lengh (ft)         25         26 </td <td></td>													
Storage Lange         0         0         1         1         0         0         0         1           Taper Length (ft)         25         25         25         25         25         25           Link Speed (mph)         30         30         30         30         30         30         30         11         1         0         0         0         1         11         0         0         0         1         11         1         0         0         0         1         11         0         0         0         1         1         1         0         0         0         1         1         1         0         0         0         1         1         0         0         0         1         1         0         0         0         1         1         1         0         0         0         1         1         1         0         0         0         1         1         1         0         0         0         1         1         1         1         1         1         1         1         1         1         1         1         1         1         1         1         1													
Taper Length (ft)         25         25         25         25           Right Turn on Red         Yes         Yes         Yes         Yes           Link Speed (mph)         30         30         30         30           Link Speed (mph)         405         717         1086         800           Link Speed (mph)         092         0.92         0.92         0.93         0.92         0.78         0.85         0.92         0.84         0.76           Peak Hour Factor         0.92         0.92         0.92         0.92         0.93         0.92         0.78         0.85         0.92         0.84         0.76           Shared Lane Traffic (%)         2%         1%         2%         2%         2%         2%         2%         2%         2%         2%         2%													
Right Turn on Red         Yes         Yes         Yes         Yes         Yes           Link Speed (mph)         30         30         30         30         30           Link Distance (th)         405         717         1086         800         177           Peak Hour Factor         0.92         0.92         0.92         0.92         0.92         0.93         0.92         0.48         0.76         448         2%         <				-									
Link Speed (mph)         30         30         30         30           Link Distance (ft)         405         717         1086         800           Link Distance (ft)         405         717         1086         800           Travel Time (s)         9.2         0.82         0.92         0.93         0.92         0.78         0.85         0.92         0.84         0.76           Peak Hour Factor         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.92         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93 <td></td> <td>20</td> <td></td> <td>Yes</td> <td>20</td> <td></td> <td>Yes</td> <td>20</td> <td></td> <td>Yes</td> <td>20</td> <td></td> <td>Yes</td>		20		Yes	20		Yes	20		Yes	20		Yes
Link Distance (n)         405         717         1086         800           Iravel Time (s)         9.2         0.82         0.92         0.93         0.92         0.78         0.85         0.92         0.93         0.92         0.78         0.85         0.92         0.93         0.92         0.78         0.85         0.92         0.93         0.92         0.93         0.92         0.78         0.85         0.92         0.93         0.93         0.93         1.2         0.93         1.2         Destin many sonin insin sing sing sing sing sing sing			30	100		30	100		30	100		30	100
Trave Time (s)         9.2         16.3         24.7         18.2           Peak Hour Factor         0.92         0.92         0.92         0.93         0.93         0.92         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.93         0.94         0.93         0.9													
Peak Hour Factor         0.92         0.92         0.92         0.92         0.92         0.78         0.85         0.92         0.84         0.76           Heavy Vehicles (%)         2%         2%         2%         0%         2%         4%         2%													
Heavy Vehicles (%)       2%       2%       2%       2%       2%       2%       2%       2%       1%       2%         Shared Lane Trafic (%)       0       0       0       224       0       960       0       402       0       0       1635       841         Turn Type       Prot       custom       NA       NA       Permited Phases       12       12         Permited Phases       3       1       2 <t< td=""><td></td><td>0.92</td><td></td><td>0.92</td><td>0.82</td><td></td><td>0.93</td><td>0.92</td><td></td><td>0.85</td><td>0.92</td><td></td><td>0.76</td></t<>		0.92		0.92	0.82		0.93	0.92		0.85	0.92		0.76
Shared Lane Traffic (%)       0       0       224       0       960       0       402       0       0       1635       841         Irun Type       Prote       custom       NA       NA       Perm         Protected Phases       3       1.3       2       1.2         Permitted Phases       3       1       2       2         Switch Phase       3       1       2       2         Switch Phase       3.0       2.3.0       14.5       12         Detector Phase       3.0       23.0       23.0       15         Total Split (s)       3.0       23.0       24.0       34.0       34.0         All-Red Time (s)       3.5       4.0       40.0%       28.8%       12.0       14.5         Lost Time Adjust (s)       -1.0       -2.5       14.0													
Lane Group Flow (vph)         0         0         0         224         0         960         0         402         0         0         1635         841           Jun Type         Prote Ced Phases         3         13         2         12           Permitted Phases         3         1         2         2         2           Detector Phase         3         1         2         2         2           Switch Phase         3         1         2         2         2           Minimum Split (s)         6.0         8.0		270	270	270	070	270	770	270	70	270	270	170	270
Turn Type         Prot         custom         NA         NA         Permitted           Protected Phases         3         1.3         2         1.2           Detector Phase         3         1         2         2           Switch Phase         3         1         2         2           Minimum Initial (s)         6.0         8.0		Ο	0	0	224	0	060	0	102	Ο	0	1635	8/1
Protected Phases         3         13         2         12           Permitted Phases         3         1         2         2           Detector Phase         3         1         2         2           Minimum Initial (s)         6.0         8.0         12           Minimum Split (s)         12.0         14.5         12           Total Split (s)         32.0         23.0         12           Total Split (s)         3.5         4.0         12           All-Red Time (s)         1.5         2.5         1           Lost Time Adjust (s)         -1.0         -2.5         1           Total Split (%)         40.0         4.0         4.0           Lead-Lag Optimize?         Yes         1         1           Recall Mode         Min         C-Min         1           Act Effed Green (s)         15.4         34.5         37.5         56.6         56.6           Actuated g/C Ratio         0.19         0.43         0.47         0.71         0.71         0.71           Vic Ratio         0.65         0.72         0.25         0.65         0.62         20.11         6.1         1.1           Outroue Delay		0	0	0		0		0		0	0		
Permitted Phases         3         1         2         2           Switch Phase         3         1         2         2           Switch Phase         0         8.0         0         8.0           Minimum Initial (S)         6.0         8.0         0         14.5         0           Total Split (S)         12.0         14.5         0         14.5         0         14.5         0         14.5         0         14.5         0         15.5         2.5         0         15.5         15.5         15.5         15.5         16.1         15.5         16.1         16.1         16.1         16.1         16.1         16.1         16.1         16.1         16.1         16.1         16.1         16.1         16.1         17.1         16.1         17.1													r enn
Detector Phase         3         1         2         2           Switch Phase					3		15		Z			ΙZ	1 0
Switch Phase         Minimum Initial (s)         6.0         8.0           Minimum Split (s)         12.0         14.5         Total Split (s)         32.0         23.0           Total Split (s)         32.0         28.8%           All-Red Time (s)         1.5         2.5             Lost Time Adjust (s)         -1.0         -2.5					2		1		2			C	ΙZ
Minimum Initial (s)         6.0         8.0           Minimum Split (s)         12.0         14.5           Total Split (s)         32.0         23.0           Total Split (%)         40.0%         28.8%           Yellow Time (s)         3.5         4.0           All:Red Time (s)         1.5         2.5           Lost Time Adjust (s)         -1.0         -2.5           Total Split Time (s)         4.0         4.0           Lead/Lag         Lag         Lead/Lag           Lead/Lag (Lag (Lag (Lag (Lag (Lag (Lag (Lag (					3		I		Z			Z	
Minimum Split (s)         12.0         14.5           Total Split (s)         32.0         23.0           Total Split (s)         32.0         28.8%           Vellow Time (s)         3.5         4.0           All-Red Time (s)         1.5         2.5           Lost Time Adjust (s)         -1.0         -2.5           Total Lost Time (s)         4.0         4.0           Lead/Lag         Lag         Lead/Lag           Lead/Lag Optimize?         Yes         Yes           Recall Mode         Min         C-Min           Act Effect Green (s)         15.4         34.5         37.5         56.6         56.6           Actuated g/C Ratio         0.19         0.43         0.47         0.71         0.71           V/c Ratio         0.65         0.72         0.25         0.65         0.62           Control Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0					6.0				0.0				
Total Split (s)         32.0         23.0           Total Split (%)         40.0%         28.8%           Yellow Time (s)         3.5         4.0           All-Red Time (s)         1.5         2.5           Lost Time Adjust (s)         -1.0         -2.5           Total Lost Time (s)         4.0         4.0           Lead/Lag         Lag           Lead/Lag (c)         Yels           Recall Mode         Min           Act Effct Green (s)         15.4         34.5           Act affct Green (s)         0.19         0.43         0.47           Actated g/C Ratio         0.65         0.72         0.25           Contol Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         37.9         16.1         12.1         6.1         1.1           Loss Contol Delay         37.9         16.1         12.1         6.1         1.1           Loss Contol Delay         20.2         12.1         4.4         4           Approach LOS         C         B         A         A           Ou													
Total Split (%)         40.0%         28.8%           Yellow Time (s)         3.5         4.0           All-Red Time (s)         1.5         2.5           Lost Time Adjust (s)         -1.0         -2.5           Total Lost Time (s)         4.0         4.0           Lead/Lag         Lag													
Yellow Time (s)         3.5         4.0           All-Red Time (s)         1.5         2.5           Lost Time Adjust (s)         -1.0         -2.5           Total Lost Time (s)         4.0         4.0           Lead/Lag         Lag         Lag           Lead/Lag Optimize?         Yes         Yes           Recall Mode         Min         C-Min           Act Effct Green (s)         15.4         34.5         37.5         56.6         56.6           Actuated g/C Ratio         0.19         0.43         0.47         0.71         0.71           v/c Ratio         0.65         0.72         0.25         0.65         0.62           Control Delay         37.9         16.1         12.1         6.1         1.1           LOS         D         B         B         A           Approach Delay         20.2         12.1         4.4           Approach LOS         C         B         A           Queue Length 50th (ft)         104         156         67         104         0           Queue Length 50th (ft)         142         197         93         m240         m2           Internal Link Dist (ft)         325													_
All-Red Time (s)       1.5       2.5         Lost Time Adjust (s)       -1.0       -2.5         Total Lost Time (s)       4.0       4.0         Lead/Lag       Lag         Lead/Lag Optimize?       Yes         Recall Mode       Min       C-Min         Act Effct Green (s)       15.4       34.5       37.5       56.6       56.6         Actuated g/C Ratio       0.19       0.43       0.47       0.71       0.71         V/c Ratio       0.65       0.72       0.25       0.65       0.62         Control Delay       37.9       16.1       12.1       6.1       1.1         Los       D       B       B       A       A         Approach Delay       0.0       0.0       0.0       0.0       0.0         Oucue Length Solth (ft)       104       156       67       104       0         Queue Length Solth (ft)       142       197       93       m240       m2         Internal Link Dist (ft)       325       637       1006       720       100       104       00         Queue Length Solth (ft)       151       1513       1612       2528       1365       Starvatino Cap Reductn </td <td></td>													
Lost Time Adjust (s)       -1.0       -2.5         Total Lost Time (s)       4.0       4.0         Lead/Lag       Lag         Lead-Lag Optimize?       Yes         Recall Mode       Min       C-Min         Act Effct Green (s)       15.4       34.5       37.5       56.6       56.6         Actuated g/C Ratio       0.19       0.43       0.47       0.71       0.71         v/c Ratio       0.65       0.72       0.25       0.65       0.62         Control Delay       37.9       16.1       12.1       6.1       1.1         Queue Delay       0.0       0.0       0.0       0.0       0.0         Total Delay       37.9       16.1       12.1       6.1       1.1         Los       D       B       B       A       A         Approach Delay       20.2       12.1       4.4       Approach LOS       C       B       A         Queue Length 50th (ft)       104       156       67       104       0         Queue Length 95th (ft)       142       197       93       m240       m20         Internal Link Dist (ft)       325       637       1006       720 <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>_</td></td<>													_
Total Lost Time (s)         4.0         4.0           Lead/Lag         Lag           Lead-Lag Optimize?         Yes           Recall Mode         Min         C-Min           Act Effct Green (s)         15.4         34.5         37.5         56.6         56.6           Actuated g/C Ratio         0.19         0.43         0.47         0.71         0.71           v/c Ratio         0.65         0.72         0.25         0.65         0.62           Control Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         20.2         12.1         4.4         4         Approach LOS         C         B         A           Queue Length 50th (ft)         104         156         67         104         0           Queue Length 95th (tt)         325         637         1006         720         Turn Bay Length (ft) <td< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></td<>													
Lead/Lag         Lag           Lead-Lag Optimize?         Yes           Recall Mode         Min         C-Min           Act Effet Green (s)         15.4         34.5         37.5         56.6         56.6           Actuated g/C Ratio         0.19         0.43         0.47         0.71         0.71           v/c Ratio         0.65         0.72         0.25         0.65         0.62           Control Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         37.9         16.1         12.1         6.1         1.1           LOS         D         B         B         A         A           Approach Delay         20.2         12.1         4.4         A           Approach LOS         C         B         A         A           Queue Length 95th (ft)         104         156         67         104         0           Queue Length 95th (ft)         325         637         1006         720         100         10           Turn Bay Length (ft)         325         637         1006													
Lead-Lag Optimize?         Yes           Recall Mode         Min         C-Min           Act Effct Green (s)         15.4         34.5         37.5         56.6         56.6           Actuated g/C Ratio         0.19         0.43         0.47         0.71         0.71           v/c Ratio         0.65         0.72         0.25         0.65         0.62           Control Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         37.9         16.1         12.1         6.1         1.1           LOS         D         B         B         A         A           Approach Delay         20.2         12.1         4.4         4           Approach LOS         C         B         A         A           Queue Length 50th (ft)         104         156         67         104         0           Queue Length 95th (ft)         325         637         1006         720         1014         102           Internal Link Dist (ft)         325         637         1006         720         1005					4.0								
Recall Mode         Min         C-Min           Act Effct Green (s)         15.4         34.5         37.5         56.6         56.6           Actuated g/C Ratio         0.19         0.43         0.47         0.71         0.71           v/c Ratio         0.65         0.72         0.25         0.65         0.62           Control Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         37.9         16.1         12.1         6.1         1.1           LOS         D         B         B         A         A           Approach Delay         20.2         12.1         4.4           Approach LOS         C         B         A           Queue Length 50th (ft)         104         156         67         104         0           Queue Length 95th (ft)         325         637         1006         720         1           Internal Link Dist (ft)         325         637         1006         720         1           Internal Link Dist (ft)         325         637         1006         720         1	0												
Act Effct Green (s)       15.4       34.5       37.5       56.6       56.6         Actuated g/C Ratio       0.19       0.43       0.47       0.71       0.71         v/c Ratio       0.65       0.72       0.25       0.65       0.62         Control Delay       37.9       16.1       12.1       6.1       1.1         Queue Delay       0.0       0.0       0.0       0.0       0.0         Total Delay       37.9       16.1       12.1       6.1       1.1         LOS       D       B       B       A       A         Approach Delay       20.2       12.1       4.4       4         Approach LOS       C       B       A       A         Queue Length 50th (ft)       104       156       67       104       0         Queue Length 95th (ft)       142       197       93       m240       m2         Internal Link Dist (ft)       325       637       1006       720       700         Base Capacity (vph)       631       1513       1612       2528       1365         Starvation Cap Reductn       0       0       0       0       0       0         Spil													
Actuated g/C Ratio       0.19       0.43       0.47       0.71       0.71         w/c Ratio       0.65       0.72       0.25       0.65       0.62         Control Delay       37.9       16.1       12.1       6.1       1.1         Queue Delay       0.0       0.0       0.0       0.0       0.0         Total Delay       37.9       16.1       12.1       6.1       1.1         LOS       D       B       B       A       A         Approach Delay       20.2       12.1       4.4       A         Approach LOS       C       B       A       A         Queue Length 50th (ft)       104       156       67       104       0         Queue Length 95th (ft)       142       197       93       m240       m2         Internal Link Dist (ft)       325       637       1006       720       700         Base Capacity (vph)       631       1513       1612       2528       1365         Starvation Cap Reductn       0       0       0       0       0       0         Spillback Cap Reductn       0       2       36       0       0       0       0       0							0.4 5					<b>F</b> ( )	<b>F</b> ( )
v/c Ratio         0.65         0.72         0.25         0.65         0.62           Control Delay         37.9         16.1         12.1         6.1         1.1           Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         37.9         16.1         12.1         6.1         1.1           LOS         D         B         B         A         A           Approach Delay         20.2         12.1         4.4         4           Approach LOS         C         B         A         A           Queue Length 50th (ft)         104         156         67         104         0           Queue Length 95th (ft)         325         637         1006         720         100         102         1365           Turn Bay Length (ft)         325         637         1006         720         100         1400         140         150         400         140         151         1612         2528         1365         1365         1365         1365         1365         1365         1365         1365         1365         1365         1365         1365         1365         1365         1365													
Control Delay       37.9       16.1       12.1       6.1       1.1         Queue Delay       0.0       0.0       0.0       0.0       0.0         Total Delay       37.9       16.1       12.1       6.1       1.1         LOS       D       B       B       A       A         Approach Delay       20.2       12.1       4.4       A         Approach LOS       C       B       A       A         Queue Length 50th (ft)       104       156       67       104       0         Queue Length 95th (ft)       142       197       93       m240       m2         Internal Link Dist (ft)       325       637       1006       720       70         Turn Bay Length (ft)       325       637       1006       720       70         Base Capacity (vph)       631       1513       1612       2528       1365         Starvation Cap Reductn       0       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0       0       0         Storage Cap Reductn       0       0       0       0       0       0													
Queue Delay         0.0         0.0         0.0         0.0         0.0           Total Delay         37.9         16.1         12.1         6.1         1.1           LOS         D         B         B         A         A           Approach Delay         20.2         12.1         4.4         4           Approach LOS         C         B         A         A           Queue Length 50th (ft)         104         156         67         104         0           Queue Length 95th (ft)         142         197         93         m240         m2           Internal Link Dist (ft)         325         637         1006         720         100           Turn Bay Length (ft)         1513         1612         2528         1365         1365         5tarvation Cap Reductn         0													
Total Delay       37.9       16.1       12.1       6.1       1.1         LOS       D       B       B       A       A         Approach Delay       20.2       12.1       4.4         Approach LOS       C       B       A         Queue Length 50th (ft)       104       156       67       104       0         Queue Length 95th (ft)       142       197       93       m240       m2         Internal Link Dist (ft)       325       637       1006       720         Turn Bay Length (ft)       1513       1612       2528       1365         Starvation Cap Reductn       0       0       0       0       0         Spillback Cap Reductn       0       2       36       0       0         Storage Cap Reductn       0       0       0       0       0	5												
LOS         D         B         B         A         A           Approach Delay         20.2         12.1         4.4           Approach LOS         C         B         A           Queue Length 50th (ft)         104         156         67         104         0           Queue Length 95th (ft)         142         197         93         m240         m2           Internal Link Dist (ft)         325         637         1006         720         70           Turn Bay Length (ft)         325         631         1513         1612         2528         1365           Starvation Cap Reductn         0         0         0         0         0         0         0           Spillback Cap Reductn         0         0         0         0         0         0         0         0	3												
Approach Delay       20.2       12.1       4.4         Approach LOS       C       B       A         Queue Length 50th (ft)       104       156       67       104       0         Queue Length 95th (ft)       142       197       93       m240       m2         Internal Link Dist (ft)       325       637       1006       720         Turn Bay Length (ft)       325       631       1513       1612       2528       1365         Starvation Cap Reductn       0       0       0       0       0       0       0       0         Spillback Cap Reductn       0       0       0       0       0       0       0       0       0         Storage Cap Reductn       0       0       0       0       0       0       0       0													
Approach LOSCBAQueue Length 50th (ft)104156671040Queue Length 95th (ft)14219793m240m2Internal Link Dist (ft)3256371006720Turn Bay Length (ft)150400Base Capacity (vph)6311513161225281365Starvation Cap Reductn000000Spillback Cap Reductn000000Storage Cap Reductn000000					D		В						A
Oueue Length 50th (ft)         104         156         67         104         0           Queue Length 95th (ft)         142         197         93         m240         m2           Internal Link Dist (ft)         325         637         1006         720           Turn Bay Length (ft)         325         631         1513         1612         2528         1365           Starvation Cap Reductn         0         0         0         0         0         0         0         0           Spillback Cap Reductn         0													
Queue Length 95th (ft)         142         197         93         m240         m2           Internal Link Dist (ft)         325         637         1006         720           Turn Bay Length (ft)         150         400           Base Capacity (vph)         631         1513         1612         2528         1365           Starvation Cap Reductn         0         0         0         0         0         0           Spillback Cap Reductn         0         2         36         0 </td <td></td> <td></td> <td></td> <td></td> <td></td> <td>С</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>						С							
Internal Link Dist (ft)         325         637         1006         720           Turn Bay Length (ft)         150         400           Base Capacity (vph)         631         1513         1612         2528         1365           Starvation Cap Reductn         0         0         0         0         0         0           Spillback Cap Reductn         0         2         36         0         0           Storage Cap Reductn         0         0         0         0         0													
Turn Bay Length (ft)       150       400         Base Capacity (vph)       631       1513       1612       2528       1365         Starvation Cap Reductn       0       0       0       0       0       0         Spillback Cap Reductn       0       2       36       0       0       0       0         Storage Cap Reductn       0       0       0       0       0       0       0					142		197						m2
Base Capacity (vph)         631         1513         1612         2528         1365           Starvation Cap Reductn         0			325			637			1006			720	
Starvation Cap Reductin00000Spillback Cap Reductin023600Storage Cap Reductin00000													
Spillback Cap Reductn023600Storage Cap Reductn00000													
Storage Cap Reductn 0 0 0 0 0												0	0
													0
Reduced v/c Ratio         0.35         0.64         0.26         0.65         0.62	Reduced v/c Ratio				0.35		0.64		0.26			0.65	0.62

PM Peak Hour Existing Conditions

Synchro 9 Report Page 1

Lane Group	Ø1	
LaneConfigurations		
Traffic Volume (vph)		
Future Volume (vph)		
Ideal Flow (vphpl)		
Storage Length (ft)		
Storage Lanes		
Taper Length (ft)		
Right Turn on Red		
Link Speed (mph)		
Link Distance (ft)		
Travel Time (s)		
Peak Hour Factor		
Heavy Vehicles (%)		
Shared Lane Traffic (%)		
Lane Group Flow (vph)		
Turn Type		
Protected Phases	1	
Permitted Phases		
Detector Phase		
Switch Phase		
Minimum Initial (s)	14.0	
Minimum Split (s)	20.0	
Total Split (s)	25.0	
Total Split (%)	31%	
Yellow Time (s)	3.5	
All-Red Time (s)	1.5	
Lost Time Adjust (s)	110	
Total Lost Time (s)		
Lead/Lag	Lead	
Lead-Lag Optimize?	Yes	
Recall Mode	None	
Act Effct Green (s)	Nono	
Actuated g/C Ratio		
v/c Ratio		
Control Delay		
Queue Delay		
Total Delay		
LOS		
Approach Delay		
Approach LOS		
Queue Length 50th (ft)		
Queue Length 95th (ft)		
Internal Link Dist (ft)		
Turn Bay Length (ft)		
Base Capacity (vph)		
Starvation Cap Reductn		
Spillback Cap Reductn		
Storage Cap Reductn		
Reduced v/c Ratio		

PM Peak Hour Existing Conditions

Intersection Summary	
Area Type: Other	
Cycle Length: 80	
Actuated Cycle Length: 80	
Offset: 0 (0%), Referenced to phase 2:NBSB, Start of Yello	DW
Natural Cycle: 60	
Control Type: Actuated-Coordinated	
Maximum v/c Ratio: 0.72	
Intersection Signal Delay: 9.8	Intersection LOS: A
Intersection Capacity Utilization 54.8%	ICU Level of Service A
Analysis Period (min) 15	

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 1: Route 125 & New On-ramp to I-93 NB/I-93 NB Ramp

<b>↓</b> <sub>Ø1</sub>	<b>↓↑</b> <sub>Ø2 (R)</sub>	<b>₽</b> <sub>Ø3</sub>
25 s	23 s	32 s

						-
	∕	$\rightarrow$	1	T.	↓ I	-
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	<b>ካካ</b> 101	104	<b>1</b>	100	F 41	1001
Traffic Volume (vph)	121	104	22	188	541	1001
Future Volume (vph)	121	104	22	188	541	1001
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	200	0			0
Storage Lanes	2	1	1			1
Taper Length (ft)	25		25			
Right Turn on Red		Yes				Yes
Link Speed (mph)	30			30	30	
Link Distance (ft)	707			588	1086	
Travel Time (s)	16.1			13.4	24.7	
Peak Hour Factor	0.66	0.90	0.79	0.81	0.79	0.77
Heavy Vehicles (%)	10%	0%	0%	1%	1%	1%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	183	116	28	232	685	1300
Turn Type		custom	D.P+P	NA	NA	custom
Protected Phases	3	1 2 3!	1!	12	2!	1 2 3!
Permitted Phases	5	120;	2	12	2;	120;
Detector Phase	3		1	2	2	
Switch Phase	5		I	Z	Z	
	9.0		7.0		10.0	
Minimum Initial (s)					10.0	
Minimum Split (s)	14.5		12.0		15.0	
Total Split (s)	25.0		15.0		40.0	
Total Split (%)	31.3%		18.8%		50.0%	
Yellow Time (s)	2.0		3.5		4.0	
All-Red Time (s)	3.5		1.5		1.0	
Lost Time Adjust (s)	-1.5		-1.0		-1.0	
Total Lost Time (s)	4.0		4.0		4.0	
Lead/Lag			Lead		Lag	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		None		C-Min	
Act Effct Green (s)	11.3	80.0	59.1	60.7	55.9	80.0
Actuated g/C Ratio	0.14	1.00	0.74	0.76	0.70	1.00
v/c Ratio	0.41	0.07	0.05	0.16	0.52	0.81
Control Delay	34.0	0.1	2.9	3.1	5.8	9.4
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	34.0	0.0	2.9	3.1	5.8	9.4
LOS	C	A	Α	A	 А	A
Approach Delay	20.8		Λ	3.1	8.1	Λ
	20.8 C					
Approach LOS		0	ſ	A	A	221
Queue Length 50th (ft)	44	0	3	23	75	231
Queue Length 95th (ft)	51	0	8	42	115	209
Internal Link Dist (ft)	627	0.00		508	1006	
Turn Bay Length (ft)		200				
Base Capacity (vph)	835	1615	608	1426	1314	1599
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.22	0.07	0.05	0.16	0.52	0.81

PM Peak Hour Existing Conditions

Intersection Summary		
Area Type:	Other	
Cycle Length: 80		
Actuated Cycle Length: 80	)	
Offset: 14 (18%), Referen	ced to phase 2:NBSB, Start of Yellow	I
Natural Cycle: 65		
Control Type: Actuated-C	oordinated	
Maximum v/c Ratio: 0.81		
Intersection Signal Delay:	9.1	Intersection LOS: A
Intersection Capacity Utili	zation 74.5%	ICU Level of Service D
Analysis Period (min) 15		
! Phase conflict betweer	n lane groups.	

Splits and Phases: 2: Route 125 & I-93 SB Ramp

<b>∜</b> ø1	<b>↓</b> Ø2 (R)		🛠 <sub>Ø3</sub>
15 s	40 s		25 s

	٦	$\mathbf{i}$	1	Ť	Ļ	1	
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	<u> </u>	77	<u> </u>	<u></u>	1001	ODIN	
Traffic Volume (vph)	338	1255	214	<b>T</b> 938	<b>⊤</b> ₽ 779	61	
Future Volume (vph)	338	1255	214	938	779	61	
Ideal Flow (vphpl)	1900	1200	1900	1900	1900	1900	
Storage Length (ft)	0	200	250	1700	1700	0	
Storage Lanes	1	200	1			0	
Taper Length (ft)	25	I	25			0	
Right Turn on Red	20	Yes	20			Yes	
Link Speed (mph)	30	103		30	30	105	
Link Distance (ft)	690			800	604		
Travel Time (s)	15.7			18.2	13.7		
Peak Hour Factor	0.72	0.77	0.91	0.92	0.88	0.73	
Heavy Vehicles (%)	1%	1%	15%	2%	2%	5%	
Shared Lane Traffic (%)	I /0	1 70	1070	2 /0	2 /0	J /0	
Lane Group Flow (vph)	469	1630	235	1020	969	0	
Turn Type	469 Prot		custom	NA	969 NA	U	
Protected Phases	P101 3	pt+ov 1 3	CUSI0111	NA 12	NA 2		
Permitted Phases	3	13	1	ΙZ	Z		
Detector Phase	3	1	1	2	2		
Switch Phase	3	1	1	2	Z		
	10		0 0		10.0		
Minimum Initial (s)	4.0		8.0		10.0		
Minimum Split (s)	10.5		15.0		20.0		
Total Split (s)	22.0		24.0		34.0		
Total Split (%)	27.5%		30.0%		42.5%		
Yellow Time (s)	3.5		3.5		4.0		
All-Red Time (s)	3.0 2.5		3.5		1.5 1 E		
Lost Time Adjust (s)	-2.5		-3.0		-1.5		
Total Lost Time (s)	4.0		4.0		4.0		
Lead/Lag			Lead		Lag		
Lead-Lag Optimize?	N I -		N I		C MAL		
Recall Mode	None	10 7	None	F 0 0	C-Min		
Act Effct Green (s)	18.7	42.7	20.0	53.3	29.3		
Actuated g/C Ratio	0.23	0.53	0.25	0.67	0.37		
v/c Ratio	1.12	1.06	0.31	0.82	0.75		
Control Delay	113.6	60.4	21.4	12.9	26.2		
Queue Delay	0.0	0.0	0.0	0.0	0.0		
Total Delay	113.6	60.4	21.4	12.9	26.2		
LOS	F	E	С	В	С		
Approach Delay	72.3			14.5	26.2		
Approach LOS	E			В	С		
Queue Length 50th (ft)	~283	~513	32	350	212		
Queue Length 95th (ft)	#326	#494	60	482	274		
Internal Link Dist (ft)	610			720	524		
Turn Bay Length (ft)		200	250				
Base Capacity (vph)	418	1540	761	1257	1315		
Starvation Cap Reductn	0	0	0	0	0		
Spillback Cap Reductn	0	0	0	0	0		
Storage Cap Reductn	0	0	0	0	0		
Reduced v/c Ratio	1.12	1.06	0.31	0.81	0.74		

PM Peak Hour Existing Conditions

Intersection Summary		
Area Type: Other		
Cycle Length: 80		
Actuated Cycle Length: 80		
Offset: 0 (0%), Referenced to phase 2:NBSB, Start of Y	/ellow	
Natural Cycle: 80		
Control Type: Actuated-Coordinated		
Maximum v/c Ratio: 1.12		
Intersection Signal Delay: 45.2	Intersection LOS: D	
Intersection Capacity Utilization 74.8%	ICU Level of Service D	
Analysis Period (min) 15		
<ul> <li>Volume exceeds capacity, queue is theoretically inf</li> </ul>	inite.	
Queue shown is maximum after two cycles.		
# 95th percentile volume exceeds capacity, queue ma	ay be longer.	
Queue shown is maximum after two cycles.		

Splits and Phases: 3: Route 125 & Ballardvale Street

<b>\$</b> 01	<b>↓</b> ¶ <sub>Ø2 (R)</sub>	🖈 ø3	
24 s	34 s	22 s	

Location 1 - I-93 Northbound between Exit 40 (Route 2) and Exit 41 (Route 125) Freeway Merge, Diverge, and Weave Analyses

### HCS7 Freeway Merge Report

Project Information						
•	en Erban	Date	4/25/2019	)		
,	TPS	Analysis Year	2019			
	1assDOT District 4	Time Period Analyzed	AM Peak I	lour 7:00-8:00		
fr	Vilmington - Exit 40 On-Ramp rom Route 62 - Low Cost reeway Bottlenecks	Unit	United Sta	United States Customary		
Geometric Data						
		Freeway	Ramp			
Number of Lanes (N), In		4	1			
Free-Flow Speed (FFS), mi/h		75.4	30.0			
Segment Length (L) / Acceleration Len	ngth (LA),ft	1200	300			
Terrain Type		Level	Level			
Percent Grade, %		-	-			
Segment Type / Ramp Side		Freeway	Right			
Adjustment Factors						
Driver Population		Mostly Familiar	Mostly Fa	miliar		
Weather Type		Non-Severe Weather	e Weather			
Incident Type		No Incident	-			
Final Speed Adjustment Factor (SAF)		0.975	0.975			
Final Capacity Adjustment Factor (CA	F)	0.968	0.968			
Demand Adjustment Factor (DAF)		1.000	1.000			
Demand and Capacity						
Demand Volume (Vi)		4950	450			
Peak Hour Factor (PHF)		0.95				
Total Trucks, %		5.00				
Single-Unit Trucks (SUT), %		-				
Tractor-Trailers (TT), %		-	-			
Heavy Vehicle Adjustment Factor (fHV	()	0.952	0.952			
Flow Rate (vi),pc/h		5473	498			
Capacity (c), pc/h		9293	1839			
Volume-to-Capacity Ratio (v/c)		0.64	0.27			
Speed and Density						
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on Fr	eeway (NO)	2		
Distance to Upstream Ramp (LUP), ft	-	Speed Index (MS)		0.361		
Downstream Equilibrium Distance (LE	Q), ft -	Flow Outer Lanes (vOA), pc/h/	/In	1642		
Distance to Downstream Ramp (LDOW	vn), ft -	On-Ramp Influence Area Spe	ed (SR), mi/h	62.1		
Prop. Freeway Vehicles in Lane 1 and	2 (Рғм) 0.156	Outer Lanes Freeway Speed (	SO), mi/h	69.4		
Flow in Lanes 1 and 2 (v12), pc/h	2189	Ramp Junction Speed (S), mi/	′h	65.9		
Flow Entering Ramp-Infl. Area (vR12),	pc/h 2687	Average Density (D), pc/mi/ln	1	22.7		
Level of Service (LOS)	С	Density in Ramp Influence Ar	ea (DR), pc/mi/ln	24.4		

Service Volume Table												
Target LOS		A		В			с		D			E
- reeway												
Max Service Flow Rate (MS	SF), pc/h/ln	410		1	1077	1	603		206	7		2130
Service Flow Rate (SF), veh	/h	1560		4	4100	6	5104		787	0		8110
Service Volume, veh/h		1482		3	3895	5	5799		747	7		7704
One Direction DSV, 1000 v	eh/day	15			39		58		75			77
Bi-Directional DSV, 1000 v	eh/day	27			71		105		136			140
Ramp												
Max Service Flow Rate (MS	SF), pc/h/ln	149		392		583			752			774
Service Flow Rate (SF), veh	/h	142			373	555			715			737
Service Volume, veh/h		135			354		527		680	)		700
One Direction DSV, 1000 v	eh/day	1			4		5 7		7			7
Design Analysis Ta	ble											
Freeway Lanes, In	2	2	2 3		3	4		4		5		5
Ramp Lanes, In	1	2	1		2	1		2		1		2
Density, pc/mi/ln	-	-	32.2		31.9	22.7	22.7 22		22.5			17.8
LOS	F	F	D		D	С	c c		c C			В

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### HCS7 Freeway Merge Report

		,	_				
Project Information							
Analyst B	en Erban	Date	4/25/2019				
5	TPS	Analysis Year	2019				
Jurisdiction N	lassDOT District 4	Time Period Analyzed	PM Peak H	lour 5:00-6:00			
fr	/ilmington - Exit 40 On-Ramp om Route 62 - Low Cost reeway Bottlenecks	Unit	United Sta	tes Customary			
Geometric Data							
		Freeway	Ramp				
Number of Lanes (N), In		4	1				
Free-Flow Speed (FFS), mi/h		75.4	30.0				
Segment Length (L) / Acceleration Ler	ngth (LA),ft	1200	300				
Terrain Type		Level	Level				
Percent Grade, %		-	-				
Segment Type / Ramp Side		Freeway	Right				
Adjustment Factors							
Driver Population		Mostly Familiar	Mostly Far	miliar			
Weather Type		Non-Severe Weather	re Weather				
Incident Type		No Incident					
Final Speed Adjustment Factor (SAF)		0.975	0.975				
Final Capacity Adjustment Factor (CAI	-)	0.968	0.968				
Demand Adjustment Factor (DAF)		1.000	1.000 1.000				
Demand and Capacity							
Demand Volume (Vi)		7200	350				
Peak Hour Factor (PHF)		0.95					
Total Trucks, %		3.00					
Single-Unit Trucks (SUT), %		-					
Tractor-Trailers (TT), %		-					
Heavy Vehicle Adjustment Factor (fHV	)	0.971	0.971				
Flow Rate (vi),pc/h		7805	379				
Capacity (c), pc/h		9293	1839				
Volume-to-Capacity Ratio (v/c)		0.88	0.21				
Speed and Density							
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on F	reeway (NO)	2			
Distance to Upstream Ramp (LUP), ft	-	Speed Index (MS)		0.433			
Downstream Equilibrium Distance (LE	ຊ), ft -	Flow Outer Lanes (vOA), pc/h	2342				
Distance to Downstream Ramp (LDOW	/N), ft -	On-Ramp Influence Area Spe	eed (SR), mi/h	59.9			
Prop. Freeway Vehicles in Lane 1 and	2 (Рғм) 0.170	Outer Lanes Freeway Speed	(SO), mi/h	66.7			
Flow in Lanes 1 and 2 (v12), pc/h	3122	Ramp Junction Speed (S), mi,	/h	63.6			
Flow Entering Ramp-Infl. Area (vR12),	pc/h 3501	Average Density (D), pc/mi/lr	n	32.2			
Level of Service (LOS)	D	Density in Ramp Influence Ar	rea (DR), pc/mi/ln	30.8			

Service Volume Table											
Target LOS	A	В	с	D	E						
Freeway	Freeway										
Max Service Flow Rate (MSF), pc/h/ln	458	1173	1746	2216	-						
Service Flow Rate (SF), veh/h	1778	4554	6781	8605	-						
Service Volume, veh/h	1689	4327	6442	8175	-						
One Direction DSV, 1000 veh/day	17	43	64	82	-						
Bi-Directional DSV, 1000 veh/day	31	79	117	149	-						
Ramp											
Max Service Flow Rate (MSF), pc/h/ln	89	228	340	431	-						
Service Flow Rate (SF), veh/h	86	221	330	418	-						
Service Volume, veh/h	82	210	313	397	-						
One Direction DSV, 1000 veh/day	1	2	3	4	-						

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# HCS7 Freeway Diverge Report

Project Information					
Analyst B	en Erban	Date	4/25/2019		
Agency C	TPS	Analysis Year	2019		
Jurisdiction N	lassDOT District 4	Time Period Analyzed	AM Peak P	Period 7:00-8:00	
R	/ilmington - Exit 41 Off-Ramp to oute 125 - Low Cost Freeway ottlenecks	Unit	United Sta	tes Customary	
Geometric Data		•			
		Freeway	Ramp		
Number of Lanes (N), In		4	1		
Free-Flow Speed (FFS), mi/h		75.4	30.0		
Segment Length (L) / Deceleration Le	ngth (LA),ft	875	400		
Terrain Type		Level	Level		
Percent Grade, %		-	-		
Segment Type / Ramp Side		Freeway	Right		
Adjustment Factors					
Driver Population		Mostly Familiar	Mostly Far	niliar	
Weather Type		Non-Severe Weather	e Weather		
Incident Type		No Incident			
Final Speed Adjustment Factor (SAF)		0.975	0.975		
Final Capacity Adjustment Factor (CA	F)	0.968	0.968		
Demand Adjustment Factor (DAF)		1.000	1.000		
Demand and Capacity					
Demand Volume (Vi)		5400	750		
Peak Hour Factor (PHF)		0.95			
Total Trucks, %		5.00			
Single-Unit Trucks (SUT), %		-			
Tractor-Trailers (TT), %		-			
Heavy Vehicle Adjustment Factor (fHV	)	0.952	0.952		
Flow Rate (vi),pc/h		5971	829		
Capacity (c), pc/h		9293	1839		
Volume-to-Capacity Ratio (v/c)		0.64	0.45		
Speed and Density					
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on F	reeway (NO)	2	
Distance to Upstream Ramp (LUP), ft	-	Speed Index (DS)		0.578	
Downstream Equilibrium Distance (LE	Q), ft -	Flow Outer Lanes (vOA), pc/h	n/ln	1450	
Distance to Downstream Ramp (LDOW	Off-Ramp Influence Area Sp	eed (SR), mi/h	55.3		
Prop. Freeway Vehicles in Lane 1 and	2 (Pfd) 0.436	Outer Lanes Freeway Speed	78.9		
Flow in Lanes 1 and 2 (v12), pc/h	3071	Ramp Junction Speed (S), m	64.7		
Flow Entering Ramp-Infl. Area (vR12),	pc/h -	Average Density (D), pc/mi/ln         23.1			
Level of Service (LOS)	С	Density in Ramp Influence A	rea (DR), pc/mi/ln	27.1	

Service Volume Tab	Service Volume Table											
Target LOS		A			В		с		D			E
Freeway		- -		<u>.</u>								
Max Service Flow Rate (MS	F), pc/h/ln	520		-	1091		1545		194	0		2323
Service Flow Rate (SF), veh/	'n	1981		2	4154		5882		738	9		8847
Service Volume, veh/h		1882		3	3946		5587		7020			8404
One Direction DSV, 1000 ve	eh/day	19			39		56		70			84
Bi-Directional DSV, 1000 ve	h/day	34			72		102		128	3		153
Ramp				<u>.</u>								
Max Service Flow Rate (MS	F), pc/h/ln	289			606		858		107	8		1291
Service Flow Rate (SF), veh/	'n	275			577		817		102	6		1229
Service Volume, veh/h		261			548		776		975	5		1167
One Direction DSV, 1000 ve	eh/day	3			5		8		10			12
Design Analysis Tak	ole											
Freeway Lanes, In	2	2	3		3	4	4	4		5		5
Ramp Lanes, In	1	2	1		2	1	1	2		1		2
Density, pc/mi/ln	-	-	32.4		32.0	2	23.1	22.3		18.5		17.7
LOS	F	F	D		С	(	C	В		С		В

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# HCS7 Freeway Diverge Report

Project Information						
Analyst B	en Erban	Date	4/25/2019			
Agency C	TPS	Analysis Year	2019			
Jurisdiction N	lassDOT District 4	Time Period Analyzed	PM Peak P	eriod 5:00-6:00		
R	Vilmington - Exit 41 Off-Ramp to oute 125 - Low Cost Freeway ottlenecks	Unit	United Sta	tes Customary		
Geometric Data						
		Freeway	Ramp			
Number of Lanes (N), In		4	1			
Free-Flow Speed (FFS), mi/h		75.4	30.0			
Segment Length (L) / Deceleration Le	ngth (LA),ft	875	400			
Terrain Type		Level	Level			
Percent Grade, %		-	-			
Segment Type / Ramp Side		Freeway	Right			
Adjustment Factors		1				
Driver Population		Mostly Familiar	Mostly Far	niliar		
Weather Type		Non-Severe Weather	Non-Sever	e Weather		
Incident Type		No Incident	-			
Final Speed Adjustment Factor (SAF)		0.975	0.975			
Final Capacity Adjustment Factor (CA	F)	0.968	0.968			
Demand Adjustment Factor (DAF)		1.000	1.000			
Demand and Capacity		8				
Demand Volume (Vi)		7550	1050			
Peak Hour Factor (PHF)		0.95	0.95			
Total Trucks, %		3.00	3.00			
Single-Unit Trucks (SUT), %		-	-			
Tractor-Trailers (TT), %		-	-			
Heavy Vehicle Adjustment Factor (fHV	/)	0.971	0.971			
Flow Rate (vi),pc/h		8185	1138			
Capacity (c), pc/h		9293	1839			
Volume-to-Capacity Ratio (v/c)		0.88	0.62			
Speed and Density		-				
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on F	reeway (NO)	2		
Distance to Upstream Ramp (LUP), ft	-	Speed Index (DS)		0.606		
Downstream Equilibrium Distance (LE	Q), ft -	Flow Outer Lanes (vOA), pc/h	ı/ln	1988		
Distance to Downstream Ramp (LDOV	vn), ft -	Off-Ramp Influence Area Speed (SR), mi/h 54.4				
Prop. Freeway Vehicles in Lane 1 and	2 (PFD) 0.436	Outer Lanes Freeway Speed (SO), mi/h 76.8				
Flow in Lanes 1 and 2 (v12), pc/h	4210	Ramp Junction Speed (S), mi/h 63.4				
Flow Entering Ramp-Infl. Area (vR12),	pc/h -	Average Density (D), pc/mi/ln32.3				
Level of Service (LOS)	E	Density in Ramp Influence Area (DR), pc/mi/ln 36.9				

Service Volume Table										
Target LOS	A	В	с	D	E					
Freeway										
Max Service Flow Rate (MSF), pc/h/ln	521	1092	1544	1939	2323					
Service Flow Rate (SF), veh/h	2025	4242	5998	7530	9023					
Service Volume, veh/h	1924	4029	5699	7153	8572					
One Direction DSV, 1000 veh/day	19	40	57	72	86					
Bi-Directional DSV, 1000 veh/day	35	73	104	130	156					
Ramp										
Max Service Flow Rate (MSF), pc/h/ln	290	608	859	1078	1292					
Service Flow Rate (SF), veh/h	282	590	834	1047	1255					
Service Volume, veh/h	268	560	793	995	1192					
One Direction DSV, 1000 veh/day	3	6	8	10	12					

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### HCS7 Freeway Merge Report

Project Information					
Analyst Be	en Erban	Date	4/25/2019	1	
Agency C	TPS	Analysis Year	2030		
Jurisdiction N	lassDOT District 4	Time Period Analyzed	AM Peak H	Hour 7:00-8:00	
fr	/ilmington - Exit 40 On-Ramp om Route 62 - Low Cost reeway Bottlenecks	Unit	United Sta	ites Customary	
Geometric Data					
		Freeway	Ramp		
Number of Lanes (N), In		4	1		
Free-Flow Speed (FFS), mi/h		75.4	30.0		
Segment Length (L) / Acceleration Ler	ngth (LA),ft	1200	300		
Terrain Type		Level	Level		
Percent Grade, %		-	-		
Segment Type / Ramp Side		Freeway	Right		
Adjustment Factors					
Driver Population		Mostly Familiar	Mostly Far	miliar	
Weather Type		Non-Severe Weather	Non-Seve	re Weather	
Incident Type		No Incident			
Final Speed Adjustment Factor (SAF)		0.975	0.975		
Final Capacity Adjustment Factor (CA	-)	0.968	0.968		
Demand Adjustment Factor (DAF)		1.000	1.000		
Demand and Capacity					
Demand Volume (Vi)		5170	470		
Peak Hour Factor (PHF)		0.95	0.95		
Total Trucks, %		5.00	5.00		
Single-Unit Trucks (SUT), %		-	-		
Tractor-Trailers (TT), %		-	-		
Heavy Vehicle Adjustment Factor (fHV	)	0.952	0.952		
Flow Rate (vi),pc/h		5716	520		
Capacity (c), pc/h		9293	1839		
Volume-to-Capacity Ratio (v/c)		0.67	0.28		
Speed and Density					
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on Fre	eway (No)	2	
Distance to Upstream Ramp (LUP), ft	-	Speed Index (MS)		0.368	
Downstream Equilibrium Distance (LE	Q), ft -	Flow Outer Lanes (vOA), pc/h/ln 1715			
Distance to Downstream Ramp (LDOW	/N), ft -	On-Ramp Influence Area Spee	d (SR), mi/h	61.9	
Prop. Freeway Vehicles in Lane 1 and	2 (Рғм) 0.153	Outer Lanes Freeway Speed (SO), mi/h 69.1			
Flow in Lanes 1 and 2 (v12), pc/h	2286	Ramp Junction Speed (S), mi/h 65.7			
Flow Entering Ramp-Infl. Area (vR12),	pc/h 2806	Average Density (D), pc/mi/ln 23.7			
Level of Service (LOS)	Service (LOS) C Density in Ramp Influence Area (DR), pc/mi/ln 25.3				

Service Volume Tab	Service Volume Table											
Target LOS		A			В		с	Τ	D			E
Freeway												
Max Service Flow Rate (MSI	F), pc/h/ln	410		1	1077		1603		206	7		2130
Service Flow Rate (SF), veh/	'n	1560		2	4100		6104		787	0		8110
Service Volume, veh/h		1482		3	3895		5799		747	7		7704
One Direction DSV, 1000 ve	eh/day	15			39		58		75			77
Bi-Directional DSV, 1000 ve	h/day	27			71		105		136	<u>5</u>		140
Ramp												
Max Service Flow Rate (MSI	F), pc/h/ln	149			392		583		752	2		774
Service Flow Rate (SF), veh/	'n	142	142		373		555		715	5		737
Service Volume, veh/h		135			354		527		680	)		700
One Direction DSV, 1000 ve	eh/day	1			4		5		7			7
Design Analysis Tak	ole											
Freeway Lanes, In	2	2	3		3	4	ļ.	4		5		5
Ramp Lanes, In	1	2	1		2	1		2		1		2
Density, pc/mi/ln	-	-	34.0		33.7	2	23.7	23.6		18.7		18.6
LOS	F	F	D		D	С	2	С		С		В

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### HCS7 Freeway Merge Report

		<i>y</i>				
Project Information						
,	en Erban	Date	4/25/2019			
5	TPS	Analysis Year	2030			
Jurisdiction N	lassDOT District 4	Time Period Analyzed	PM Peak H	lour 5:00-6:00		
fr	/ilmington - Exit 40 On-Ramp om Route 62 - Low Cost reeway Bottlenecks	Unit	United Sta	tes Customary		
Geometric Data						
		Freeway	Ramp			
Number of Lanes (N), In		4	1			
Free-Flow Speed (FFS), mi/h		75.4	30.0			
Segment Length (L) / Acceleration Len	ngth (LA),ft	1200	300			
Terrain Type		Level	Level			
Percent Grade, %		-	-			
Segment Type / Ramp Side		Freeway	Right			
Adjustment Factors						
Driver Population		Mostly Familiar	Mostly Far	niliar		
Weather Type		Non-Severe Weather	Non-Sever	re Weather		
Incident Type		No Incident	-			
Final Speed Adjustment Factor (SAF)		0.975	0.975			
Final Capacity Adjustment Factor (CA	F)	0.968	0.968			
Demand Adjustment Factor (DAF)		1.000	1.000			
Demand and Capacity						
Demand Volume (Vi)		7480	400			
Peak Hour Factor (PHF)		0.95	0.95			
Total Trucks, %		3.00	3.00			
Single-Unit Trucks (SUT), %		-	-			
Tractor-Trailers (TT), %		-	-			
Heavy Vehicle Adjustment Factor (fHV	)	0.971	0.971			
Flow Rate (vi),pc/h		8109	434			
Capacity (c), pc/h		9293	1839			
Volume-to-Capacity Ratio (v/c)		0.92	0.24			
Speed and Density						
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on Fi	reeway (NO)	2		
Distance to Upstream Ramp (LUP), ft	-	Speed Index (MS) 0.45		0.458		
Downstream Equilibrium Distance (LE	Q), ft -	Flow Outer Lanes (vOA), pc/h/ln 2433				
Distance to Downstream Ramp (LDOW	/N), ft -	On-Ramp Influence Area Speed (SR), mi/h 59.1				
Prop. Freeway Vehicles in Lane 1 and	2 (Рғм) 0.164	Outer Lanes Freeway Speed (SO), mi/h 66.2				
Flow in Lanes 1 and 2 (v12), pc/h	3244	Ramp Junction Speed (S), mi/h 62.9				
Flow Entering Ramp-Infl. Area (vR12),	pc/h 3678	Average Density (D), pc/mi/ln34.0				
Level of Service (LOS)	D	Density in Ramp Influence Area (DR), pc/mi/ln 32.2				

Service Volume Tal	Service Volume Table											
Target LOS		A			В		С	Т	D			E
Freeway												
Max Service Flow Rate (MS	SF), pc/h/ln	444		ŕ	162		1732		220	5		-
Service Flow Rate (SF), veh	/h	1725		4	4512		6727		8565			-
Service Volume, veh/h		1639		4	1287		6391		813	7		-
One Direction DSV, 1000 v	eh/day	16			43		64		81			-
Bi-Directional DSV, 1000 v	eh/day	30			78		116		148			-
Ramp		·										
Max Service Flow Rate (MS	F), pc/h/ln	95			249		371		472	2		-
Service Flow Rate (SF), veh	/h	92			241		360		458	3		-
Service Volume, veh/h		88			229		342		43	5		-
One Direction DSV, 1000 v	eh/day	1			2		3		4			-
Design Analysis Ta	ble											
Freeway Lanes, In	2	2	3		3	4		4		5		5
Ramp Lanes, In	1	2	1		2	1		2	1			2
Density, pc/mi/ln	-	-	-		-	34	.0	33.8	.8 26.0			25.9
LOS	F	F	F		F	D		D		С		С

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#### HCS7 Freeway Diverge Report

		STITCEWay	Diverge Report	L				
Project Information								
Analyst Be	en Erban		Date		4/25/2019			
Agency C	TPS		Analysis Year		2030			
Jurisdiction M	lassDOT Distri	ct 4	Time Period Analyzed		AM Peak P	eriod 7:00-8:00		
		kit 41 Off-Ramp - ay Bottlenecks	Unit		United Stat	tes Customary		
Geometric Data								
			Freeway		Ramp			
Number of Lanes (N), In			4		1			
Free-Flow Speed (FFS), mi/h			75.4		30.0			
Segment Length (L) / Deceleration Lei	ngth (LA),ft		875		400			
Terrain Type			Level		Level			
Percent Grade, %			-		-			
Segment Type / Ramp Side			Freeway		Right			
Adjustment Factors								
Driver Population			Mostly Familiar		Mostly Fan	niliar		
Weather Type			Non-Severe Weather		Non-Sever	e Weather		
Incident Type			No Incident		-			
Final Speed Adjustment Factor (SAF)	nal Speed Adjustment Factor (SAF)				0.975			
Final Capacity Adjustment Factor (CAF	F)		0.968		0.968			
Demand Adjustment Factor (DAF)			1.000		1.000			
Demand and Capacity								
Demand Volume (Vi)			5640		800			
Peak Hour Factor (PHF)			0.95		0.95			
Total Trucks, %			5.00		5.00			
Single-Unit Trucks (SUT), %			-		-			
Tractor-Trailers (TT), %			-		-			
Heavy Vehicle Adjustment Factor (fHV	/)		0.952		0.952			
Flow Rate (vi),pc/h			6236		885			
Capacity (c), pc/h			9293		1839			
Volume-to-Capacity Ratio (v/c)			0.67		0.48			
Speed and Density								
Upstream Equilibrium Distance (LEQ),	ft -		Number of Outer Lanes	on Freeway	y (NO)	2		
Distance to Upstream Ramp (LUP), ft	-		Speed Index (DS)		0.583			
Downstream Equilibrium Distance (LEG	Q), ft -		Flow Outer Lanes (vOA)	, pc/h/ln		1509		
Distance to Downstream Ramp (LDOW	vn), ft -		Off-Ramp Influence Area Speed (SR), mi/h 55.1					
Prop. Freeway Vehicles in Lane 1 and	2 (PFD) 0.43	36	Outer Lanes Freeway Speed (SO), mi/h         78.6			78.6		
Flow in Lanes 1 and 2 (v12), pc/h	321	8	Ramp Junction Speed (S), mi/h 64.4			64.4		
Flow Entering Ramp-Infl. Area (vR12), J	pc/h -		Average Density (D), pc/mi/ln         24.2			24.2		
Level of Service (LOS)	D		Density in Ramp Influer	nce Area (DF	R), pc/mi/ln	28.3		

Service Volume Table												
Target LOS		A			В		с	Τ	D			E
Freeway												
Max Service Flow Rate (M	SF), pc/h/ln	518		-	1087		1539		193	2		2323
Service Flow Rate (SF), ver	ı/h	1973		4	1141		5862		735	9		8847
Service Volume, veh/h		1875		3	3934		5569		6991			8404
One Direction DSV, 1000	/eh/day	19			39		56		70	)		84
Bi-Directional DSV, 1000 v	eh/day	34			72		101		12	7	153	
Ramp												
Max Service Flow Rate (M	SF), pc/h/ln	294			617		873		109	6		1318
Service Flow Rate (SF), ver	ı/h	280	280		587		832		104	4		1255
Service Volume, veh/h		266			558		790		992	2		1192
One Direction DSV, 1000	/eh/day	3			6		8		10	)		12
Design Analysis Ta	ble											
Freeway Lanes, In	2	2	3		3	4		4		5		5
Ramp Lanes, In	1	2	1		2	1		2		1		2
Density, pc/mi/ln	-	-	33.9		33.6 24.2 23.4 19.4			18.6				
LOS	F	F	D		С	D	)	В		С		В

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### HCS7 Freeway Diverge Report

	l		Diverge Report			
Project Information						
Analyst B	Ben Erban		Date	4/25/2019		
Agency C	CTPS		Analysis Year	2030		
Jurisdiction N	/lassDOT [	District 4	Time Period Analyzed	PM Peak P	eriod 5:00-6:00	
Project Description V L	Vilmingtoı .ow Cost F	n - Exit 41 Off-Ramp - reeway Bottlenecks	Unit	United Sta	tes Customary	
Geometric Data						
			Freeway	Ramp		
Number of Lanes (N), In			4	1		
Free-Flow Speed (FFS), mi/h			75.4	30.0		
Segment Length (L) / Deceleration Le	ength (LA),	ft	875	400		
Terrain Type			Level	Level		
Percent Grade, %			-	-		
Segment Type / Ramp Side			Freeway	Right		
Adjustment Factors						
Driver Population			Mostly Familiar	Mostly Far	niliar	
Weather Type			Non-Severe Weather	Non-Sever	e Weather	
Incident Type			No Incident	-		
Final Speed Adjustment Factor (SAF)			0.975	0.975		
Final Capacity Adjustment Factor (CA	F)		0.968	0.968		
Demand Adjustment Factor (DAF)			1.000	1.000		
Demand and Capacity						
Demand Volume (Vi)			7880	1100		
Peak Hour Factor (PHF)			0.95	0.95		
Total Trucks, %			3.00	3.00		
Single-Unit Trucks (SUT), %			-	-		
Tractor-Trailers (TT), %			-	-		
Heavy Vehicle Adjustment Factor (fH\	V)		0.971	0.971		
Flow Rate (vi),pc/h			8542	1192		
Capacity (c), pc/h			9293	1839		
Volume-to-Capacity Ratio (v/c)			0.92	0.65		
Speed and Density						
Upstream Equilibrium Distance (LEQ),	ft	-	Number of Outer Lanes on Freev	vay (NO)	2	
Distance to Upstream Ramp (LUP), ft		-	Speed Index (Ds) 0.611			
Downstream Equilibrium Distance (LE	Q), ft	-	Flow Outer Lanes (vOA), pc/h/ln 2073			
Distance to Downstream Ramp (LDOV	WN), ft	-	Off-Ramp Influence Area Speed (SR), mi/h 54.3			
Prop. Freeway Vehicles in Lane 1 and	2 (Pfd)	0.436	Outer Lanes Freeway Speed (SO), mi/h 76.4			
Flow in Lanes 1 and 2 (v12), pc/h		4397	Ramp Junction Speed (S), mi/h 63.2			
Flow Entering Ramp-Infl. Area (vR12),	pc/h	-	Average Density (D), pc/mi/ln 33.8			
Level of Service (LOS)		E	Density in Ramp Influence Area (DR), pc/mi/ln 38.5			

Service Volume Tak	Service Volume Table											
Target LOS		A			В		с	Τ	D			E
Freeway		·										
Max Service Flow Rate (MS	F), pc/h/ln	521	521		1091		1543		193	8		2323
Service Flow Rate (SF), veh,	/h	2024		2	1236		5993		7525			9023
Service Volume, veh/h		1923		2	1024		5694		714	.9		8572
One Direction DSV, 1000 ve	eh/day	19			40		57		71			86
Bi-Directional DSV, 1000 ve	eh/day	35			73		104		130			156
Ramp												
Max Service Flow Rate (MS	F), pc/h/ln	291			609		862		108	2		1297
Service Flow Rate (SF), veh,	/h	283	283		591		837		105	0		1260
Service Volume, veh/h		268			562		795		998	8		1197
One Direction DSV, 1000 ve	eh/day	3			6		8		10	)		12
Design Analysis Tal	ble			-		-						
Freeway Lanes, In	2	2	3		3	4		4		5		5
Ramp Lanes, In	1	2	1		2	1		2		1		2
Density, pc/mi/ln	-	-	-		-	33	3.8	33.0	3.0 27.0			26.0
LOS	F	F	F		F	E		C		D		с

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#### HCS7 Freeway Weaving Report

г	ICS/ Freeway	weaving Repo	rt			
Project Information						
Analyst	Chen-Yuan Wang	Date	6/20/2019			
Agency	CTPS	Analysis Year		2030		
Jurisdiction	MassDOT District 4	Time Period Analyzed		AM Peak Hour 7:00-8:00		
Project Description	I-93 Northbound Between Exit 40 and Exit 41 in Wilmington - Low Cst Freeway Bottlenecks	Unit		United States Customary		
Geometric Data						
Number of Lanes (N), ln	5	Segment Type		Freeway		
Segment Length (Ls), ft	1200	Number of Maneuver	Lanes (NWL), In	2		
Weaving Configuration	One-Sided	Ramp-to-Freeway Lane	e Changes (LCRF), lc	1		
Terrain Type	Level	Freeway-to-Ramp Lane	e Changes (LCFR), lc	1		
Percent Grade, %	-	Ramp-to-Ramp Lane C	Changes (LCRR), lc	0		
Interchange Density (ID), int/mi	0.66	Cross Weaving Manag	ed Lane	No		
Adjustment Factors	•			•		
Driver Population	Mostly Familiar	Final Speed Adjustmer	nt Factor (SAF)	0.975		
Weather Type	Non-Severe Weather	Final Capacity Adjustm	ent Factor (CAF)	0.968		
Incident Type	No Incident	Demand Adjustment F	actor (DAF)	1.000		
Demand and Capacity	-	•				
	FF	RF	RR	FR		
Demand Volume (Vi), veh/h	4390	470	0	800		
Peak Hour Factor (PHF)	0.95	0.95	0.95	0.95		
Total Trucks, %	5.00	5.00	5.00	5.00		
Heavy Vehicle Adjustment Factor (fHv)	0.952	0.952	0.952	0.952		
Flow Rate (vi), pc/h	4854	520	0	885		
Weaving Flow Rate (vw), pc/h	1405	Freeway Max Capacity	(cIFL), pc/h/ln	2400		
Non-Weaving Flow Rate (vNW), pc/h	4854	Density-Based Capacity	y (cIWL), pc/h/ln	2126		
Total Flow Rate (v), pc/h	6259	Demand Flow-Based C	apacity (cɪw), pc/h	10714		
Volume Ratio (VR)	0.224	Weaving Segment Cap	acity (cw), veh/h	10120		
Minimum Lane Change Rate (LCMIN), lc/h	1405	Adjusted Weaving Area	a Capacity, pc/h	10290		
Maximum Weaving Length (LMAX), ft	4783	Volume-to-Capacity Ra	atio (v/c)	0.61		
Speed and Density	-	2				
Non-Weaving Vehicle Index (INW)	384	Average Weaving Spee	56.6			
Non-Weaving Lane Change Rate (LCNW), lc/h	687	Average Non-Weaving	Speed (SNW), mi/h	57.4		
Weaving Lane Change Rate (LCW), lc/h	1844	Average Speed (S), mi/	′h	57.2		
Weaving Lane Change Rate (LCAII), Ic/h	2531	Density (D), pc/mi/ln	21.9			
Weaving Intensity Factor (W)	0.407	Level of Service (LOS)		С		

Service Volume Ta	ble						
Target LOS		A	В	с	D		E
Max Service Flow Rate (M	SF), pc/h/ln	644	1162	1518	178	9	2058
Service Flow Rate (SF), vel	ı/h	3067	5531	7223	851	7	9796
Service Volume, veh/h		2914	5254	6862	809	2	9306
One Direction DSV, 1000	veh/day	29	53	69	81		93
Bi-Directional DSV, 1000 v	/eh/day	53	96	125	14	7	169
Design Analysis Ta	ble						-
Number of Lanes, In	3	4		5		6	
Density, pc/mi/ln	-	2	7.9	21.9		18.0	
LOS	F	C	2	С		В	
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F	HCS/ Freeway	Weaving Repo	rt	
Project Information				
Analyst	Chen-Yuan Wang	Date		6/20/2019
Agency	CTPS	Analysis Year	2030	
Jurisdiction	MassDOT District 4	Time Period Analyzed	PM Peak Hour 5:00-6:00	
Project Description	I-93 Northbound Between Exit 40 and Exit 41 in Wilmington - Low Cst Freeway Bottlenecks	Unit	United States Customary	
Geometric Data				
Number of Lanes (N), In	5	Segment Type		Freeway
Segment Length (Ls), ft	1200	Number of Maneuver I	Lanes (NWL), In	2
Weaving Configuration	One-Sided	Ramp-to-Freeway Lane	e Changes (LCRF), lc	1
Terrain Type	Level	Freeway-to-Ramp Lane	e Changes (LCFR), lc	1
Percent Grade, %	-	Ramp-to-Ramp Lane C	Changes (LCRR), lc	0
Interchange Density (ID), int/mi	0.66	Cross Weaving Manag	ed Lane	No
Adjustment Factors				
Driver Population	Mostly Familiar	Final Speed Adjustmer	nt Factor (SAF)	0.975
Weather Type	Non-Severe Weather	Final Capacity Adjustm	0.968	
Incident Type	No Incident	Demand Adjustment F	actor (DAF)	1.000
Demand and Capacity				
	FF	RF	RR	FR
Demand Volume (Vi), veh/h	6480	400	0	1100
Peak Hour Factor (PHF)	0.95	0.95	0.95	0.95
Total Trucks, %	3.00	3.00	3.00	3.00
Heavy Vehicle Adjustment Factor (fHV)	0.971	0.971	0.971	0.971
Flow Rate (vi), pc/h	7025	434	0	1192
Weaving Flow Rate (vw), pc/h	1626	Freeway Max Capacity	(cIFL), pc/h/ln	2400
Non-Weaving Flow Rate (vNW), pc/h	7025	Density-Based Capacity	y (cIWL), pc/h/ln	2154
Total Flow Rate (v), pc/h	8651	Demand Flow-Based C	apacity (cɪw), pc/h	12766
Volume Ratio (VR)	0.188	Weaving Segment Cap	acity (cw), veh/h	10458
Minimum Lane Change Rate (LCMIN), lc/h	1626	Adjusted Weaving Area	a Capacity, pc/h	10425
Maximum Weaving Length (LMAX), ft	4414	Volume-to-Capacity Ra	atio (v/c)	0.83
Speed and Density	<u>.</u>	<u></u>		
Non-Weaving Vehicle Index (INW)	556	Average Weaving Spee	ed (Sw), mi/h	54.3
Non-Weaving Lane Change Rate (LCNW), lc/h	1135	Average Non-Weaving	Speed (SNW), mi/h	53.5
Weaving Lane Change Rate (LCW), lc/h	2065	Average Speed (S), mi/	53.6	
Weaving Lane Change Rate (LCAII), Ic/h	3200	Density (D), pc/mi/ln	32.3	
Weaving Intensity Factor (W)	0.490	Level of Service (LOS)		D

Service Volume Ta	ble										
Target LOS		A		В	с	D		E			
Max Service Flow Rate (MS	SF), pc/h/ln	653		1190	1552	183	9	2085			
Service Flow Rate (SF), veh/h         3168         5778         7535         8928         10123											
Service Volume, veh/h         3010         5489         7158         8481         9617											
One Direction DSV, 1000 v	eh/day	30		55	72	85		96			
Bi-Directional DSV, 1000 v	eh/day	55		100	130	15	4	175			
Design Analysis Ta	ble	·						-			
Number of Lanes, In	3		4		5		6				
Density, pc/mi/ln	-		-		32.3		26.3				
LOS	F		F		D		С				
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HCS™ Freeways Version 7.8 2030Alt2\_I93NB\_Exits40-41\_Weaving\_PM.xuf Location 2 - I-93 Southbound at the End of the HOV Zipper Lane Freeway Merge, Diverge, and Weave Analyses

AnalystCRF>Date2/25/201 JAnalystAnalyst Vac2/25/201 JAuridictionMasDOT District 6Time Period Analyzed4.00 to 5.00 FMPreseQuine y - Exit 8 On-Ramp - Low Quine y - Exit 8 On-Ramp - Low Sement Severe Wettherecks Exit 8 On-Ramp - Low Severe Ramp Severe Wettherecks Exit 8 On-Ramp - Low Severe Wettherecks Exit 8 On-Ramp - Low Severe Ramp Severe Ramp Severe Wettherecks Exit 8 On-Ramp - Low Severe Ramp Severe Ramp Severe Wettherecks Exit 8 On-Ramp Ramp Severe Ramp Se								
AgencyCTPSAnalysis Year2019JurisdictionMassDOT District 6Time Period Analyzed4.00 to 5:00 PMProject DescriptionQuincy - Exit 8 On-Ramp - Low Cost Pereway Battenedics ExistionUnitUnited States Customary Customary ExistionGeometric DataFreeway Controlled States Customary ExistingFreeway State Customary ExistingBatter States Customary ExistingBatter States Customary ExistingBatter States Customary ExistingFreeway State Customary ExistingState States Customary ExistingState States Customary ExistingStates Customary ExistingStat	Project Information							
JurisdictionMassDOT District 6Time Period Analyzed4.00 to 5:00 PMProject DescriptionQuircy - Exit 8 On-Ramp - Low Cast Freeway Bottlenecks EasisingUnitUnited States CustomaryGeometric DataFreewayRampMumber of Lanes (N), InFreewayRampFree Flow Speed (FFS), mi/h55.035.0Segment Length (L) / Acceleration Length (LA), It55.035.0Segment Length (L) / Acceleration Length (LA), ItSpecific GradeSpecific GradeSegment Type / Ramp Side5.001.00Percent Grade, %1.001.00Segment Type / Ramp SideNon Severe WeatherNorter PopulationNon Severe WeatherNorter PopulationNon Severe WeatherNon-Severe WeatherIncident TypeNon	Analyst E	Ben Erban		Date	2/25/2019			
Project Description         Quincy - Exit 8 On-Ramp - Low Cost Freeway Bottlenecks Existing         Unit         United States Customary           Geometric Data         Free-Now         Ramp           Number of Lanes (N), In         Free-Now         Ramp           Number of Lanes (N), In         4         1           Free-Now Speed (FFS), mi/h         55.0         35.0           Segment Langh (L) / Acceleration         Specific Grade         Specific Grade           Percent Grade, %         Specific Grade         Specific Grade           Percent Grade, %         100         1.00           Segment Type / Ramp Side         Income Specific Grade         Specific Grade           Percent Grade, %         Non-Severe Weather         Non-Severe Weather           Driver Population         Non-Severe Weather         Non-Severe Weather           Incident Type         1.000         1.000           Frinal Capacity Adjustment Factor (DAF)         1.000         1.000           Demand Adjustment Factor (DAF)         1.000         1.000           Peak Hour Factor (PHF)         V         1.000         V           Single-Unit Twacks (SUT), %         700         0.0         V           Single-Unit Twacks (SUT), %         0.95         0.95         V	Agency C	CTPS		Analysis Year	2019			
Cost Freewy Bottlencks ExistingGeneration CostGeneration CostFreewayRampNumber of Lanes (N). InFreewayRampSpecific GradeSpecific GradeNon-Severe WeatherNon-Severe WeatherNon-Severe WeatherIncident TypeNon-Severe WeatherNon-Severe WeatherNon-Severe WeatherIncident TypeNon-Severe WeatherIncident TypeNon-Severe WeatherIncident TypeNon-Severe WeatherIncident TypeNon-Severe WeatherIncident TypeNon-Severe WeatherI	Jurisdiction N	MassDOT [	District 6	Time Period Analyzed	4:00 to 5:0	0 PM		
InductionFreewayRampNumber of Lanes (N), In41Free-Flow Speed (FFS), mi/n55.035.0Segment Length (L) / Acceleration Length (LA), ft150092Ferrain TypeSpecific GradeSpecific GradeSegment Type / Ramp Side1.001.00Segment Type / Ramp Side1.001.00Segment Type / Ramp SideNorsever WeatherNorsever WeatherAdjustment FactorsNon-Severe WeatherNorsever WeatherDriver PopulationNon-Severe Weather1.000Weather TypeNon-Severe Weather1.000Incident Type1.0001.00Final Speed Adjustment Factor (CAF)1.0001.000Final Speed Adjustment Factor (CAF)1.0001.000Emand Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (CAF)1.0000.00Demand Adjustment Factor (PMF)0.950.95Demand Adjustment Factor (PMF)1.0000.00Total Tracks (SUT), %700.00Tractor-Trailers (TI), %300Heavy Vehicle Adjustment Factor (HW)9.9660.37Foreative (N), pc/h7.727.72Gapacity (C), pc/h9.950.37Tractor-Trailers (TI), %9.060.37Up tractor (PMF)0.9660.37Gapacity (C), pc/h9.330.37Foreative (N), pc				Unit	United Sta	tes Customary		
Number of Lanes (N), In     4     1       Free-Flow Speed (FFS), mi/h     55.0     35.0       Segment Length (L) / Acceleration Length (LA),ft     1500     920       Terrain Type     Specific Grade     Specific Grade       Percent Grade, %     10.0     1.00       Segment Length (L) / Acceleration Length (LA),ft     100     100       Segment Type / Ramp Side     Freeway     Right       Segment Type / Ramp Side     Freeway     Right       Adjustment Factors     Non-Severe Weather     Non-Severe Weather       Driver Population     Mostly Familiar     Mostly Familiar       Weather Type     Non Severe Weather     Non-Severe Weather       Incident Type     Non Severe Weather     Non-Severe Weather       Incident Type     Non Severe Weather     1.000       Final Speed Adjustment Factor (CAF)     1.000     1.000       Demand Adjustment Factor (DAF)     1.000     1.000       Demand Adjustment Factor (PAF)     700     0.00       Peak Hour Factor (PHF)     0.95     0.95       Total Trucks %     0.96     100       Single-Unit Tuck (SUT), %     70     0       Tractor Tailers (T), %     0.966     737       Capacity (c), pc/h     0.966     737       Sogment Lactor (HMI)     0	Geometric Data							
Free-Flow Speed (FFS), mi/h     55.0     35.0       Segment Length (L) / Acceleration Length (LA),It     1500     920       Terrain Type     Specific Grade     Specific Grade       Percent Grade, %     1.00     1.00       Segment Type / Ram Side     Terway Rew     No       Segment Type / Ram Side     No     No       Adjustment Factors     Mostly Familiar     Mostly Familiar       Weather Type     Non-Severe Weather     Non-Severe Weather       Non-Severe Meather     Non-Severe Weather     Non-Severe Weather       Incident Type     Non-Severe Weather     Non-Severe Weather       Demand Adjustment Factor (SAF)     Non-Severe Weather     Non-Severe Weather       Percenat Oloume (V)     700     No				Freeway	Ramp			
Segment Length (L) / Acceleration Length (LA),ft     1500     920       Terrain Type     Specific Grade     Specific Grade       Percent Grade, %     1.00     1.00       Segment Type / Ramp Side     Freeway     Right       Adjustment Factors     Freeway     Right       Driver Population     Mostly Familiar     Mostly Familiar       Weather Type     Non-Severe Weather     Non-Severe Weather       Incident Type     Non-Severe Weather     1.000       Final Speed Adjustment Factor (SAF)     No Incident     1.000       Incident Type     1.000     1.000       Final Capacity Adjustment Factor (CAF)     1.000     1.000       Demand Adjustment Factor (SAF)     1.000     1.000       Demand Adjustment Factor (DAF)     1.000     1.000       Demand Adjustment Factor (DAF)     1.000     0.00       Peak Hour Factor (PHF)     0.95     0.00       Single-Unit Tucks (SUT),%     70     0.00       Total Trucks, %     2.00     0.00       Single-Unit Tucks (SUT),%     50     0.00       Factor Trailer (T1),%     9000     0.00       Heavy Vehicle Adjustment Factor (Hv)     0.966     0.01       For Rate (vi),pc/h     -     628     0.37       Capacity (c), pc/h     9000	Number of Lanes (N), In			4	1			
Terrain Type       Specific Grade       Specific Grade         Percent Grade, %       1.00       1.00         Segment Type / Ramp Side       Freeway       Right         Adjustment Factors        Mostly Familiar       Mostly Familiar         Driver Population       Mostly Familiar       Mostly Familiar       Mostly Familiar         Weather Type       Non - Severe Weather       Non-Severe Weather       Non-Severe Weather         Incident Type       No Incident       1.000       1.000         Final Capacity Adjustment Factor (SAF)       1.000       1.000         Demand Adjustment Factor (CAF)       1.000       1.000         Demand Adjustment Factor (DAF)       1.000       1.000         Demand Adjustment Factor (PHF)       0.95       0.95         Total Trucks, %       2.00       0.00         Single-Unit Trucks (SUT), %       70       100         Tractor-Trailers (TI), %       9.966       1.000         Heavy Vehice Adjustment Factor (Hrv)       0.966       0.37         Flow Rate (vi), pc/h       7628       737         Capacity (c), pc/h       9.093       0.37         Speed Index (MS)       0.37       289         Distance to Upstream Ramp (Luo), ft       9.0	Free-Flow Speed (FFS), mi/h			55.0	35.0			
Percent Grade, %1.001.00Segment Type / Ramp SideFreewayRightAdjustment FactorsDriver PopulationMostly FamiliarMostly FamiliarNon-Severe WeatherNon-Severe WeatherIndicating Colspan="2">Indicating Colspan="2">Indicating Colspan="2">Mostly FamiliarMostly FamiliarMostly FamiliarMostly FamiliarMostly FamiliarDemand Volume (Va) <th colsp<="" td=""><td>Segment Length (L) / Acceleration Le</td><td>ength (LA),</td><td>ft</td><td>1500</td><td>920</td><td></td></th>	<td>Segment Length (L) / Acceleration Le</td> <td>ength (LA),</td> <td>ft</td> <td>1500</td> <td>920</td> <td></td>	Segment Length (L) / Acceleration Le	ength (LA),	ft	1500	920		
Segment Type / Ramp Side       Freeway       Right         Adjustment Factors       Mostly Familiar       Mostly Familiar         Driver Population       Mostly Familiar       Non-Severe Weather         Weather Type       Non Sovere Weather       Non-Severe Weather         Incident Type       No Incident       -         Final Speed Adjustment Factor (SAF)       1000       1.000         Final Capacity Adjustment Factor (CAF)       1000       1.000         Demand Adjustment Factor (DAF)       1000       1.000         Demand Adjustment Factor (CAF)       0.000       1.000         Demand Adjustment Factor (DAF)       0.000       0.00         Peak Hour Factor (PHF)       0.95       0.95         Total Tracks, %       2.00       0.00         Single-Unit Trucks (SUT), %       70       100         Tractor-Trailers (TT), %       30       0         Heavy Vehicle Adjustment Factor (Hrlv)       9.966       1.000         Flow Rate (w), pc/h       0.966       1.000         Gapacity (c), pc/h       0.93       0.37         Volume-to-Capacity Ratio (v/c)       9.93       0.37         Obstrace to Upstream Raum (LuP), ft       -       1.600       -         Distance to Up	Terrain Type			Specific Grade	Specific Gr	ade		
Adjustment Factors       Mostly Familiar       Mostly Familiar         Driver Population       Mostly Familiar       Mostly Familiar         Weather Type       Non-Severe Weather       Non-Severe Weather         Incident Type       No Incident       -         Final Speed Adjustment Factor (SAF)       1.000       1.000         Final Capacity Adjustment Factor (CAF)       1.000       1.000         Demand Adjustment Factor (DAF)       1.000       1.000         Demand Adjustment Factor (DAF)       1.000       1.000         Demand Adjustment Factor (DAF)       0.95       0.95         Demand Volume (V)       700       70         Peak Hour Factor (PHF)       0.95       0.95         Total Trucks, %       2.00       0.00         Single-Unit Trucks (SUT), %       70       100         Tractor-Trailers (TT), %       30       0         Heavy Vehicle Adjustment Factor (Hrv)       0.966       1.000         Flow Rate (w), pc/h       7628       73         Capacity (c), pc/h       0.93       0.37         Speed and Density       0.429       0.429         Downstream Equilibrium Distance (LEQ), ft       -       Number of Outer Lanes on Freeway (No)       2         Distan	Percent Grade, %			1.00	1.00			
Driver PopulationMostly FamiliarMostly FamiliarWeather TypeNon-Severe WeatherNon-Severe WeatherIncident TypeNo Incident-Final Speed Adjustment Factor (SAF)1.0001.000Final Capacity Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adjustment Factor (DAF)0.950.95Demand Volume (Vi)700700Peak Hour Factor (PHF)0.950.95Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (fitrv)0.9661.000Flow Rate (w), pc/h7628737Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Downstream Ramp (LUP), ft-Speed Index (Ms)0.429Downstream Ramp (LUD), ft-On-Ramp Influence Area Speed (SN, mi/h)49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO, mi/h)48.6Flow in Lanes 1 and 2 (vi2), pc/h3051Ramp Junction Speed (SD, mi/h)40.7Flow Entering Ramp-Infl. Area (wR12), pc/h3788Average Density (D), pc/mi/ln42.7	Segment Type / Ramp Side			Freeway	Right			
Weather Type         Non-Severe Weather         Non-Severe Weather           Incident Type         No Incident         -           Final Speed Adjustment Factor (SAF)         1.000         1.000           Final Capacity Adjustment Factor (DAF)         1.000         1.000           Demand Adjustment Factor (DAF)         1.000         1.000           Demand Adjustment Factor (DAF)         1.000         1.000           Demand Adjustment Factor (PAF)         0.00         700           Peak Hour Factor (PHF)         0.95         0.95           Total Trucks, %         2.00         0.00           Single-Unit Trucks (SUT), %         70         100           Tractor-Trailers (TT), %         30         0           Heavy Vehicle Adjustment Factor (Hv)         0.966         1.000           Flow Rate (vi), pc/h         7628         737           Capacity (c), pc/h         9000         2000           Volume-to-Capacity Ratio (v/c)         0.93         0.37           Speed and Density         0.93         0.37           Upstream Equilibrium Distance (LEQ), ft         -         Number of Outer Lanes on Freeway (NO)         2           Distance to Upstream Ramp (LDWN), ft         -         On-Ramp Influence Area Speed (SR), mi/H <t< td=""><td>Adjustment Factors</td><td></td><td></td><td></td><td></td><td></td></t<>	Adjustment Factors							
Incident TypeNo Incident-Final Speed Adjustment Factor (SAF)1.0001.000Final Capacity Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adjustment Factor (DAF)1.000Demand Ad CapacityDemand and CapacityDemand and Capacity7000700Peak Hour Factor (PHF)0.950.95Demand Volume (Vi)Peak Hour Factor (PHF)0.950.95Total Trucks, SUT), %2.000.00Total Trucks (SUT), %70100Tractor-Trailers (TT), %0.9661.000Heavy Vehicle Adjustment Factor (HvV)0.9661.000No 0002.000Foreward (c), pc/h0.930.37Capacity (c), pc/h9.030.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freewary (NO)2Distance to Upstream Ramp (LDOWN), ft-0n-Ramp Influence Area Speed (SD, mi/h)49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SD, mi/h)49.0Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SD, mi/h)49.0Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SD, mi/h)49.0<	Driver Population			Mostly Familiar	Mostly Fan	niliar		
Final Speed Adjustment Factor (SAF)1.0001.000Final Capacity Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adjustment Factor (DAF)1.000700Demand Ad CapacityDemand Adjustment Factor (DAF)7000700Demand Volume (Vi)700700Peak Hour Factor (PHF)0.950.95Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2"Demand Adjustment Factor (PHF)0.950.95Colspan="2">Colspan="2"<	Weather Type			Non-Severe Weather	Non-Sever	e Weather		
Final Capacity Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adjustment Factor (DAF)1.000700Demand Nolume (Vi)7000700Peak Hour Factor (PHF)0.950.95Colspan="2">On 0.00Single-Unit Trucks (SUT), %2.000.00Total Trucks (SUT), %70100Tractor-Trailers (TI), %0.9661.000Heavy Vehicle Adjustment Factor (Hw)0.9661.000Foega and DensityOpea and DensityOpea and DensityOpea and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LuP), ft-Speed Index (Ms)0.429Downstream Ramp (LuP), ft-On-Ramp Influence Area Speed (SA), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Incident Type			No Incident	-			
non-space in the second secon	Final Speed Adjustment Factor (SAF)			1.000	1.000			
Demand and Capacity7000700Peak Hour Factor (PHF)70000.950.950.95Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (fHV)0.9661.000Flow Rate (vi), pc/h7628737Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (voA), pc/h/In2289Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (wR12), pc/h3788Average Density (D), pc/mi/In42.7	Final Capacity Adjustment Factor (CA	NF)		1.000	1.000			
Demand Volume (V)700700Peak Hour Factor (PHF)0.950.95Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (Hv)0.9661.000Flow Rate (v), pc/h7628737Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freewy (No)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SD, mi/h48.6Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SD, mi/h49.0Flow In Lanes 1 and 2 (v12), pc/h3788Average Density (D), pc/mi/ln42.7	Demand Adjustment Factor (DAF)			1.000	1.000			
Peak Hour Factor (PHF)0.950.95Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (fHV)0.9661.000Flow Rate (v), pc/h7628737Capacity (c), pc/h0.9002000Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Istance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (So), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (So), mi/h49.0Flow In Lanes 1 and 2 (v12), pc/h3788Average Density (D), pc/mi/ln42.7	Demand and Capacity							
Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (fHV)0.9661.000Flow Rate (vi), pc/h7628737Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/I289Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SN, mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SD, mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3788Average Density (D), pc/mi/I42.7	Demand Volume (Vi)			7000	700			
Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (fHv)0.9661.000Flow Rate (vi), pc/h7628737Capacity (c), pc/h90000.37Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SO, mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO, mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (wR12), pc/h3788Average Density (D), pc/mi/ln42.7	Peak Hour Factor (PHF)			0.95	0.95			
Jack Product of the function	Total Trucks, %			2.00	0.00			
Heavy Vehicle Adjustment Factor (fHV)0.9661.000Flow Rate (vi),pc/h7628737Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln289Distance to Upstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SD), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Single-Unit Trucks (SUT), %			70	100			
Flow Rate (vi),pc/h7628737Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2289Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Tractor-Trailers (TT), %			30	0			
Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2289Downstream Equilibrium Distance (LEQ), ft-On-Ramp Influence Area Speed (SK), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Heavy Vehicle Adjustment Factor (fH	V)		0.966	1.000			
Volume-to-Capacity Ratio (v/c)0.930.37Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln289Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Flow Rate (vi),pc/h			7628	737			
Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2289Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Capacity (c), pc/h			9000	2000			
Upstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2289Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Volume-to-Capacity Ratio (v/c)			0.93	0.37			
Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.429Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2289Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Speed and Density							
Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2289Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Upstream Equilibrium Distance (LEQ),	, ft	-	Number of Outer Lanes on Freew	ay (NO)	2		
Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Distance to Upstream Ramp (LUP), ft -			Speed Index (MS)		0.429		
Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.126Outer Lanes Freeway Speed (SO), mi/h48.6Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Downstream Equilibrium Distance (Le	EQ), ft	-	Flow Outer Lanes (vOA), pc/h/ln		2289		
Flow in Lanes 1 and 2 (v12), pc/h3051Ramp Junction Speed (S), mi/h49.0Flow Entering Ramp-Infl. Area (vR12), pc/h3788Average Density (D), pc/mi/ln42.7	Distance to Downstream Ramp (LDOV	WN), ft	-	On-Ramp Influence Area Speed (S	SR), mi/h	49.4		
Flow Entering Ramp-Infl. Area (vR12), pc/h     3788     Average Density (D), pc/mi/ln     42.7	Prop. Freeway Vehicles in Lane 1 and	I 2 (Рғм)	0.126	Outer Lanes Freeway Speed (SO),	mi/h	48.6		
	Flow in Lanes 1 and 2 (v12), pc/h		3051	Ramp Junction Speed (S), mi/h49.0				
Level of Service (LOS)     D     Density in Ramp Influence Area (DR), pc/mi/ln     29.0	Flow Entering Ramp-Infl. Area (vR12),	pc/h	3788	Average Density (D), pc/mi/ln 42.7				
	Level of Service (LOS)		D	Density in Ramp Influence Area (DR), pc/mi/ln 29.0				

Service Volume Tab	le										
Target LOS		A			В		с	Τ	D		E
Freeway											
Max Service Flow Rate (MSI	-), pc/h/ln	562		1	319		1841		204	5	-
Service Flow Rate (SF), veh/	h	2170		5	5095		7114		790	4	-
Service Volume, veh/h		2062		2	1840		6759		750	8	-
One Direction DSV, 1000 ve	eh/day	21			48		68		75		-
Bi-Directional DSV, 1000 ve	h/day	37			88		123		137	7	-
Ramp											
Max Service Flow Rate (MSI	-), pc/h/ln	217			510		711		790	)	-
Service Flow Rate (SF), veh/	h	217			510		711		790	)	-
Service Volume, veh/h		206			484		676		75 <sup>-</sup>	1	-
One Direction DSV, 1000 ve	eh/day	2			5		7		8		-
Design Analysis Tak	ole			-							
Freeway Lanes, In	2	2	3		3	4	1	4		5	5
Ramp Lanes, In	1	2	1		2	1	I	2		1	2
Density, pc/mi/ln	-	-	-		-	4	12.7	42.5		32.9	32.7
LOS	F	F	F		F	C	)	С		С	В

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Analyst     Ben Erban     Date     2/25/2019       Ageny     CTPS     Analysis Yaar     2019       Juridsdiction     MassDO District 6     Time Period Analyzed     400 to 5.00 PM       Project Description     Quincy - Erit 6 On-Ampp - Low     Unit     United States Customary       Geometric Data     Unity - Erit 6 On-Ampp - Low     Unit     United States Customary       Geometric Data     Freeway Bottlenecks Alternative 1     Freeway Bottlenecks Alternative 1     Freeway Bottlenecks Alternative 1       Segment Length (L) / Acceleration Length (LA).     4     1     Freeway Bottlenecks Alternative 1       Segment Length (L) / Acceleration Length (LA).     1500     900       Terrain Type     Specific Grade     Specific Grade       Segment Type / Ramp Side     Specific Grade     Specific Grade       Percent Grade, %     1.00     1.00     Internative 1       Adjustment Factor SAP     Mostly Familiar     Mostly Familiar       Adjustment Factor (SAF)     No Incident     Non Severe Weather       Non Severe Weather     Non Severe Weather     Non Severe Weather       Incident Type     No Incident     1.00     Internative 1       Fread Adjustment Factor (SAF)     No Incident     1.00     Internative 1       Fread Adjustment Factor (SAF)     No Incident     Internative 1<	Project Information							
AgencyCTPSAnalysis Year2019JurisdictionMassDOT District 6Time Period Analyzed4:00 to 5:00 PMProject DescriptionQuine y Enit 8 On-Ramp Low Alternative 1UnitUnited States CustomaryGeometric DataFreewayRampRemative 1FreewayRampNumber of Lanes (N), In41Free-Row Speed (FS), mi/n55.035.0Segment Length (L) / Acceleration Longth (LX)55.035.0Segment Length (L) / Acceleration Longth (LX)Specific GradeSpecific GradePercent Grade, %1.001.00Free-Row Speed (FS), mi/nSpecific GradeSpecific GradePercent Grade, %1.001.00Segment Lipp / Lang SideNon-Severe WeatherNon-Severe WeatherNamer PaulationNon-Severe WeatherNon-Severe WeatherNon-Severe Weather TypeNon-Severe WeatherNon-Severe WeatherIncident TypeNon-Severe WeatherNon-Severe Weather	-	on Erban	Data	2/25/2010				
Initial citicition         MassDOT District 6         Time Period Analyzed         400 to 5:00 PM           Project Description         Quinty -: Exit 8 On-Ramp - Law Atternative 1         Unit         Unitéd States Customary Atternative 1           Geometric Data         Freeway         Ramp         Image: Control Cont	, ,							
Project Description     Quincy - Erit & On-Ramp - Low Atternative 1     Unit     Unit     Unit       Geometric Data     Freeway Bottlenecks Atternative 1     Ramp     Ramp       Number of Lanes (N), In     Freeway Bottlenecks Atternative 1     Ramp     Ramp       Segment Length (L) / Acceleration Length (LA), It     55.0     35.0     Segment Type       Segment Length (L) / Acceleration Length (LA), It     1500     900     Total Segment Type       Percent Grade, %     Ion     1.00     1.00       Segment Type / Ramp Side     Freeway Breiter     Nostly Familiar       Adjustment Factor Segment Type / Ramp Side     Mostly Familiar     Mostly Familiar       Mostly Familiar     Non Swere Weather     Non Swere Weather       Inicident Type     1.000     1.000       Final Speed Adjustment Factor (CAF)     1.000     1.000       Emand Adjustment Factor (CAF)     1.000     1.000       Demand Adjustment Factor (PMF)     1.000     1.000					0 D1 /			
Cost Freeway Bottlenecks Alternative 1         Ramp           Geometric Data         Ramp           Geometric Data         Ramp           Number of Lanes (N). In         Free-Way         Ramp           Speed (FS), mi/h         Specific Grade         Specific Grade           Segment Length (L) / Acceleration Length (LA)/t         IS00         900           Ternain Type         Specific Grade         Specific Grade           Segment Tope / Ramp Side         Veroway         Right         Veroway           Segment Type / Ramp Side         Veroway         Right         Veroway           Order Opulation         Mostly Familiar         Mostly Familiar           Owner Speed Adjustment Factor (SAF)         Non Incident         Incident         Non-Severe Weather           Final Speed Adjustment Factor (CAF)         Non Incident         Incident         Incident           Demand Adjustment Factor (CAF)         Incident         Incident         Incident           Demand Volume (V)         Incident Factor (CAF)         Incident Factor (PAF)         Incident Factor (PAF)         Incident Factor (PAF)           Terractor Trailer S(TD, %         Incident Factor (PAF)         Incident Factor (								
FreewayRampNumber of Lanes (N), In41Free-Flow Speed (FFS), ni/h55.035.0Segment Length (L) / Acceleration Length (LA), It150090Terrain TypeSpecific GradeSpecific GradePercent Grade, %1.001.00Segment Type / Ramp SidereewayRight -Percent Grade, %1.001.00Segment Type / Ramp SidereewayRight -Adjustment FactorsMostly FamiliarMostly FamiliarDriver PopulationMostly FamiliarMostly FamiliarWeather TypeNo Incidento-Incident TypeNo IncidentFinal Speed Adjustment Factor (CAF)No Incident-Final Speed Adjustment Factor (CAF)1.0001.00Final Capacity Adjustment Factor (CAF)1.0001.00Demand Adjustment Factor (CAF)1.0001.00Demand Adjustment Factor (CAF)1.0001.00Demand Adjustment Factor (CAF)1.0001.00Demand Adjustment Factor (CAF)1.0000.00Total Tucks (SUT), %70100Tractor-Trailers (TI), %300Gaped-Hack (N)pc/h0.9661.000Flow Yehicle Adjustment Factor (Fivr)0.966Flow Yehicle Adjustment Factor (Fivr)0.966Tractor-Trailers (TI), %0.9661.000Flow Yehicle Adjustment Factor (Fivr)0.966Flow Yehicle Adjustment Factor (Fivr)0.966Flow Yehicle Adjustment Factor (Fivr)<	C	ost Freeway Bottlenecks	Unit	United Sta	tes Customary			
Number of Lanes (N), In         4         1           Free-Flow Speed (FFS), mi/h         55.0         35.0           Segment Length (L) / Acceleration Length (LA),ft         1500         900           Terrain Type         Specific Grade         Specific Grade           Percent Grade, %         Non-Severe Weather         Right	Geometric Data							
Free-Flow Speed (FFS), mi/n     55.0     35.0       Segment Length (L) / Acceleration Length (LA), ft     1500     900       Terrain Type     Specific Grade     Specific Grade       Percent Grade, %     1.00     1.00       Segment Length (L) / Acceleration Length (LA), ft     Specific Grade       Segment Grade, %     1.00     1.00       Segment Type / Ramp Side     Freeway     No       Segment Type / Ramp Side     Mostly Familiar     Mostly Familiar       Owner Specific Grade     Non-Severe Weather     Non-Severe Weather       Diver Specific Magnetic Factor (SAF)     No Incident     -       Final Speed Adjustment Factor (SAF)     1.000     1.000       Final Speed Adjustment Factor (CAF)     1.000     1.00       Demand Adjustment Factor (CAF)     1.000     1.00       Demand Adjustment Factor (DAF)     1.000     1.00       Demand Adjustment Factor (DAF)     1.00     1.00       Demand Adjustment Factor (PMF)     1.00     1.00       Total Trucks, %     0     0     -       Single-Unit Trucks (SUT), %     70     0.0       Tractor Trailers (TI), %     0     0.0       Flow Rate (w), p.c/h     704     0.0       Volume to-Capacity Ratio (v/c)     764     0.0       Obertare Equili			Freeway	Ramp				
Segment Length (L) / Acceleration Length (LA),ft1500900Terrain TypeSpecific GradeSpecific GradePercent Grade, %1.001.00Segment Type / Ramp SideFreewayRightAdjustment FactorsUriver PopulationMostly FamillarMostly FamillarNon-Severe WeatherNon-Severe WeatherNon-Severe WeatherNon-Severe WeatherNon-Severe WeatherIncident TypeNon Non Severe WeatherNon-Severe WeatherIncident Type1.0001.000Final Speed Adjustment Factor (SAF)1.0001.000Final Capacity Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (SAF)1.0001.000Demand Adjustment Factor (SAF)Demand Volume (Vi)7500700Peak Inder Grade Signer	Number of Lanes (N), In		4	1				
Terrain Type       Specific Grade       Specific Grade         Percent Grade, %       1.00       1.00         Segment Type / Ramp Side       Freeway       Right         Adjustment Factors       Freeway       Mostly Familiar       Mostly Familiar         Muster Population       Mostly Familiar       Mostly Familiar       Mostly Familiar         Weather Type       Non - Severe Weather       Non-Severe Weather       Non-Severe Weather         Incident Type       Non Incident       -       -         Final Speed Adjustment Factor (SAF)       1.000       1.000         Final Gapacity Adjustment Factor (CAF)       1.000       1.000         Demand Adjustment Factor (DAF)       1.000       1.000         Demand Adjustment Factor (DAF)       1.000       1.000         Demand Volume (V)       7500       700         Peak Hour Factor (PHF)       1.00       1.00         Total Trucks, %       30       0         Single-Unit Trucks (SUT), %       70       100         Total Trucks (Vapt/n       0.966       1.000         Flow Rate (vapt/n)       0.94       0.35         Speed and Density       9000       200         Volume-to-Capacity Ratio (v/c)       0       1.043	Free-Flow Speed (FFS), mi/h		55.0	35.0				
Percent Grade, %         1.00         1.00           Segment Type / Ramp Side         Freeway         Right           Adjustment Factors         Mostly Familiar         Mostly Familiar           Driver Population         Mostly Familiar         Mostly Familiar           Weather Type         Non-Severe Weather         Non-Severe Weather           Incident Type         No Incident         -           Final Speed Adjustment Factor (SAF)         No Incident         1.000           Erinal Capacity Adjustment Factor (CAF)         1.000         1.000           Demand Sector (PHF)         1.000         1.000           Total Trucks, %         2.00         0.00           Single-Unit Trucks (SUT), %         70         1.00           Tractor-Trailers (TT, %         0.966         0.000           Heavy Vehicle Adjustment Factor (H+V)         0.966         0.000           Speed Ad Densit	Segment Length (L) / Acceleration Le	ngth (LA),ft	1500	900				
Segment Type / Ramp Side       Preeway       Right         Adjustment Factors       Mostly Familiar       Mostly Familiar         Driver Population       Mostly Familiar       Mostly Familiar         Weather Type       Non-Severe Weather       Non-Severe Weather         Incident Type       Non-Severe Weather       Non-Severe Weather         Demand Adjustment Factor (CAF)       1.000       1.00         Demand Adjustment Factor (DAF)       1.00       1.00         Total Trucks, %       2.00       0.00         Single-Unit Trucks (SUT), %       70       1.00         Tractor-Trailers (TT), %       0.966       1.00         Heavy Vehicle Adjustme	Terrain Type		Specific Grade	Specific Gr	ade			
Adjustment Factors         Mostly Familiar       Mostly Familiar         Mostly Familiar         Mostly Familiar         Mostly Familiar         Mostly Familiar         Mostly Familiar         Mostly Familiar         Mostly Familiar         Mostly Familiar         Mostly Familiar         Non-Severe Weather         Non-Severe Weather         Incident         Incident         Incident         Incident         Familiar         No Incident         Incident         Incident         Motion         Incident         Motion         No Incident         Incident         Incident         Incident         Motion         Demand Adjustment Factor (CAF)         Toto         Toto         Tota          Toto	Percent Grade, %		1.00	1.00				
Driver PopulationMostly FamiliarMostly FamiliarWeather TypeNon-Severe WeatherNon-Severe WeatherNon-Severe WeatherIncident TypeNo Incident-Final Speed Adjustment Factor (SAF)1.0001.000Final Capacity Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand M Capacity1.0001.000Demand Volume (Vi)750070Peak Hour Factor (PHF)1.001.00Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (Hiv)0.9661.000Flow Rate (vi), pc/h7764700Volume-to-Capacity Ratio (v/c)90002000Volume-to-Capacity Ratio (v/c)0.940.35Speed and Density90000.433Upstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LuD), ft-Speed Index (Ms)0.433Downstream Raum (LuD), ft-Prow. Parena Speed (SN, mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (vi2), pc/h3106Ramp Junction Speed (SN, mi/h48.8Flow Untariens 1 and 2 (vi2), pc/h3806Average Density (D), pc/mi/In43.4	Segment Type / Ramp Side		Freeway	Right				
Weather Type         Non-Severe Weather         Non-Severe Weather           Incident Type         No Incident         -           Final Speed Adjustment Factor (SAF)         1.000         1.000           Final Capacity Adjustment Factor (CAF)         1.000         1.000           Demand Adjustment Factor (DAF)         1.000         1.000           Demand Adjustment Factor (DAF)         1.000         1.000           Demand Adjustment Factor (PAF)         1.000         1.000           Peak Hour Factor (PHF)         1.00         1.00           Total Trucks, %         2.00         0.00           Single-Unit Trucks (SUT), %         70         100           Tractor-Trailers (TT), %         30         0           Heavy Vehicle Adjustment Factor (HV)         0.966         1.000           Flow Rate (vi),pc/h         7764         700           Capacity (c), pc/h         9000         2000           Volume-to-Capacity Ratio (v/c)         0.94         0.35           Speed and Density         9000         200           Upstream Equilibrium Distance (LEQ), ft         -         Number of Outer Lanes on Freeway (NO)         2           Distance to Upstream Ramp (LDO,h), ft         -         Speed Index (MS)         0.433 <td>Adjustment Factors</td> <td></td> <td></td> <td></td> <td></td>	Adjustment Factors							
Incident TypeNo Incident-Final Speed Adjustment Factor (SAF)1.0001.000Final Capacity Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adjustment Factor (DAF)1.000Demand Adjustment Factor (DAF)1.000Demand CapacityDemand CapacityDemand CapacityDemand Volume (Vi)7500700Peak Hour Factor (PHF)1.001.00Total Trucks, %Single-Unit Trucks (SUT), %Total Trucks (SUT), %To 00.000Single-Unit Trucks (SUT), %Tractor-Trailers (TT), %No 0.06Heavy Vehicle Adjustment Factor (Hv)O 0.06Single-Unit Trucks (SUT), %Tode 0.00Tractor-Trailers (TT), %300Capacity (c), pc/hO 0.06Capacity (c), pc/hO	Driver Population		Mostly Familiar	Mostly Far	niliar			
Final Speed Adjustment Factor (SAF)1.0001.000Final Capacity Adjustment Factor (CAF)1.0001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adigustment Factor (DAF)1.000Demand Ad CapacityDemand Ad CapacityDemand Ad CapacityDemand Volume (Vi)7500700Peak Hour Factor (PHF)1.001.00Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (HV)0.9661.000Sologica (Licy), c/h0.9661.000Speed and DensityVelocime to Capacity (c), c/h0.940.35Speed Index (Ms)0.433Downstream Rayn (Lucy), ft-Number of Outer Lanes on Freewy (No)2Distance to Upstream Ramp (Lucy), ft-Speed Index (Ms)0.433Downstream Rayn (Lucy), ft-On-Ramp Influence Area Speed (S), mi/h48.3Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (S), mi/h48.8Freeway Vehicles in Lane 1 and 2 (PFM)3806Average Density (D), pc/mi/n43.4	Weather Type		Non-Severe Weather	Non-Sever	re Weather			
Final Capacity Adjustment Factor (CAF)10001.000Demand Adjustment Factor (DAF)1.0001.000Demand Adjustment Factor (DAF)1.000Demand Adjustment Factor (DAF)1.000Openand Adjustment Factor (DAF)7500700Peak Hour Factor (PHF)1.001.00Total Trucks, %Single-Unit Trucks (SUT), %700.00Total Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (HtV)0.9661.000Flow Rate (v),pc/h7060.00Capacity (c), pc/h0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Mer of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.8Flow in Lanes 1 and 2 (v12), pc/h3806Average Density (D), pc/mi/In43.4	Incident Type		No Incident	No Incident -				
non-digitation (DAF)1.0001.000Demand Adjustment Factor (DAF)1.000700Demand Volume (Vi)7500700Peak Hour Factor (PHF)1.001.00Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %0.9661.000Heavy Vehicle Adjustment Factor (fHv)0.9661.000Flow Rate (vi), pc/h70020002000Capacity (c), pc/h90002000Colspan="2">2000Volume-to-Capacity Ratio (v/c)0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-0.433Downstream Ramp (LUP), ft-2329Distance to Upstream Ramp (LUP), ft-2329Distance to Downstream Ramp (LDOWN), ft-0.040.433Downstream Equilibrium Distance (LEQ), ft-6.433Downstream Ramp (LDOWN), ft-2329Distance to Downstream Ramp (LDOWN), ft <th cols<="" td=""><td>Final Speed Adjustment Factor (SAF)</td><td></td><td>1.000</td><td colspan="3">1.000 1.000</td></th>	<td>Final Speed Adjustment Factor (SAF)</td> <td></td> <td>1.000</td> <td colspan="3">1.000 1.000</td>	Final Speed Adjustment Factor (SAF)		1.000	1.000 1.000			
Demand and Capacity7500700Peak Hour Factor (PHF)75007001.001.00Total Trucks, %Single-Unit Trucks (SUT), %700.00Tractor-Trailers (TT), %70100Tractor-Trailers (TT), %0.9661.000Flow Rate (vi), pc/h0.9661.000Flow Rate (vi), pc/h7764700Capacity (c), pc/h0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (voA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow Intares 1 and 2 (v12), pc/h3806Average Density (D), pc/mi/ln43.4	Final Capacity Adjustment Factor (CA	F)	1.000	1.000 1.000				
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Peak Hour Factor (PHF)1.001.00Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (fHV)0.9661.000Flow Rate (vi), pc/h7764700Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (voA), pc/h/In2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (St), mi/h48.3Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (Sc), mi/h48.3Flow in Lanes 1 and 2 (vt2), pc/h3806Average Density (D), pc/mi/In43.4	Demand and Capacity							
Total Trucks, %2.000.00Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (Hv)0.9661.000Flow Rate (vi), pc/h7764700Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO, mi/h48.8Flow In Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.4Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Demand Volume (Vi)		7500	700	700			
Single-Unit Trucks (SUT), %70100Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (fHV)0.9661.000Flow Rate (vi), pc/h7764700Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SD), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3806Average Density (D), pc/mi/ln43.4	Peak Hour Factor (PHF)		1.00	1.00	1.00			
Tractor-Trailers (TT), %300Tractor-Trailers (TT), %300Heavy Vehicle Adjustment Factor (fHV)0.9661.000Flow Rate (vi),pc/h7764700Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.940.35Speed and Density0.433Upstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LUP), ft-0.433Downstream Equilibrium Distance (LEQ), ft-000 Cuter Lanes (vOA), pc/h/In2329Distance to Downstream Ramp (LDOWN), ft-0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (V12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/In43.4	Total Trucks, %		2.00	0.00				
Heavy Vehicle Adjustment Factor (fHV)0.9661.000Flow Rate (vi),pc/h7764700Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (So), mi/h48.3Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (So), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3806Average Density (D), pc/mi/ln43.4	Single-Unit Trucks (SUT), %		70					
Flow Rate (vi),pc/h7764700Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (Ms)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Tractor-Trailers (TT), %		30	0				
Capacity (c), pc/h90002000Volume-to-Capacity Ratio (v/c)0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freews/2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)2Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Upstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Heavy Vehicle Adjustment Factor (fHV	)	0.966	1.000				
Volume-to-Capacity Ratio (v/c)0.940.35Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SO), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Flow Rate (vi),pc/h		7764	700				
Speed and DensityUpstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (NO)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Capacity (c), pc/h		9000	2000				
Upstream Equilibrium Distance (LEQ), ft-Number of Outer Lanes on Freeway (No)2Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Volume-to-Capacity Ratio (v/c)		0.94	0.35				
Distance to Upstream Ramp (LUP), ft-Speed Index (MS)0.433Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Speed and Density							
Downstream Equilibrium Distance (LEQ), ft-Flow Outer Lanes (vOA), pc/h/ln2329Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on F	reeway (NO)	2			
Distance to Downstream Ramp (LDOWN), ft-On-Ramp Influence Area Speed (SR), mi/h49.4Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Distance to Upstream Ramp (LUP), ft	-	Speed Index (MS)		0.433			
Prop. Freeway Vehicles in Lane 1 and 2 (PFM)0.130Outer Lanes Freeway Speed (SO), mi/h48.3Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Downstream Equilibrium Distance (LE	Q), ft -	Flow Outer Lanes (vOA), pc/h	n/ln	2329			
Flow in Lanes 1 and 2 (v12), pc/h3106Ramp Junction Speed (S), mi/h48.8Flow Entering Ramp-Infl. Area (vR12), pc/h3806Average Density (D), pc/mi/ln43.4	Distance to Downstream Ramp (LDOW	/N), ft -	On-Ramp Influence Area Spe	eed (SR), mi/h	49.4			
Flow Entering Ramp-Infl. Area (vR12), pc/h     3806     Average Density (D), pc/mi/ln     43.4	Prop. Freeway Vehicles in Lane 1 and	2 (Рғм) 0.130	Outer Lanes Freeway Speed	(SO), mi/h	48.3			
	Flow in Lanes 1 and 2 (v12), pc/h	3106	Ramp Junction Speed (S), m	i/h	48.8			
Level of Service (LOS)     D     Density in Ramp Influence Area (DR), pc/mi/ln     29.3	Flow Entering Ramp-Infl. Area (vR12),	pc/h 3806	Average Density (D), pc/mi/ln 43.4					
	Level of Service (LOS)	Density in Ramp Influence Area (DR), pc/mi/ln 29.3						

Service Volume Tal	ole										
Target LOS		A			В		с	Т	D		E
Freeway											
Max Service Flow Rate (MS	F), pc/h/ln	566		ŕ	1327		1857		205	8	-
Service Flow Rate (SF), veh,	/h	2186		5	5127		7175		795	52	-
Service Volume, veh/h		2186		5	5127		7175		795	52	-
One Direction DSV, 1000 v	eh/day	22			51		72		80	)	-
Bi-Directional DSV, 1000 ve	eh/day	40			93		130		14	5	-
Ramp		·									
Max Service Flow Rate (MS	F), pc/h/ln	204			479		670		74	2	-
Service Flow Rate (SF), veh,	/h	204			479		670		74	2	-
Service Volume, veh/h		204			479		670		74	2	-
One Direction DSV, 1000 v	eh/day	2			5		7		7		-
Design Analysis Tal	ble										
Freeway Lanes, In	2	2	3		3	4		4		5	5
Ramp Lanes, In	1	2	1		2	1		2		1	2
Density, pc/mi/ln	-	-	-		-	43.4	ļ.	43.2		33.3	33.2
LOS	F	F	F		F	D		С		С	В

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Project Information					
Analyst B	en Erban	Date	2/25/2019		
Agency C	TPS	Analysis Year	2019		
Jurisdiction N	lassDOT District 6	Time Period Analyzed	4:00 to 5:0	0 PM	
C	uincy - Exit 8 On-Ramp - Low ost Freeway Bottlenecks Iternative 2	Unit	United Sta	tes Customary	
Geometric Data					
		Freeway	Ramp		
Number of Lanes (N), In		4	1		
Free-Flow Speed (FFS), mi/h		55.0	35.0		
Segment Length (L) / Acceleration Ler	ngth (LA),ft	1500	1500		
Terrain Type		Specific Grade	Specific G	rade	
Percent Grade, %		1.00	1.00		
Segment Type / Ramp Side		Freeway	Right		
Adjustment Factors					
Driver Population		Mostly Familiar	Mostly Far	niliar	
Weather Type		Non-Severe Weather	Non-Sever	re Weather	
Incident Type		No Incident	-		
Final Speed Adjustment Factor (SAF)		1.000	1.000		
Final Capacity Adjustment Factor (CAI	F)	1.000			
Demand Adjustment Factor (DAF)		1.000			
Demand and Capacity					
Demand Volume (Vi)		7500	700		
Peak Hour Factor (PHF)		1.00	1.00		
Total Trucks, %		2.00	0.00		
Single-Unit Trucks (SUT), %		70	100		
Tractor-Trailers (TT), %		30	0		
Heavy Vehicle Adjustment Factor (fHV	)	0.966	1.000		
Flow Rate (vi),pc/h		7764	700		
Capacity (c), pc/h		9000	2000		
Volume-to-Capacity Ratio (v/c)		0.94	0.35		
Speed and Density					
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on Free	eway (NO)	2	
Distance to Upstream Ramp (LUP), ft -		Speed Index (MS)		0.391	
Downstream Equilibrium Distance (LE	Q), ft -	Flow Outer Lanes (vOA), pc/h/lr	ו <u>יי</u>	2329	
Distance to Downstream Ramp (LDOW	/N), ft -	On-Ramp Influence Area Speed	d (SR), mi/h	49.9	
Prop. Freeway Vehicles in Lane 1 and	2 (Рғм) 0.130	Outer Lanes Freeway Speed (Sc	D), mi/h	48.3	
Flow in Lanes 1 and 2 (v12), pc/h	3106	Ramp Junction Speed (S), mi/h		49.0	
Flow Entering Ramp-Infl. Area (vR12),	pc/h 3806	Average Density (D), pc/mi/ln		43.2	
Level of Service (LOS)	Density in Ramp Influence Area (DR), pc/mi/ln 25.5				

Service Volume Tab	le									
Target LOS		A			В	с		D		E
Freeway		- -		<u>.</u>						
Max Service Flow Rate (MSF	), pc/h/ln	585		-	575	2058		-		-
Service Flow Rate (SF), veh/h	า	2261		6	5086	7952		-		-
Service Volume, veh/h		2261		6	5086	7952		-		-
One Direction DSV, 1000 ve	h/day	23			61	80		-		-
Bi-Directional DSV, 1000 vel	n/day	41			111	145		-		-
Ramp				<u>.</u>						
Max Service Flow Rate (MSF	), pc/h/ln	211			568	742		-		-
Service Flow Rate (SF), veh/h	า	211			568	742		-		-
Service Volume, veh/h		211			568	742		-		-
One Direction DSV, 1000 ve	h/day	2			6	7		-		-
Design Analysis Tab	le									
Freeway Lanes, In	2	2	3		3	4	4		5	5
Ramp Lanes, In	1	2	1		2	1	2		1	2
Density, pc/mi/ln	-	-	-		-	43.2	43.2	.2 33.2		33.2
LOS	F	F	F		F	С	С		В	В

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Project Information		,					
-	Ben Erban		Date	2/25/2019			
,	CTPS			2019			
5,		iet C	Analysis Year				
	MassDOT Distr		Time Period Analyzed	4:00 to 5:0			
· · · · · · · · · · · · · · · · · · ·	Cost Freeway B Alternative 3	On-Ramp - Low ottlenecks	Unit	United Sta	ites Customary		
Geometric Data							
			Freeway	Ramp			
Number of Lanes (N), In			4	1			
Free-Flow Speed (FFS), mi/h			55.0	35.0			
Segment Length (L) / Acceleration Le	ength (LA),ft		1500	1500			
Terrain Type			Specific Grade	Specific G	rade		
Percent Grade, %			1.00	1.00			
Segment Type / Ramp Side			Freeway	Right			
Adjustment Factors			•				
Driver Population			Mostly Familiar	Mostly Far	niliar		
Weather Type			Non-Severe Weather	Non-Sever	re Weather		
Incident Type			No Incident	o Incident -			
Final Speed Adjustment Factor (SAF)			1.000	1.000			
Final Capacity Adjustment Factor (CA	λF)		1.000				
Demand Adjustment Factor (DAF)			1.000				
Demand and Capacity							
Demand Volume (Vi)			7500	700			
Peak Hour Factor (PHF)			1.00	1.00			
Total Trucks, %			2.00	0.00			
Single-Unit Trucks (SUT), %			70	100			
Tractor-Trailers (TT), %			30	0			
Heavy Vehicle Adjustment Factor (fH	V)		0.966	1.000			
Flow Rate (vi),pc/h			7764	700			
Capacity (c), pc/h			9000	2000			
Volume-to-Capacity Ratio (v/c)			0.94	0.35			
Speed and Density							
Upstream Equilibrium Distance (LEQ), ft -			Number of Outer Lanes on Free	eway (No)	2		
Distance to Upstream Ramp (LUP), ft -			Speed Index (MS)		0.391		
Downstream Equilibrium Distance (Le	EQ), ft -		Flow Outer Lanes (vOA), pc/h/ln		2329		
Distance to Downstream Ramp (LDOV	wn), ft -		On-Ramp Influence Area Speed	(SR), mi/h	49.9		
Prop. Freeway Vehicles in Lane 1 and	d 2 (РFM) 0.1	30	Outer Lanes Freeway Speed (SO	), mi/h	48.3		
Flow in Lanes 1 and 2 (v12), pc/h	310	)6	Ramp Junction Speed (S), mi/h 49.0				
Flow Entering Ramp-Infl. Area (vR12),	, pc/h 380	)6	Average Density (D), pc/mi/ln		43.2		
Level of Service (LOS) C			Density in Ramp Influence Area (DR), pc/mi/ln 25.5				

Service Volume Tab	le										
Target LOS		A			В	с		D			E
Freeway											
Max Service Flow Rate (MSF	), pc/h/ln	585		-	575	2058		-			-
Service Flow Rate (SF), veh/h	h	2261		6	5086	7952		-			-
Service Volume, veh/h		2261		6	5086	7952		-			-
One Direction DSV, 1000 ve	h/day	23			61	80		-			-
Bi-Directional DSV, 1000 vel	h/day	41			111	145		-			-
Ramp		·			°						
Max Service Flow Rate (MSF	), pc/h/ln	211			568	742		-			-
Service Flow Rate (SF), veh/h	n	211			568	742		-			-
Service Volume, veh/h		211			568	742		-			-
One Direction DSV, 1000 ve	h/day	2			6	7		-			-
Design Analysis Tab	le										
Freeway Lanes, In	2	2	3		3	4	4		5		5
Ramp Lanes, In	1	2	1		2	1	2		1		2
Density, pc/mi/ln	-	-	-		-	43.2	43.2	.2 33.2			33.2
LOS	F	F	F		F	С	С		В		В

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			merge Report					
Project Information				-				
Analyst E	Ben Erban		Date	2/25/2019				
Agency	CTPS		Analysis Year	2019				
Jurisdiction N	MassDOT D	istrict 6	Time Period Analyzed	0 PM				
	Quincy - HC Cost Freewa Existing	)V Lane Merge - Low y Bottlenecks	Unit	United Sta	tes Customary			
Geometric Data								
			Freeway	Ramp				
Number of Lanes (N), In			4	1				
Free-Flow Speed (FFS), mi/h			55.0	55.0				
Segment Length (L) / Acceleration Le	ength (LA),ft	:	1200	430				
Terrain Type			Specific Grade	Specific Gr	ade			
Percent Grade, %			1.00	1.00				
Segment Type / Ramp Side			Freeway	Left				
Adjustment Factors								
Driver Population			Mostly Familiar	Mostly Far	niliar			
Weather Type			Non-Severe Weather	Non-Sever	e Weather			
Incident Type			No Incident					
Final Speed Adjustment Factor (SAF)			1.000	1.000				
Final Capacity Adjustment Factor (CA	AF)		1.000					
Demand Adjustment Factor (DAF)			1.000					
Demand and Capacity								
Demand Volume (Vi)			5500	1500				
Peak Hour Factor (PHF)			0.95	0.95				
Total Trucks, %			2.00	0.00				
Single-Unit Trucks (SUT), %			70					
Tractor-Trailers (TT), %			30	0				
Heavy Vehicle Adjustment Factor (fH	V)		0.966	1.000				
Flow Rate (vi),pc/h			5993	1579				
Capacity (c), pc/h			9000	2200				
Volume-to-Capacity Ratio (v/c)			0.84	0.72				
Speed and Density								
Upstream Equilibrium Distance (LEQ),	, ft	-	Number of Outer Lanes on Freev	way (NO)	2			
Distance to Upstream Ramp (LUP), ft		-	Speed Index (MS)		0.482			
Downstream Equilibrium Distance (L	EQ), ft	-	Flow Outer Lanes (vOA), pc/h/ln		1798			
Distance to Downstream Ramp (LDO)	WN), ft	-	On-Ramp Influence Area Speed	(SR), mi/h	48.7			
Prop. Freeway Vehicles in Lane 3 and	1 4 (PFM)	0.020	Outer Lanes Freeway Speed (SO)	, mi/h	50.3			
Flow in Lanes 3 and 4 (v34), pc/h		2397	Ramp Junction Speed (S), mi/h		49.4			
Flow Entering Ramp-Infl. Area (vR34),	, pc/h	3976	Average Density (D), pc/mi/ln		38.3			
Level of Service (LOS) D			Density in Ramp Influence Area (DR), pc/mi/In 33.1					

Service Volume Ta	ble											
Target LOS		A			В		с	Т	D			E
Freeway												
Max Service Flow Rate (MS	SF), pc/h/ln	346	346		846		1242		158	9		1768
Service Flow Rate (SF), veh	/h	1338		3	3271		4800		613	8		6831
Service Volume, veh/h		1271		3	3107		4560		5831			6489
One Direction DSV, 1000 v	eh/day	13			31		46		58			65
Bi-Directional DSV, 1000 v	eh/day	23			56		82		105			117
Ramp												
Max Service Flow Rate (MS	SF), pc/h/ln	365			892		1309		167	74		1863
Service Flow Rate (SF), veh	/h	365			892	1309		167	4		1863	
Service Volume, veh/h		347			847		1244		159	0		1770
One Direction DSV, 1000 v	eh/day	3			8		12		16			18
Design Analysis Ta	ble											
Freeway Lanes, In	2	2	3		3	4	4	4		5		5
Ramp Lanes, In	1	2 1			2	1	1	2		1		2
Density, pc/mi/ln	-			-		3	38.3	38.0		29.8		29.6
LOS	F	F	F		F	1	D	D		D		С

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Project Information					
Project Information	en Erban	Data	2/25/2019		
		Date			
	TPS	Analysis Year	2019		
	1assDOT District 6	Time Period Analyzed	4:00 to 5:0		
C	Quincy - HOV Lane Merge - Low ost Freeway Bottlenecks Iternative 1	Unit	United Sta	tes Customary	
Geometric Data					
		Freeway	Ramp		
Number of Lanes (N), In		4	1		
Free-Flow Speed (FFS), mi/h		55.0	55.0		
Segment Length (L) / Acceleration Ler	2400	1100			
Terrain Type	Specific Grade	Specific Gr	ade		
Percent Grade, %		1.00	1.00		
Segment Type / Ramp Side	Freeway	Left			
Adjustment Factors					
Driver Population		Mostly Familiar	Mostly Far	niliar	
Weather Type		Non-Severe Weather	evere Weather Non-Sever		
Incident Type		No Incident	-		
Final Speed Adjustment Factor (SAF)		1.000	1.000		
Final Capacity Adjustment Factor (CAI	F)	1.000	1.000		
Demand Adjustment Factor (DAF)		1.000	1.000		
Demand and Capacity					
Demand Volume (Vi)		6000	1500		
Peak Hour Factor (PHF)		1.00			
Total Trucks, %		2.00	0.00		
Single-Unit Trucks (SUT), %		70	100		
Tractor-Trailers (TT), %		30	0		
Heavy Vehicle Adjustment Factor (fHV	()	0.966	1.000		
Flow Rate (vi),pc/h		6211	1500		
Capacity (c), pc/h		9000	2200		
Volume-to-Capacity Ratio (v/c)		0.86	0.68		
Speed and Density					
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on Free	way (NO)	2	
Distance to Upstream Ramp (LUP), ft -		Speed Index (MS)		0.410	
Downstream Equilibrium Distance (LEQ), ft -		Flow Outer Lanes (vOA), pc/h/ln		1864	
Distance to Downstream Ramp (LDOWN), ft -		On-Ramp Influence Area Speed (SR), mi/h		49.7	
Prop. Freeway Vehicles in Lane 3 and	Outer Lanes Freeway Speed (SO)	50.1			
Flow in Lanes 3 and 4 (v34), pc/h	Ramp Junction Speed (S), mi/h 49.9				
Flow Entering Ramp-Infl. Area (vR34),	Average Density (D), pc/mi/ln		38.6		
Level of Service (LOS)	D	Density in Ramp Influence Area	(DR), pc/mi/ln	29.0	

Target LOS	<u> </u>				В	с		D			E	
Freeway									1		1	
Max Service Flow Rate	(MSF), pc/h/ln		529		-	1089	1497		180	0		-
Service Flow Rate (SF),	veh/h		2044		2	1206	5786		695	6955		-
Service Volume, veh/h			2044		4	1206	5786		695	5		-
One Direction DSV, 100	)0 veh/day		20			42	58		70			-
Bi-Directional DSV, 100	0 veh/day		37			76	105		126	5		-
Ramp												
Max Service Flow Rate	(MSF), pc/h/ln		511		1	1052	1446		173	9		-
Service Flow Rate (SF),	veh/h		511			1052	1446		173	9		-
Service Volume, veh/h			511			1052	1446		173	9		-
One Direction DSV, 100	00 veh/day		5			11	14		17			-
Design Analysis	Table											
Freeway Lanes, In	2	2		3		3	4	4		5		5
Ramp Lanes, In	1	2		1		2	1	2		1		2
Density, pc/mi/ln	-	-				-	38.6	3	8.4	30.1		29.9
LOS	F	F	F F			F	D	С		С		С
Managed Lane G	Geometric Da	ata										
Managed Lane Type			Continuous Acc		cess	Free-Flow Sp	peed (FFS), mi	i/h		7	75.4	
Number of Managed La	anes, In		1			Terrain Type	5			L	evel	
Managed Lane Length,	ft		5280			Percent Grad	de, %			-		
Managed Lane A	Adjustment F	acto	ŕS									
Driver Population			All Famil	iar		Driver Popul	oulation CAF			1	.000	
Weather Type			Non-Sev	-Severe Weather Weath		Weather Typ	eather Type CAF			1.000		
Driver Population SAF			1.000			Final Speed	ed Adjustment Factor (SAF)			1.000		
Weather Type SAF			1.000			Final Capaci	ty Adjustment	t Fac	tor (CAF)	1	.000	
Demand Adjustment Fa	actor (DAF)		1.000									
Managed Lane D	Demand and	Сара	city									
Volume (Vм∟), veh/h			0			Heavy Vehic	le Adjustmen	t Fac	tor (f <sub>HV</sub> )	1	.000	
Peak Hour Factor			0.94			Flow Rate (V	/ <sub>p,ML</sub> ), pc/h/ln			C	)	
Total Trucks, %			0.00			Capacity (c),	pc/h/ln			1	804	
Single-Unit Trucks (SUT	Г), %		-			Adjusted Cp	acity (Cadj), pc	/h/lr	1	1	804	
Tractor-Trailers (TT), %			-		Volume-to-0	Capacity Ratic	) (v/c	:)	C	0.00		
Passenger Car Equivalent (ET) 2.000												
Managed Lane S	peed and D	ensity	/									
Breakpoint (BPML)			500			Indicator Variable (Ic)			-			
						Average Speed (SML), mi/h						
Speed 1 (S1), mi/h			75.4			Average Spe	eed (SмL), mi/ł	ו		7	75.4	

Project Information		,			
-	en Erban	Date	2/25/2019		
, ,			2/25/2019		
	TPS 1assDOT District 6	Analysis Year Time Period Analyzed	4:00 to 5:0		
		Unit			
C	Quincy - HOV Lane Merge - Low ost Freeway Bottlenecks Iternative 2	Unit	tes Customary		
Geometric Data					
		Freeway	Ramp		
Number of Lanes (N), In		4	1		
Free-Flow Speed (FFS), mi/h		55.0	55.0		
Segment Length (L) / Acceleration Lei	2400	430			
Terrain Type	Specific Grade	Specific Gr	ade		
Percent Grade, %		1.00	1.00		
Segment Type / Ramp Side	Freeway	Left			
Adjustment Factors					
Driver Population	Mostly Familiar	Mostly Far	niliar		
Weather Type		Non-Severe Weather	Severe Weather Non-Sever		
Incident Type	No Incident	-			
Final Speed Adjustment Factor (SAF)		1.000	1.000		
Final Capacity Adjustment Factor (CA	F)	1.000	1.000		
Demand Adjustment Factor (DAF)		1.000	1.000		
Demand and Capacity					
Demand Volume (Vi)		6000	1500		
Peak Hour Factor (PHF)		1.00	00 1.00		
Total Trucks, %		2.00	1.00		
Single-Unit Trucks (SUT), %		70	100		
Tractor-Trailers (TT), %		30	0		
Heavy Vehicle Adjustment Factor (fHV	()	0.966	0.983		
Flow Rate (vi),pc/h		6211	1526		
Capacity (c), pc/h		9000	2200		
Volume-to-Capacity Ratio (v/c)		0.86	0.69		
Speed and Density		• •			
Upstream Equilibrium Distance (LEQ),	ft -	Number of Outer Lanes on Fre	eway (NO)	2	
Distance to Upstream Ramp (LUP), ft -		Speed Index (MS)		0.489	
Downstream Equilibrium Distance (LEQ), ft -		Flow Outer Lanes (vOA), pc/h/lr	n	1864	
Distance to Downstream Ramp (LDOWN), ft -		On-Ramp Influence Area Speed (SR), mi/h		48.6	
Prop. Freeway Vehicles in Lane 3 and	Outer Lanes Freeway Speed (So	50.1			
Flow in Lanes 3 and 4 (v34), pc/h	Ramp Junction Speed (S), mi/h 49.3				
Flow Entering Ramp-Infl. Area (vR34),	Average Density (D), pc/mi/ln		39.2		
Level of Service (LOS)	D	Density in Ramp Influence Area	a (DR), pc/mi/ln	33.4	
·					

Tarrat I OC		-	•			P			-		1	-
Target LOS Freeway			A			B	С		D			E
Max Service Flow Rate	(MSE) nc/h/ln	1	356			869	1275		163	0		1800
Service Flow Rate (SF),	·		1376			3356	4926		6299		_	6955
Service Volume, veh/h	Venym		1376			3356	4926			6299		6955
One Direction DSV, 100	)0 veh/dav	-	14	$\rightarrow$		34	49		63		+	70
Bi-Directional DSV, 100		+	14	-		34	49		63		-	70
Ramp												
Max Service Flow Rate	ervice Flow Rate (MSF), pc/h/ln 350		Т		854	1253		160	2	Т	1769	
Service Flow Rate (SF),	veh/h		344			839	1232		157	5		1739
Service Volume, veh/h			344			839	1232		157	5		1739
One Direction DSV, 100	00 veh/day		3			8	12		16			17
Design Analysis	Table											
Freeway Lanes, In	2	2		3		3	4	4		5		5
Ramp Lanes, In	1	2		1		2	1	2		1		2
Density, pc/mi/ln	-	-		-		-	39.2	3	8.9	30.5		30.3
LOS	F	F	F F			F	D	D		D		С
Managed Lane G	Geometric Da	ata										
Managed Lane Type		Continuous Acce		cess	Free-Flow S	peed (FFS), m	i/h			75.4		
Number of Managed L	anes, In		1			Terrain Type				Level		
Managed Lane Length,	ft		5280			Percent Gra	de, %					
Managed Lane A	Adjustment F	acto	rs									
Driver Population			All Famil	l Familiar Driver			iver Population CAF				1.000	
Weather Type			Non-Sev	Non-Severe Weather			Weather Type CAF				1.000	
Driver Population SAF			1.000			Final Speed	peed Adjustment Factor (SAF)				1.000	
Weather Type SAF			1.000			Final Capacity Adjustment Factor (CAF)				1.000		
Demand Adjustment Fa	actor (DAF)		1.000									
Managed Lane D	Demand and	Сара	acity									
Volume (Vml), veh/h			0			Heavy Vehic	cle Adjustmen	t Fac	tor (fнv)	·	1.000	
Peak Hour Factor			0.94			Flow Rate (V	/ <sub>р,ML</sub> ), pc/h/ln			(	D	
Total Trucks, %			0.00			Capacity (c),	, pc/h/ln				1804	
Single-Unit Trucks (SUT	Г), %		-			Adjusted Cp	acity (c₁dj), pc	/h/ln			1804	
Tractor-Trailers (TT), %			-			Volume-to-	Capacity Ratio	o (v/c	)	(	0.00	
Passenger Car Equivale	ent (E⊤)		2.000									
Managed Lane S	peed and D	ensit	у									
Breakpoint (BP <sub>ML</sub> )			500			Indicator Variable (Ic)			•	-		
Speed 1 (S1), mi/h			75.4			Average Speed (SmL), mi/h			1	75.4		
Speed I (SI), III/II			75.4		Density (DML), pc/mi/ln							

		,					
Project Information							
Analyst	Ben Erban		Date	2/25/2019			
Agency	CTPS		Analysis Year	2019			
Jurisdiction	MassDOT	District 6	Time Period Analyzed	4:00 to 5:0	0 PM		
Project Description		OV Lane Merge - Low vay Bottlenecks 2 3	Unit United States Customary				
Geometric Data							
			Freeway	Ramp			
Number of Lanes (N), In			4	1			
Free-Flow Speed (FFS), mi/h			55.0	55.0			
Segment Length (L) / Acceleration Length (LA),ft			2400	1100			
Terrain Type			Specific Grade	Specific Gr	ade		
Percent Grade, %			1.00	1.00			
Segment Type / Ramp Side			Freeway	Left			
Adjustment Factors			^				
Driver Population	Driver Population		Mostly Familiar	Mostly Far	niliar		
/eather Type			Non-Severe Weather Non-Sever		e Weather		
Incident Type			No Incident	-			
Final Speed Adjustment Factor (SAF)			1.000	1.000			
Final Capacity Adjustment Factor (C	AF)		1.000	1.000			
Demand Adjustment Factor (DAF)			1.000	1.000			
Demand and Capacity							
Demand Volume (Vi)			6000	1500			
Peak Hour Factor (PHF)			1.00	0 1.00			
Total Trucks, %			2.00	0.00			
Single-Unit Trucks (SUT), %			70	100			
Tractor-Trailers (TT), %			30	0			
Heavy Vehicle Adjustment Factor (fi	HV)		0.966	1.000			
Flow Rate (vi),pc/h			6211	1500			
Capacity (c), pc/h			9000	2200			
Volume-to-Capacity Ratio (v/c)			0.86	0.68			
Speed and Density							
Upstream Equilibrium Distance (LEQ	)), ft	-	Number of Outer Lanes on Free	way (NO)	2		
Distance to Upstream Ramp (LUP), ft -		-	Speed Index (MS)		0.410		
Downstream Equilibrium Distance (LEQ), ft -		Flow Outer Lanes (vOA), pc/h/ln		1864			
Distance to Downstream Ramp (LDOWN), ft -		On-Ramp Influence Area Speed (SR), mi/h		49.7			
Prop. Freeway Vehicles in Lane 3 and 4 (PFM) 0.030		Outer Lanes Freeway Speed (SO), mi/h		50.1			
Flow in Lanes 3 and 4 (v34), pc/h	IU 4 (PFM)	2484	Ramp Junction Speed (S), mi/h		49.9		
		2484 3984	Ramp Junction Speed (S), mi/h Average Density (D), pc/mi/ln		49.9 38.6		

Target LOS	<u> </u>				В	с		D			E	
Freeway									1		1	
Max Service Flow Rate	(MSF), pc/h/ln		529		-	1089	1497		180	0		-
Service Flow Rate (SF),	veh/h		2044		2	1206	5786		695	6955		-
Service Volume, veh/h			2044		4	1206	5786		695	5		-
One Direction DSV, 100	)0 veh/day		20			42	58		70			-
Bi-Directional DSV, 100	0 veh/day		37			76	105		126	5		-
Ramp												
Max Service Flow Rate	(MSF), pc/h/ln		511		1	1052	1446		173	9		-
Service Flow Rate (SF),	veh/h		511			1052	1446		173	9		-
Service Volume, veh/h			511			1052	1446		173	9		-
One Direction DSV, 100	00 veh/day		5			11	14		17			-
Design Analysis	Table											
Freeway Lanes, In	2	2		3		3	4	4		5		5
Ramp Lanes, In	1	2		1		2	1	2		1		2
Density, pc/mi/ln	-	-				-	38.6	3	8.4	30.1		29.9
LOS	F	F	F F			F	D	С		С		С
Managed Lane G	Geometric Da	ata										
Managed Lane Type			Continuous Acc		cess	Free-Flow Sp	peed (FFS), mi	i/h		7	75.4	
Number of Managed La	anes, In		1			Terrain Type	5			L	evel	
Managed Lane Length,	ft		5280			Percent Grad	de, %			-		
Managed Lane A	Adjustment F	acto	ŕS									
Driver Population			All Famil	iar		Driver Popul	oulation CAF			1	.000	
Weather Type			Non-Sev	-Severe Weather Weath		Weather Typ	eather Type CAF			1.000		
Driver Population SAF			1.000			Final Speed	ed Adjustment Factor (SAF)			1.000		
Weather Type SAF			1.000			Final Capaci	ty Adjustment	t Fac	tor (CAF)	1	.000	
Demand Adjustment Fa	actor (DAF)		1.000									
Managed Lane D	Demand and	Сара	city									
Volume (Vм∟), veh/h			0			Heavy Vehic	le Adjustmen	t Fac	tor (f <sub>HV</sub> )	1	.000	
Peak Hour Factor			0.94			Flow Rate (V	/ <sub>p,ML</sub> ), pc/h/ln			C	)	
Total Trucks, %			0.00			Capacity (c),	pc/h/ln			1	804	
Single-Unit Trucks (SUT	Г), %		-			Adjusted Cp	acity (Cadj), pc	/h/lr	1	1	804	
Tractor-Trailers (TT), %			-		Volume-to-0	Capacity Ratic	) (v/c	:)	C	0.00		
Passenger Car Equivalent (ET) 2.000												
Managed Lane S	peed and D	ensity	/									
Breakpoint (BPML)			500			Indicator Variable (Ic)			-			
						Average Speed (SML), mi/h						
Speed 1 (S1), mi/h			75.4			Average Spe	eed (SмL), mi/ł	ו		7	75.4	

Project Information				
Analyst	Ben Erban	Date		2/25/2019
Agency	CTPS	Analysis Year		2019
Jurisdiction	MassDOT District 6	Time Period Analyzed		
Project Description	Quincy - One-Sided Weave from HOV Lane to Route 3 - Existing	Unit		United States Customary
Geometric Data				
Number of Lanes (N), In	4	Segment Type		Freeway
Segment Length (Ls), ft	1900	Number of Maneuver	Lanes (NWL), In	2
Weaving Configuration	One-Sided	Ramp-to-Freeway Lane	e Changes (LCRF), lc	2
Terrain Type	Specific Grade	Freeway-to-Ramp Lane	e Changes (LCFR), lc	0
Percent Grade, %	1.00	Ramp-to-Ramp Lane C	Changes (LCRR), lc	0
Interchange Density (ID), int/mi	2.00	Cross Weaving Manag	ed Lane	No
Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustmer	nt Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Final Capacity Adjustm	ent Factor (CAF)	1.000
Incident Type	No Incident	Demand Adjustment F	actor (DAF)	1.000
Demand and Capacity				
	FF	RF	RR	FR
Demand Volume (Vi), veh/h	2500	700	600	2600
Peak Hour Factor (PHF)	1.00	1.00	1.00	1.00
Total Trucks, %	2.00	0.00	0.00	2.00
Heavy Vehicle Adjustment Factor (fHV)	0.970	1.000	1.000	0.968
Flow Rate (vi), pc/h	2577	700	600	2686
Weaving Flow Rate (vw), pc/h	3386	Freeway Max Capacity	(cIFL), pc/h/ln	2250
Non-Weaving Flow Rate (vNW), pc/h	3177	Density-Based Capacity	y (cIWL), pc/h/ln	1782
Total Flow Rate (v), pc/h	6563	Demand Flow-Based C	apacity (cɪw), pc/h	4651
Volume Ratio (VR)	0.516	Weaving Segment Cap	acity (cw), veh/h	4535
Minimum Lane Change Rate (LCMIN), lc/h	0	Adjusted Weaving Area	a Capacity, pc/h	4651
Maximum Weaving Length (LMAX), ft	8014	Volume-to-Capacity Ra	atio (v/c)	1.41
Speed and Density				
Non-Weaving Vehicle Index (INW)	-	Average Weaving Spee	ed (Sw), mi/h	-
Non-Weaving Lane Change Rate (LCNW), lc/h	-	Average Non-Weaving	Speed (SNW), mi/h	-
Weaving Lane Change Rate (LCw), lc/h	-	Average Speed (S), mi	ſh	-
Weaving Lane Change Rate (LCAII), lc/h	-	Density (D), pc/mi/ln		-
Weaving Intensity Factor (W)	-	Level of Service (LOS)		F
Service Volume Table				

A		В	Ľ	C D		E
pc/h/ln 493		925	1163	1163 116		1163
(SF), veh/h 1912		3589	4511	451	1	4511
ervice Volume, veh/h 1912		3589	4511	451	1	4511
Direction DSV, 1000 veh/day 19		36	45	45		45
19		36	45	45		45
	5		6		7	
			-		-	
			F		F	
	1912 1912 191 19	1912       1         1912       1         19       1         19       1         19       5         -       -         F       -		$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	

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Quincy - From HOV to 93 SB - One-Sided Weave (Ramp is Route 3) Existing.xuf

Project Information				
Analyst	Ben Erban	Date		2/25/2019
Agency	СТРЅ	Analysis Year		2019
Jurisdiction	MassDOT District 6	Time Period Analyzed		
Project Description	Quincy - One-Sided Weave from HOV Lane to Route 3 - Alternative 1	Unit		United States Customary
Geometric Data				
Number of Lanes (N), In	4	Segment Type		Freeway
Segment Length (Ls), ft	2600	Number of Maneuver I	Lanes (NWL), In	2
Weaving Configuration	One-Sided	Ramp-to-Freeway Lane	e Changes (LCRF), lc	2
Terrain Type	Specific Grade	Freeway-to-Ramp Lane	e Changes (LCFR), lc	0
Percent Grade, %	2.00	Ramp-to-Ramp Lane C	Changes (LCRR), lc	0
Interchange Density (ID), int/mi	2.00	Cross Weaving Manag	ed Lane	No
Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustmer	nt Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Final Capacity Adjustm	ent Factor (CAF)	1.000
Incident Type	No Incident	Demand Adjustment F	actor (DAF)	1.000
Demand and Capacity				·
	FF	RF	RR	FR
Demand Volume (Vi), veh/h	2500	700	600	2600
Peak Hour Factor (PHF)	1.00	1.00	1.00	1.00
Total Trucks, %	2.00	0.00	0.00	2.00
Heavy Vehicle Adjustment Factor (fHV)	0.968	1.000	1.000	0.962
Flow Rate (vi), pc/h	2583	700	600	2703
Weaving Flow Rate (vw), pc/h	3403	Freeway Max Capacity	(cIFL), pc/h/ln	2250
Non-Weaving Flow Rate (vNW), pc/h	3183	Density-Based Capacity	y (cIWL), pc/h/ln	1835
Total Flow Rate (v), pc/h	6586	Demand Flow-Based C	apacity (cɪw), pc/h	4642
Volume Ratio (VR)	0.517	Weaving Segment Cap	acity (cw), veh/h	4512
Minimum Lane Change Rate (LCMIN), lc/h	0	Adjusted Weaving Area	a Capacity, pc/h	4643
Maximum Weaving Length (LMAX), ft	8026	Volume-to-Capacity Ra	atio (v/c)	1.42
Speed and Density				
Non-Weaving Vehicle Index (INW)	-	Average Weaving Spee	ed (Sw), mi/h	-
Non-Weaving Lane Change Rate (LCNW), lc/h	-	Average Non-Weaving	Speed (SNW), mi/h	-
Weaving Lane Change Rate (LCW), lc/h	-	Average Speed (S), mi/	′h	-
Weaving Lane Change Rate (LCAII), lc/h	-	Density (D), pc/mi/ln		-
Weaving Intensity Factor (W)	-	Level of Service (LOS)		F
Service Volume Table				

Target LOS		A		В	С	D		E
Max Service Flow Rate (MS	SF), pc/h/ln	pc/h/ln 494		929	1161	1161 1161		1161
Service Flow Rate (SF), veh	vice Flow Rate (SF), veh/h 1912			3596	4494	449	4	4494
Service Volume, veh/h	e, veh/h 1912			3596	4494	449	4	4494
One Direction DSV, 1000 v	/, 1000 veh/day 19			36	45	45	1	45
Bi-Directional DSV, 1000 v	eh/day	35		65	82	82		82
Design Analysis Ta	ble							
Number of Lanes, In	4		5		6		7	
Density, pc/mi/ln	-	-			-		-	
LOS	F	F			F		F	
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Quincy - From HOV to 93 SB - One-Sided Weave (Ramp is Route 3) Alternative 1.xuf

Project Information				
Analyst	Ben Erban	Date		2/25/2019
Agency	CTPS	Analysis Year		2019
Jurisdiction	MassDOT District 6	Time Period Analyzed		
Project Description	Quincy - One-Sided Weave from HOV Lane to Route 3 - Alternative 2	Unit		United States Customary
Geometric Data				
Number of Lanes (N), In	4	Segment Type		Freeway
Segment Length (Ls), ft	2600	Number of Maneuver	Lanes (NWL), In	2
Weaving Configuration	One-Sided	Ramp-to-Freeway Lane	e Changes (LCRF), lc	2
Terrain Type	Specific Grade	Freeway-to-Ramp Lane	e Changes (LCFR), lc	0
Percent Grade, %	1.00	Ramp-to-Ramp Lane C	Changes (LCRR), lc	0
Interchange Density (ID), int/mi	2.00	Cross Weaving Manag	ed Lane	No
Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustmer	nt Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Final Capacity Adjustm	ent Factor (CAF)	1.000
Incident Type	No Incident	Demand Adjustment F	actor (DAF)	1.000
Demand and Capacity				
	FF	RF	RR	FR
Demand Volume (Vi), veh/h	2500	700	600	2600
Peak Hour Factor (PHF)	1.00	1.00	1.00	1.00
Total Trucks, %	2.00	0.00	0.00	2.00
Heavy Vehicle Adjustment Factor (fHV)	0.968	1.000	1.000	0.968
Flow Rate (vi), pc/h	2583	700	600	2686
Weaving Flow Rate (vw), pc/h	3386	Freeway Max Capacity	(cIFL), pc/h/ln	2250
Non-Weaving Flow Rate (vNW), pc/h	3183	Density-Based Capacity	y (cIWL), pc/h/ln	1837
Total Flow Rate (v), pc/h	6569	Demand Flow-Based C	apacity (cɪw), pc/h	4660
Volume Ratio (VR)	0.515	Weaving Segment Cap	acity (cw), veh/h	4541
Minimum Lane Change Rate (LCMIN), lc/h	0	Adjusted Weaving Area	a Capacity, pc/h	4661
Maximum Weaving Length (LMAX), ft	8002	Volume-to-Capacity Ra	atio (v/c)	1.41
Speed and Density				
Non-Weaving Vehicle Index (INW)	-	Average Weaving Spee	ed (Sw), mi/h	-
Non-Weaving Lane Change Rate (LCNW), lc/h	-	Average Non-Weaving	Speed (SNW), mi/h	-
Weaving Lane Change Rate (LCw), lc/h	-	Average Speed (S), mi/	′h	-
Weaving Lane Change Rate (LCAII), lc/h	-	Density (D), pc/mi/ln		-
Weaving Intensity Factor (W)	-	Level of Service (LOS)		F
Service Volume Table				

	Α		В	с	D		E
, pc/h/ln	494		924	1165	116	5	1165
	1912		3578	4512	451	2	4512
	1912		3578	4512	451	2	4512
n/day	19		36	45	45		45
/day	19		36	45	45		45
le							
4		5		6		7	
-	-			-		-	
F		F		F		F	
	n/day /day l <b>e</b>	, pc/h/ln 494 1912 1912 1/day 19 /day 19	, pc/h/ln 494 1912 1912 1/day 19 /day 19	n         494         924           1912         3578           1912         3578           1/day         19           /day         19           36	A pc/h/ln $494$ $924$ $1165$ $1912$ $3578$ $4512$ $1912$ $3578$ $4512$ $n/day$ $19$ $36$ $45$ $n/day$ $19$ $36$ $45$ $a y$ <	n, pc/h/ln     494     924     1165     116       1912     3578     4512     451       1912     3578     4512     451       n/day     19     36     45     45       /day     19     36     45     45       /day     19     36     6     5       -     -     -     -     -	n pc/h/ln       494       924       1165       1165         1912       3578       4512       4512         1912       3578       4512       4512         1912       3578       4512       4512         n/day       19       36       45       45         /day       19       36       45       45         /day       19       36       45       45         /day       19       36       45       7         /day       19       36       45       7         /day       19       36       45       7         /day       19       36       45       45         /day       19       36       45       7         /day       19       36       45       45

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Quincy - From HOV to 93 SB - One-Sided Weave (Ramp is Route 3) Alternative 2.xuf

Project Information				
Analyst	Ben Erban	Date		2/25/2019
Agency	CTPS	Analysis Year		2019
Jurisdiction	MassDOT District 6	Time Period Analyzed		
Project Description	Quincy - One-Sided Weave from HOV Lane to Route 3 - Alternative 3	Unit	United States Customary	
Geometric Data				
Number of Lanes (N), In	5	Segment Type		Freeway
Segment Length (Ls), ft	2600	Number of Maneuver	Lanes (NWL), In	2
Weaving Configuration	One-Sided	Ramp-to-Freeway Lane	e Changes (LCRF), lc	2
Terrain Type	Specific Grade	Freeway-to-Ramp Lane	e Changes (LCFR), lc	0
Percent Grade, %	1.00	Ramp-to-Ramp Lane C	Changes (LCRR), lc	0
Interchange Density (ID), int/mi	2.00	Cross Weaving Manag	ed Lane	No
Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustmer	nt Factor (SAF)	1.000
Weather Type	Non-Severe Weather	Final Capacity Adjustm	ent Factor (CAF)	1.000
Incident Type	No Incident	Demand Adjustment F	actor (DAF)	1.000
Demand and Capacity				
	FF	RF	RR	FR
Demand Volume (Vi), veh/h	2500	700	600	2600
Peak Hour Factor (PHF)	1.00	1.00	1.00	1.00
Total Trucks, %	2.00	0.00	0.00	2.00
Heavy Vehicle Adjustment Factor (fHV)	0.968	1.000	1.000	0.968
Flow Rate (vi), pc/h	2583	700	600	2686
Weaving Flow Rate (vw), pc/h	3386	Freeway Max Capacity	(cIFL), pc/h/ln	2250
Non-Weaving Flow Rate (vNW), pc/h	3183	Density-Based Capacity	y (cIWL), pc/h/ln	1837
Total Flow Rate (v), pc/h	6569	Demand Flow-Based C	apacity (c৷w), pc/h	4660
Volume Ratio (VR)	0.515	Weaving Segment Cap	oacity (cW), veh/h	4541
Minimum Lane Change Rate (LCMIN), lc/h	0	Adjusted Weaving Area	a Capacity, pc/h	4661
Maximum Weaving Length (LMAX), ft	8002	Volume-to-Capacity Ra	atio (v/c)	1.41
Speed and Density				
Non-Weaving Vehicle Index (INW)	-	Average Weaving Spee	ed (Sw), mi/h	-
Non-Weaving Lane Change Rate (LCNW), lc/h	-	Average Non-Weaving	J Speed (SNW), mi/h	-
Weaving Lane Change Rate (LCw), lc/h	-	Average Speed (S), mi	/h	-
Weaving Lane Change Rate (LCAII), lc/h	-	Density (D), pc/mi/ln		-
Weaving Intensity Factor (W)	-	Level of Service (LOS)		F
Service Volume Table				

Target LOS		Α		В	с	D		E
Max Service Flow Rate (MSF),	pc/h/ln	484		907	932	932	2	932
Service Flow Rate (SF), veh/h		2340		4390	4512	451	2	4512
Service Volume, veh/h		2340		4390	4512	451	2	4512
One Direction DSV, 1000 veh/	/day	23		44	45	45	1	45
Bi-Directional DSV, 1000 veh/	′day	43		80	82	82		82
Design Analysis Table	e							
Number of Lanes, In 4	ļ		5		6		7	
Density, pc/mi/ln -		-			-	-		
LOS F	:		F		F		F	
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Quincy - From HOV to 93 SB - One-Sided Weave (Ramp is Route 3) Alternative 3.xuf

Project Information				
Analyst	Ben Erban	Date		2/25/2019
Agency	CTPS	Analysis Year		2019
Jurisdiction	MassDOT District 6	Time Period Analyzed		
Project Description	Quincy - Two-Sided Weave from HOV Lane to I-93 SB (Ramp is I-93) - Existing	Unit	United States Customary	
Geometric Data				
Number of Lanes (N), In	4	Segment Type		Freeway
Segment Length (Ls), ft	1900	Number of Maneuver	Lanes (NWL), In	0
Weaving Configuration	Two-Sided	Ramp-to-Freeway Lan	e Changes (LCRF), lc	1
Terrain Type	Specific Grade	Freeway-to-Ramp Lan	e Changes (LCFR), lc	0
Percent Grade, %	1.00	Ramp-to-Ramp Lane C	Changes (LCRR), lc	3
Interchange Density (ID), int/mi	2.00	Cross Weaving Manag	ed Lane	No
Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)		1.000
Weather Type	Non-Severe Weather	Final Capacity Adjustment Factor (CAF)		1.000
Incident Type	No Incident	Demand Adjustment Factor (DAF)		1.000
Demand and Capacity				
	FF	RF RR		FR
Demand Volume (Vi), veh/h	2600	600	500	2600
Peak Hour Factor (PHF)	1.00	1.00	1.00	1.00
Total Trucks, %	2.00	0.00	0.00	2.00
Heavy Vehicle Adjustment Factor (fHV)	0.968	1.000	1.000	0.968
Flow Rate (vi), pc/h	2686	600	500	2686
Weaving Flow Rate (vw), pc/h	500	Freeway Max Capacity	(cIFL), pc/h/ln	2250
Non-Weaving Flow Rate (vNW), pc/h	5972	Density-Based Capacit	y (cIWL), pc/h/ln	1902
Total Flow Rate (v), pc/h	6472	Demand Flow-Based C	apacity (cɪw), pc/h	-
Volume Ratio (VR)	0.077	Weaving Segment Cap	acity (cw), veh/h	7406
Minimum Lane Change Rate (LCMIN), lc/h	1500	Adjusted Weaving Are	a Capacity, pc/h	7608
Maximum Weaving Length (LMAX), ft	6450	Volume-to-Capacity R	atio (v/c)	0.85
Speed and Density				
Non-Weaving Vehicle Index (INW)	-	Average Weaving Spee	ed (Sw), mi/h	-
Non-Weaving Lane Change Rate (LCNW), lc/h	-	Average Non-Weaving	J Speed (SNW), mi/h	-
Weaving Lane Change Rate (LCw), lc/h	-	Average Speed (S), mi,	′h	-
Weaving Lane Change Rate (LCAII), lc/h	-	Density (D), pc/mi/ln		-
Weaving Intensity Factor (W)	-	Level of Service (LOS)		F

Service Volume Ta	ble							
Target LOS		A		В	с	D		E
Max Service Flow Rate (MS	SF), pc/h/ln	494		894	1175	1373		1902
Service Flow Rate (SF), veh	/h	1911		3462	4550	531	7	7365
Service Volume, veh/h		1911		3462	4550	531	7	7365
One Direction DSV, 1000 v	/eh/day	19		35	45	53	}	74
Bi-Directional DSV, 1000 v	eh/day	35		63	83	97	7	134
Design Analysis Ta	ble	<u>.</u>						<u>.</u>
Number of Lanes, In	4		5		6		7	
Density, pc/mi/ln	-		33.9		27.6		23.2	
LOS	F		D		С		С	

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Project Information				
Analyst	Ben Erban	Date		2/25/2019
Agency	CTPS	Analysis Year		2019
Jurisdiction	MassDOT District 6	Time Period Analyzed		
Project Description	Quincy - Two-Sided Weave from HOV Lane to I-93 SB (Ramp is I-93) - Alternative 1	Unit		United States Customary
Geometric Data				
Number of Lanes (N), In	4	Segment Type		Freeway
Segment Length (Ls), ft	2500	Number of Maneuver	Lanes (NWL), In	0
Weaving Configuration	Two-Sided	Ramp-to-Freeway Lan	e Changes (LCRF), lc	1
Terrain Type	Specific Grade	Freeway-to-Ramp Lan	e Changes (LCFR), lc	0
Percent Grade, %	1.00	Ramp-to-Ramp Lane C	Changes (LCRR), lc	3
Interchange Density (ID), int/mi	2.00	Cross Weaving Manag	ed Lane	No
Adjustment Factors				
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)		1.000
Weather Type	Non-Severe Weather	Final Capacity Adjustment Factor (CAF)		1.000
Incident Type	No Incident	Demand Adjustment Factor (DAF)		1.000
Demand and Capacity	-	-		·
	FF	RF	RR	FR
Demand Volume (Vi), veh/h	2600	600	500	2600
Peak Hour Factor (PHF)	1.00	1.00	1.00	1.00
Total Trucks, %	2.00	0.00	0.00	2.00
Heavy Vehicle Adjustment Factor (fHV)	0.968	1.000	1.000	0.968
Flow Rate (vi), pc/h	2686	600	500	2686
Weaving Flow Rate (vw), pc/h	500	Freeway Max Capacity	(cIFL), pc/h/ln	2250
Non-Weaving Flow Rate (vNW), pc/h	5972	Density-Based Capacit	y (cIWL), pc/h/ln	1948
Total Flow Rate (v), pc/h	6472	Demand Flow-Based C	apacity (cɪw), pc/h	-
Volume Ratio (VR)	0.077	Weaving Segment Cap	oacity (cw), veh/h	7585
Minimum Lane Change Rate (LCMIN), lc/h	1500	Adjusted Weaving Are	a Capacity, pc/h	7792
Maximum Weaving Length (LMAX), ft	6450	Volume-to-Capacity Ra	atio (v/c)	0.83
Speed and Density				
Non-Weaving Vehicle Index (INW)	-	Average Weaving Spec	ed (Sw), mi/h	-
Non-Weaving Lane Change Rate (LCNW), lc/h	-	Average Non-Weaving	g Speed (SNW), mi/h	-
Weaving Lane Change Rate (LCw), lc/h	-	Average Speed (S), mi,	/h	-
Weaving Lane Change Rate (LCAII), lc/h	1			_
	-	Density (D), pc/mi/ln		

Target LOS		A			В	с	D		E	
Max Service Flow Rate (N	/ISF), pc/h/ln	494			899 1171		137	9	1948	
Service Flow Rate (SF), ve	eh/h	1911		3	3479 4534		533	8	7543	
Service Volume, veh/h		1911		3	3479	4534	533	8	7543	
One Direction DSV, 1000	veh/day	19			35	45	53		75	
Bi-Directional DSV, 1000	) veh/day 35 63 82		97	,	137					
Design Analysis T	able									
Number of Lanes, In	4		5			6		7		
Density, pc/mi/ln	-		33.8			27.5		23.2		
LOS	F		D			С		С		
Managed Lane G	eometric Da	ta								
Managed Lane Type		Continu	Continuous Access		Free-Flow S	Speed (FFS), mi/h		75	.4	
Number of Managed Lar	nes, In	1	1		Terrain Type			Le	Level	
Managed Lane Length, f	t	5280	5280		Percent Gra	ade, %		-	-	
Managed Lane A	djustment Fa	actors								
Driver Population		All Fam	liar		Driver Popu	ulation CAF		1.0	1.000	
Weather Type		Non-Se	vere Wea	ather	Weather Type CAF			1.0	000	
Driver Population SAF		1.000			Final Speed Adjustment Factor (SAF)			1.0	)00	
Weather Type SAF		1.000			Final Capacity Adjustment Factor (CAF)			1.0	000	
Demand Adjustment Fac	tor (DAF)	1.000								
Managed Lane Do	emand and	Capacity								
Volume (VмL), veh/h		0	Heavy Vehicle Adjustment Fa			ctor (f <sub>HV</sub> )	1.0	000		
Peak Hour Factor		0.94	0.94		Flow Rate (V <sub>p,ML</sub> ), pc/h/ln			0	0	
Total Trucks, %		0.00			Capacity (c), pc/h/ln				1804	
Single-Unit Trucks (SUT),	%	-			Adjusted C	pacity (c <sub>adj</sub> ), pc/h/l	n	18	04	
Tractor-Trailers (TT), %		-			Volume-to-	-Capacity Ratio (v/	c)	0.0	00	
	t (ET)	2.000								
Passenger Car Equivalen										
5 1	eed and De	nsity								
Managed Lane Sp	eed and De	500			Indicator V	ariable (Ic)		-		
Managed Lane Sp Breakpoint (BPML)	eed and De	-				ariable (Ic) peed (SмL), mi/h		- 75	.4	
Passenger Car Equivalen <b>Managed Lane Sp</b> Breakpoint (BPML) Speed 1 (S1), mi/h Speed 2 (S2), mi/h	eed and De	500			Average Sp					

Quincy - From HOV to 93 SB - Two-Sided Weave (Ramp is I-93) Alternative 1.xuf

Project Information				
Analyst	Ben Erban	Date		2/25/2019
Agency	CTPS	Analysis Year		2019
Jurisdiction	MassDOT District 6	Time Period Analyzed		
Project Description	Quincy - Two-Sided Weave from HOV Lane to I-93 SB (Ramp is I-93) - Alternative 2	Unit	United States Customary	
Geometric Data				
Number of Lanes (N), In	4	Segment Type		Freeway
Segment Length (Ls), ft	1400	Number of Maneuver	Lanes (NWL), In	0
Weaving Configuration	Two-Sided	Ramp-to-Freeway Land	e Changes (LCRF), lc	1
Terrain Type	Specific Grade	Freeway-to-Ramp Land	e Changes (LCFR), lc	0
Percent Grade, %	1.00	Ramp-to-Ramp Lane C	Changes (LCRR), lc	3
Interchange Density (ID), int/mi	2.00	Cross Weaving Manag	ed Lane	No
Adjustment Factors	•			·
Driver Population	All Familiar	Final Speed Adjustment Factor (SAF)		1.000
Weather Type	Non-Severe Weather	Final Capacity Adjustment Factor (CAF)		1.000
Incident Type	No Incident	Demand Adjustment Factor (DAF)		1.000
Demand and Capacity				·
	FF	RF RR		FR
Demand Volume (Vi), veh/h	2600	600	500	2600
Peak Hour Factor (PHF)	1.00	1.00	1.00	1.00
Total Trucks, %	2.00	0.00	0.00	2.00
Heavy Vehicle Adjustment Factor (fHV)	0.968	1.000	1.000	0.968
Flow Rate (vi), pc/h	2686	600	500	2686
Weaving Flow Rate (vw), pc/h	500	Freeway Max Capacity	(cIFL), pc/h/ln	2250
Non-Weaving Flow Rate (vNW), pc/h	5972	Density-Based Capacit	y (cIWL), pc/h/ln	1864
Total Flow Rate (v), pc/h	6472	Demand Flow-Based C	apacity (cɪw), pc/h	-
Volume Ratio (VR)	0.077	Weaving Segment Cap	acity (cw), veh/h	7258
Minimum Lane Change Rate (LCMIN), lc/h	1500	Adjusted Weaving Area	a Capacity, pc/h	7456
Maximum Weaving Length (LMAX), ft	6450	Volume-to-Capacity Ra	atio (v/c)	0.87
Speed and Density				
Non-Weaving Vehicle Index (INW)	-	Average Weaving Spee	ed (Sw), mi/h	-
Non-Weaving Lane Change Rate (LCNW), lc/h	-	Average Non-Weaving	Speed (SNW), mi/h	-
Weaving Lane Change Rate (LCW), lc/h	-	Average Speed (S), mi	′h	-
Weaving Lane Change Rate (LCAII), lc/h	-	Density (D), pc/mi/ln		-
Weaving Intensity Factor (W)	-	Level of Service (LOS)		F

Service Volume Ta	ble								
Target LOS		Α		В	с	D		E	
Max Service Flow Rate (MS	SF), pc/h/ln	492		894	1171	1382		1864	
Service Flow Rate (SF), veh	/h	1906		3463	4534	535	3	7217	
Service Volume, veh/h		1906		3463	4534	535	3	7217	
One Direction DSV, 1000 v	eh/day	19		35	45	54		72	
Bi-Directional DSV, 1000 v	eh/day	35		63	82	97		131	
Design Analysis Ta	ble	<u>.</u>						-	
Number of Lanes, In	4		5		6	6		7	
Density, pc/mi/ln	-		33.6		27.4	27.4		23.0	
LOS	F		D		С	С		с	

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Quincy - From HOV to 93 SB - Two-Sided Weave (Ramp is I-93) Alternative 2.xuf

Project Information				
Analyst	Ben Erban	Date		2/25/2019
Agency	CTPS	Analysis Year		2019
Jurisdiction	MassDOT District 6	Time Period Analyzed		
Project Description	Quincy - Two-Sided Weave from HOV Lane to I-93 SB (Ramp is I-93) - Alternative 2	Unit	United States Customary	
Geometric Data				
Number of Lanes (N), In	4	Segment Type		Freeway
Segment Length (Ls), ft	2000	Number of Maneuver	Lanes (NWL), In	0
Weaving Configuration	Two-Sided	Ramp-to-Freeway Land	e Changes (LCRF), lc	1
Terrain Type	Specific Grade	Freeway-to-Ramp Land	e Changes (LCFR), lc	0
Percent Grade, %	1.00	Ramp-to-Ramp Lane C	Changes (LCRR), lc	3
Interchange Density (ID), int/mi	2.00	Cross Weaving Manag	ed Lane	No
Adjustment Factors				
Driver Population	All Familiar	amiliar Final Speed Adjustment Factor (SAF)		1.000
Weather Type	Non-Severe Weather	Final Capacity Adjustment Factor (CAF)		1.000
Incident Type	No Incident	Demand Adjustment Factor (DAF)		1.000
Demand and Capacity				·
	FF	RF	RR	FR
Demand Volume (Vi), veh/h	2600	600	500	2600
Peak Hour Factor (PHF)	1.00	1.00 1.00		1.00
Total Trucks, %	2.00	0.00	0.00	2.00
Heavy Vehicle Adjustment Factor (fHV)	0.968	1.000	1.000	0.968
Flow Rate (vi), pc/h	2686	600	500	2686
Weaving Flow Rate (vw), pc/h	500	Freeway Max Capacity	(cIFL), pc/h/ln	2250
Non-Weaving Flow Rate (vNW), pc/h	5972	Density-Based Capacit	y (cIWL), pc/h/ln	1910
Total Flow Rate (v), pc/h	6472	Demand Flow-Based C	apacity (c৷w), pc/h	-
Volume Ratio (VR)	0.077	Weaving Segment Cap	oacity (cw), veh/h	7437
Minimum Lane Change Rate (LCMIN), lc/h	1500	Adjusted Weaving Area	a Capacity, pc/h	7640
Maximum Weaving Length (LMAX), ft	6450	Volume-to-Capacity Ra	atio (v/c)	0.85
Speed and Density				
Non-Weaving Vehicle Index (INW)	-	Average Weaving Spee	ed (Sw), mi/h	-
Non-Weaving Lane Change Rate (LCNW), lc/h	1	1		
Non-weaving Lane Change Rate (LCNW), IC/II	-	Average Non-Weaving	J Speed (SNW), mi/h	-
Weaving Lane Change Rate (LCW), Ic/h	-	Average Non-Weaving Average Speed (S), mi,	•	-
	- - -		•	

Service Volume Ta	ble								
Target LOS		A		В	с	D		E	
Max Service Flow Rate (M	SF), pc/h/ln	494		894	1175	138	80	1910	
Service Flow Rate (SF), ver	ı/h	1911		3462	4550	534	2	7396	
Service Volume, veh/h		1911		3462	4550	534	2	7396	
One Direction DSV, 1000	veh/day	19		35	45	53	;	74	
Bi-Directional DSV, 1000 v	/eh/day	35		63	83	97	,	134	
Design Analysis Ta	ble	<u>.</u>						- -	
Number of Lanes, In	4		5		6		7		
Density, pc/mi/ln	-		33.8		27.5	27.5		23.2	
LOS	F		D		С	С		с	

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Quincy - From HOV to 93 SB - Two-Sided Weave (Ramp is I-93) Alternative 3.xuf